GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS. DO NOT SCALE STRUCTURAL DRAWINGS.
7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.
8. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
9. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
10. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
11. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
12. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
13. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.
14. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
15. SUBMITTALS
    - 15.1 SUBMITTALS BY THE CONTRACTOR ARE NOT A PART OF THE CONTRACT DOCUMENTS. PRIOR TO THE INITIAL SUBMITTAL, CONTRACTOR SHALL SUBMIT TO THE DESIGN PROFESSIONAL A SCHEDULE OF SUBMITTED INFORMATION.
    - 15.2 SUBMITTALS SHALL BE ACCOMPANIED BY A TRANSMITTAL LETTER WITH THE FOLLOWING INFORMATION:
      - PROJECT NAME
      - CONTRACTOR'S NAME
      - DATE SUBMITTED
      - DESCRIPTION OF ITEMS SUBMITTED. IDENTIFY WORK AND PRODUCT BY SPECIFICATION SECTION
      - NUMBER OF DRAWINGS AND OTHER PERTINENT DATA.
    - 15.3 CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION ON THE SUBMITTAL TO ANY DEVIATION FROM THE CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP AND SIGN EACH SHEET OF SHOP DRAWINGS AND PRODUCT DATA, AND SIGN OR INITIAL EACH SAMPLE TO CERTIFY COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. SUBMITTALS RECEIVED WITHOUT THE CONTRACTOR'S STAMP OF REVIEW WILL BE RETURNED TO THE CONTRACTOR FOR REVIEW AND RESUBMITTAL.
    - 15.4 WORK REQUIRING SHOP DRAWINGS, WHETHER CALLED FOR BY THE CONTRACT DOCUMENTS OR REQUESTED BY THE CONTRACTOR, SHALL NOT COMMENCE UNTIL THE SUBMISSION HAS BEEN REVIEWED BY THE DESIGN PROFESSIONAL. WORK MAY COMMENCE IF THE CONTRACTOR VERIFIES THE ACCURACY OF THE DESIGN PROFESSIONAL'S CORRECTIONS AND NOTATIONS AND COMPLIES WITH THEM WITHOUT EXCEPTION AND WITHOUT REQUESTING CHANGE IN CONTRACT SUM OR CONTRACT TIME AT COPY OF THE MARKED STRUCTURAL SHOP DRAWINGS WITH THE DESIGN PROFESSIONAL'S REVIEW STAMP IS TO BE MAINTAINED AT THE JOB SITE.
    - 15.5 ADJUSTMENTS MADE ON SHOP DRAWINGS BY ENGINERS ARE NOT INTENDED TO CHANGE CONTRACT PRICE. IF ADJUSTMENTS AFFECT THE VALUE OF THE WORK, STATE SUCH IN WRITING TO ENGINEER PRIOR TO PROCEEDING WITH THE WORK.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
   - INTERNATIONAL BUILDING CODE, 2024 EDITION WITH 2026 GEORGIA AMENDMENTS.
2. GRAVITY LOADS
   - 2.1 DEAD LOADS:
     - STRUCTURE SELF WEIGHT
3. WIND LOADS:
   - BASIC DESIGN WIND SPEED, V 118 MPH
   - ALLOWABLE DESIGN WIND SPEED, V_{ASD} 92 MPH
   - TORNADO SPEED, V_T 86 MPH
   - EFFECTIVE PLAN AREA, A_e 40,000 SF
   - RISK CATEGORY IV
   - EXPOSURE C
   - INTERNAL PRESSURE COEFFICIENT +/- 0
   - TORNADO INTERNAL PRESSURE COEFFICIENT +/- 0
   - COMPONENT AND CLADDING DESIGN WIND PRESSURE = 36 PSF
4. EARTHQUAKE LOADS:
   - RISK CATEGORY: IV
   - SEISMIC IMPORTANCE FACTOR: I = 1.5
   - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.240
   - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_1 = 0.091
   - SITE CLASS C
   - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{DS} = 0.17
   - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{D1} = 0.130
   - SEISMIC DESIGN CATEGORY: C
5. SPECIAL INSPECTIONS:
   - 5.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SHEET S-0.02, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE SOIL, CONCRETE, AND STEEL CONSTRUCTION. SEE SHEET S-0.02 FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.
   - 5.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
   - 5.3 THE PROJECT OWNER WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
   - 5.4 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE DESIGN PROFESSIONAL WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.
   - 5.5 SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.
6. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY MATRIX ENGINEERING GROUP INC., REPORT NUMBER MEG3303297 DATED JUNE 8, 2026. DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
3. CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3000 PSF.
   - 3.1 NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.
4. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING EQUIVALENT FLUID DENSITIES:
   - WALLS SUPPORTED AT TOP (AT-REST CONDITION): 63.7 PCF
   - WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION): 43.3 PCF
   - SLIDING COEFFICIENT OF FRICTION (ULTIMATE) 0.4
5. BACKFILL PLACED AGAINST EXTERIOR OR RETAINING WALLS SHALL NOT EXCEED 120 PCF WEIGHT FOR WET UNIT WEIGHT OF SOIL.
6. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 10'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY. STRUCTURAL FILL SOIL DENSITY SHALL BE 120 PCF.
7. BACKFILL SHALL NOT BE PLACED AGAINST EXTERIOR OR RETAINING WALLS UNTIL THE WALLS HAVE ACHIEVED THEIR DESIGN STRENGTH AND THEIR LATERAL SUPPORT ELEMENTS ARE INSTALLED. PROVIDE ADEQUATE DRAINAGE AT BASEMENT AND RETAINING WALLS (SEE ARCHITECTURAL).
8. ALL FOOTINGS SHALL PENETRATE TO A MINIMUM DEPTH OF 24" BELOW FINISHED GRADE.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS. EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.
5. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL UNLESS NOTED OTHERWISE.
6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
   - 6.1 CONCRETE REINFORCEMENT COVER

     EXPOSED TO EARTH OR WEATHER:
     - UNFORMED CAST AGAINST EARTH 3" CLEAR
     - FORMED #6 AND LARGER 2" CLEAR
     - FORMED #5 AND SMALLER 1-1/2" CLEAR
7. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
   - CONCRETE REINFORCEMENT: CLASS B TENSION LAP
8. ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL CONFORM TO ASTM C881, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. REINFORCING INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
9. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
10. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS SHOWN OTHERWISE IN DETAILS.

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:
   - 2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:
     - FOOTINGS 3000 PSI F0
     - FOUNDATION/RETAINING WALLS 4500 PSI F2
3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED, EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
   - MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
2. BOLTS AND ANCHORS:
   - 2.1 EXPANSION ANCHORS SHALL HAVE BEEN EVALUATED BY THE ICC EVALUATION SERVICES, INC (ICC-ES) WITH A PUBLISHED EVALUATION REPORT. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 193 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. ALL ANCHORS SHALL BE APPROVED FOR RESISTING WIND AND SEISMIC LOADS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
   - 2.2 ADHESIVE ANCHORS SHALL CONSIST OF AN ALL-THREAD STEEL ANCHOR WITH ADHESIVE CONFORMING TO ASTM C881, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

ABBREVIATIONS

ABT	ABOUT	(E)	EXISTING	IF	INSIDE FACE	OC	ON CENTER	T&B	TOP AND BOTTOM
ACI	AMERICAN CONCRETE INSTITUTE	EA	EACH	IBC	INTERNATIONAL BUILDING CODE	OD	OUTSIDE DIAMETER	TF	TOP OF
ADDL	ADDITIONAL	EF	EACH FACE	ICC	INTERNATIONAL CODE COUNCIL	OF	OUTSIDE FACE	THK	THINK
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	EJ	EXPANSION JOINT	ID	INSIDE DIAMETER	OPNG	OPENING	THRU	THROUGH
ALT	ALTERNATE	EL	ELEVATION	IE	INVERT ELEVATION	OPP	OPPOSITE	TYP	TYPICAL
APPROX	APPROXIMATE	ELEC	ELECTRICAL	IN	INCH	OSH	OVERSIZED HOLE		
ARCH	ARCHITECTURAL/ARCHITECT	ENGR	ENGINEER	INT	INTERIOR			UL	UNDERWRITER'S LABORATORIES
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	EOD	EDGE OF DECK			PCF	POUNDS PER CUBIC FOOT	UNO	UNLESS NOTED OTHERWISE
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	EOR	ENGINEER OF RECORD	JT	JOINT	PE	PROFESSIONAL ENGINEER		
AWS	AMERICAN WELDING SOCIETY	EOS	EDGE OF SLAB			PERIM	PERIMETER	VERT	VERTICAL
		EQ	EQUAL	K	KIP(S)	PJF	PREMOLDED JOINT FILLER		
B	BOTTOM OF	EQUIP	EQUIPMENT	KSF	KIPS PER SQUARE FOOT	PL	PLATE	W	WITH
BLDG	BUILDING	EW	EACH WAY	KSI	KIPS PER SQUARE INCH	PLCS	PLACES	W/O	WITHOUT
BM	BEAM	EXP	EXPANSION			PLF	POUNDS PER LINEAR FOOT	WP	WORKING POINT
BOTT	BOTTOM	EXT	EXTERIOR	(LLH)	LONG LEG HORIZONTAL (ANGLE)	PREFAB	PREFABRICATED	WS	WATERSTOP
BP	BASE PLATE			(LLV)	LONG LEG VERTICAL (ANGLE)	PSF	POUNDS PER SQUARE FOOT	WT	WEIGHT
BRG	BEARING	FD	FLOOR DRAIN	(LSH)	LONG SIDE HORIZONTAL (HSS)	PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC
BTWN	BETWEEN	FDN	FOUNDATION	(LSV)	LONG SIDE VERTICAL (HSS)	PT	POINT		
		FF	FINISHED FLOOR	LB	POUND				
C/C	CENTER TO CENTER	FFE	FINISHED FLOOR ELEVATION	LF	LINEAR FEET	R	RADIUS		
CALC	CALCULATION(S)	FG	FINISHED GRADE	LL	LIVE LOAD	RD	ROOF DRAIN		
CANT	CANTILEVER	FIN	FINISH	LOC	LOCATION	REF	REFERENCE		
CFMF	COLD-FORMED METAL FRAMING	FLG	FLANGE	LONG	LONGITUDINAL	REINF	REINFORCING		
CHKD	CHECKED	FLR	FLOOR	LP	LOW POINT	REQD	REQUIRED		
CIP	CAST-IN-PLACE CONCRETE	FRMG	FRAMING	LSH	LONG SLOTTED HOLE	RET	RETURN		
CJ	CONSTRUCTION CONTROL JOINT	FRP	FIBER REINFORCED PLASTIC	LWC	LIGHT WEIGHT CONCRETE	REV	REVISION		
CJP	COMPLETE JOINT PENETRATION	FS	FAR SIDE			RO	ROUGH OPENING		
CL	CENTERLINE	FT	FOOT	M	MOMENT	RTU	ROOFTOP UNIT		
CLR	CLEAR, CLEARANCE	FTG	FOOTING	MAX	MAXIMUM				
COL	COLUMN	FV	FIELD VERIFY	MC	MOMENT CONNECTION	(SLV)	SHORT LEG VERTICAL		
CONC	CONCRETE			MECH	MECHANICAL	SC	SLIP CRITICAL		
CONN	CONNECTION	GA	GAGE, GAUGE	MEP	MECHANICAL, ELECTRICAL, AND PLUMBING	SCHED	SCHEDULE		
CONT	CONTINUOUS	GALV	GALVANIZED (HOT DIP)	MFC	MANUFACTURED	SECT	SECTION		
COORD	COORDINATE	GC	GENERAL CONTRACTOR	MFR	MANUFACTURER	SHT	SHEET		
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	GRTG	GRATING	MIN	MINIMUM	SIM	SIMILAR		
CTR	CENTER			MISC	MISCELLANEOUS	SL	SLOPE		
CTRD	CENTERED	(H)	HORIZONTAL BEAM ORIENTATION	MTD	MOUNTED	SPCS	SPACES		
CW	CURTAINWALL	HCA	HEADED CONCRETE ANCHOR	MTL	METAL	SPEC(S)	SPECIFICATION(S)		
		HDR	HEADER			SQ	SQUARE		
DBA	DEFORMED BAR ANCHOR	HGR	HANGER	N&F	NEAR AND FAR	SS	STAINLESS STEEL		
DBE	DECK BEARING ELEVATION	HK	HOOK	N/A	NOT APPLICABLE	SSH	SHORT SLOTTED HOLE		
DBL	DOUBLE	HORIZ	HORIZONTAL	NIC	NOT IN CONTRACT	STD	STANDARD		
DEG	DEGREES	HP	HIGH POINT	NO/NO.	NUMBER	STIF	STIFFENER		
DET	DETAIL	HR	HANDRAIL	NOM	NOMINAL	STL	STEEL		
DIA	DIAMETER	HSB	HIGH STRENGTH BOLT	NS	NEAR SIDE	STRUCT	STRUCTURAL		
DIAG	DIAGONAL			NTS	NOT TO SCALE	SYM	SYMMETRICAL		
DIR	DIRECTION			NWC	NORMAL WEIGHT CONCRETE				
DL	DEAD LOAD								
DWG	DRAWING								

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GEORGIA REGISTERED No. 34000174 PROFESSIONAL ENGINEER MOLLY C. JEFFREYS

georgia civil CIVIL ENGINEERING LANDSCAPE ARCHITECTURE LAND SURVEYING

SHEAR STRUCTURAL

TOTAL ENGINEERS

Electrical Engineers

CITY OF COVINGTON ESTABLISHED 1822 FIRE RESCUE GEORGIA FIRE DEPARTMENT

FIRE STATION #23

City of Covington

HIGHWAY 36
COVINGTON, GA 30014

PRINT RECORD

No.	DATE	DESCRIPTION
	07/02/2026	WALL PACKAGE

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STRUCTURAL GENERAL NOTES AND ABBREVIATIONS

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