

GENERAL NOTES:

1. ALL WORK SHALL COMPLY WITH THE 2024 EDITION OF THE INTERNATIONAL BUILDING CODE, AND IN STRICT COMPLIANCE WITH GOVERNING MUNICIPAL CODES (CITY, STATE, AND FEDERAL).
2. ASTM SPECIFICATIONS ARE THOSE CONTAINED IN THE LATEST EDITION OF THE STANDARDS OF THE AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM).
3. IN THE CASE OF A CONFLICT BETWEEN THESE PROJECT SPECIFICATIONS, DRAWINGS, AND/OR THOSE LISTED, REFERENCED SPECIFICATIONS, OR CODE, THE MORE STRINGENT SHALL GOVERN.
4. USE ALL MEANS NECESSARY TO CONTROL DUST ON AND NEAR THE WORK AND ON AND NEAR ALL OFF-SITE BORROW AREAS IF SUCH DUST IS CAUSED BY THE CONTRACTOR'S OPERATIONS DURING PERFORMANCE OF THE WORK OR IF RESULTING FROM THE CONDITION IN WHICH THE CONTRACTOR LEAVES THE SITE.
5. THOROUGHLY MOISTEN ALL SURFACES AS REQUIRED TO PREVENT DUST BEING A NUISANCE TO THE PUBLIC, NEIGHBORS, AND CONCURRENT PERFORMANCE OF OTHER WORK ON THE SITE.
6. USE ALL MEANS NECESSARY TO PROTECT ALL MATERIALS ON THIS PROJECT BEFORE, DURING, AND AFTER INSTALLATION. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTION OF ALL WORK AND MATERIALS.
7. ALL WORK SHALL BE ACCOMPLISHED IN A WORKMAN LIKE MANNER. ALL WORK SHALL BE CLEAN AND NEAT AND EASILY INSPECTED.
8. CALCULATED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS.
9. CONTRACTOR TO VERIFY ALL MEASUREMENTS ON JOB SITE TO ENSURE FIT. IN CASE OF DISCREPANCIES BETWEEN DRAWINGS, SHOP DRAWINGS, AND SPECIFICATIONS NOTIFY THE ARCHITECT AND ENGINEER IMMEDIATELY.
10. THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURAL DESIGN OF BUILDING IS BASED ON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS, WITH NO PROVISION FOR CONDITION OCCURRING DURING CONSTRUCTION. THEREFORE, CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION.
11. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND SPECIFICATIONS. CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES REGARDING INSERTS, CLIPS, OPENINGS, ETC., TO BE PLACED IN THE STRUCTURAL WORK. STRUCTURAL DRAWINGS DO NOT NECESSARILY SHOW ALL OPENINGS IN STRUCTURAL WORK. VERIFY NUMBER, SIZE AND LOCATION OF ALL OPENINGS IN ROOF DECK FROM ARCHITECTURAL DRAWINGS AND APPROVED MECHANICAL, PLUMBING AND ELECTRICAL SHOP DRAWINGS.
12. NO LOADS IN EXCESS OF DESIGN LOADS LISTED SHALL BE PLACED ON ANY AREA DURING CONSTRUCTION, UNLESS ADEQUATE SHORING OR OTHER METHODS APPROVED BY THE ENGINEER ARE PROVIDED TO SUPPORT THE EXCESSIVE LOADS. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE FRAMING UNTIL PERMANENT BRACING IS INSTALLED.
13. WHERE ALIGNMENT OF MATERIALS SUCH AS WALLS AND FACING MATERIALS WILL BE AFFECTED BY DEFLECTIONS AND ROTATIONS OF THE STRUCTURE DURING PLACING OF THE MATERIALS, PLACING PROCEDURES SHALL BE USED WHICH WILL ASSURE THE CORRECT FINAL POSITION OF MATERIALS.
14. ALL NOTES ON STRUCTURAL DRAWINGS SHALL BE ASSUMED TYPICAL UNLESS OTHERWISE SHOWN BY OTHER DETAILS AND/OR SECTIONS.
15. STRUCTURE DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOW DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND SPECIFIED REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS. SECTIONS AND DETAILS ARE TO BE USED IN ALL SIMILAR LOCATIONS UNLESS NOTED OTHERWISE ON DRAWINGS OR SPECIFICATIONS.
16. SEE ARCHITECTURAL DRAWINGS FOR WEATHERPROOFING DETAILS.
17. REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
18. STRUCTURAL DESIGN DRAWINGS SHALL NOT BE REPRODUCED AS SHOP DRAWINGS. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL PREPARE THEIR OWN SHOP DRAWINGS.
19. MATERIALS:
 - CONCRETE (NORMAL WEIGHT - 28 DAY COMPRESSIVE STRENGTH) - SLAB ON GRADE = 3600 PSI
 - FOOTINGS, WALLS AND PLASTER = 4000 PSI
 - REINFORCING = ASTM A615, GRADE 60
 - REINFORCING TO BE WELDED = ASTM A706, GRADE 60
 - WELDED WIRE FABRIC = ASTM A185
 - PLATES, ANGLES AND BARS = ASTM A36, Fy = 36 KSI
 - STEEL TUBING (HSS) = ASTM A500, GRADE B, Fy = 46 KSI
 - ELECTRODES FOR WELDING = AWS SERIES E70XX
 - FRAMING ANCHORS (CONCRETE) = HILTI OR EQUIVALENT
20. FIELD VERIFY ALL EXISTING ABOVE AND BELOW GROUND CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTIONS.
21. THE STRUCTURAL DESIGN OF BUILDING IS BASED ON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS, WITH NO PROVISION FOR CONDITION OCCURRING DURING CONSTRUCTION. THEREFORE, CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION.

FOUNDATION NOTES

1. ALL FOUNDATIONS ARE DESIGNED BASED UPON AN ASSUMED ALLOWABLE BEARING CAPACITY OF 2,000 PSF. IT IS STRONGLY RECOMMENDED THAT THE CONTRACTOR RETAIN THE SERVICES OF A LICENSED GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF GEORGIA TO VERIFY ALLOWABLE BEARING CAPACITY.
2. FOOTINGS AND PIERS SHOWN ON PLAN ARE DIAGRAMMATIC ONLY. REFER TO SCHEDULES AND DETAILS FOR SIZE OF EACH FOOTING AND PIER.
3. DIMENSIONS AND LINES SHOWN FOR GRADE BEAMS ARE TO INSIDE BOTTOM EDGE OF GRADE BEAM. TAPER TO SLAB NOT SHOWN FOR CLARITY.
4. CENTER LINE OF COLUMN = CENTER LINE OF FOOTING = CENTER LINE OF ANCHOR BOLT TEMPLATE UNLESS NOTED OTHERWISE.
5. ALL FILL SHALL BE SELECT STRUCTURAL FILL CLASSIFIED AS OR "ENGINEERED FILL," AFTER STRIPPING TOPSOIL FROM AREAS TO BE GRADED REMOVE ALL UNSUITABLE MATERIAL FROM EXPOSED SUB GRADE SURFACE, SUCH AS DEBRIS, TRASH OR ORGANIC MATTER. SOIL SURFACES TO RECEIVE FILL SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER BEFORE FILL IS PLACED.
6. FILL MATERIAL SHOULD BE FREE OF ORGANICS, STONE GREATER THAN SIX INCHES IN DIAMETER, OR OTHER DELETERIOUS MATERIAL. ALL FILL SHALL BE PLACED IN MAXIMUM 8" UNCOMPACTED LIFTS AND COMPACTED TO AT LEAST 95% STANDARD PROCTOR DENSITY (ASTM D698). COMPACTION SHALL BE AT THE OPTIMUM MOISTURE CONTENT +/- 2%, THE FINAL 12 INCHES (EXISTING SOIL OR ENGINEERED FILL) SHALL BE COMPACTED TO 98% STANDARD PROCTOR DENSITY. REFER TO THE GEOTECHNICAL ENGINEERING REPORT FOR GUIDANCE ON FILL MATERIAL, SPECIFICATIONS AND COMPACTION EQUIPMENT AND PROCEDURES.
7. ALL FOUNDATION EXCAVATIONS SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER, AND APPROVED FOR FOOTINGS, PRIOR TO PLACING CONCRETE. ALL FOUNDATIONS SHALL BE CONCRETED PROMPTLY AFTER INSPECTION.
8. BACK FILL AGAINST WALLS SPANNING VERTICALLY SHALL NOT BE PLACED, WHERE POSSIBLE, UNTIL ALL FLOORS AGAINST THOSE WALLS ARE IN PLACE AND AT FULL DESIGN STRENGTH. IF FLOORS CANNOT BE PLACED BEFORE FILL, WALLS SHALL BE ADEQUATELY BRACED TO PREVENT OVER STRESSING OR MOVEMENT.
9. ALL MASONRY WALLS BELOW GRADE SHALL BE BACK FILLED ON BOTH SIDES OF WALL SIMULTANEOUSLY.
10. HORIZONTAL STEPS IN WALL FOOTINGS, IF REQUIRED, SHALL HAVE A MINIMUM SPACING OF DOUBLE THE CHANGE IN ELEVATION.
11. PROTECT STRUCTURAL STEEL ITEMS BELOW GRADE WITH 3" CLEAR COVER OF CONCRETE ENCASEMENT.
12. CONTRACTOR SHALL MAINTAIN ADEQUATE SITE DRAINAGE DURING CONSTRUCTION TO DIRECT RAINWATER AWAY FROM FOUNDATION CONSTRUCTION AREAS.
13. COORDINATE EXTERIOR SITE WORK, INCLUDING STEPS, WALKS, WALLS AND FINISHED GRADES, WITH FOUNDATION WORK.
14. ALL SLABS SHALL BE SUPPORTED ON A 4" LAYER OF COMPACTED CLEAN, GRANULAR BASE. THE GRANULAR BASE SHALL BE COMPACTED TO AT LEAST 98% STANDARD PROCTOR DENSITY (ASTM D698). THE GRANULAR BASE SHALL BE COMPRISED OF NATURAL OR ARTIFICIALLY GRADED MIXTURE OF NATURAL OR CRUSHED GRAVEL, CRUSHED STONE, AND NATURAL OR CRUSHED SAND. ASTM D 2540: WITH AT LEAST 85 PERCENT PASSING A 1-1/2" INCH SIEVE AND NOT MORE THAN 8 PERCENT PASSING A NO. 200 SIEVE.
15. A MINIMUM 10 MIL VAPOR BARRIER SHALL BE INSTALLED UNDER ALL CONCRETE SLAB WORK. ALL LAPS IN THE VAPOR BARRIER SHALL BE 6" MINIMUM AND SHALL BE TACKED TOGETHER. COORDINATE VAPOR BARRIER REQUIREMENTS WITH ARCHITECTURAL.
16. THE TOPS OF ALL FOOTINGS SHALL BE PLACED A MINIMUM OF 12" BELOW UNDISTURBED GROUND SURFACE.
17. ALL MASONRY FOUNDATION WALLS SHALL BE DAMP PROOFED IN ACCORDANCE WITH CHAPTER 18 OF IBC.
18. SLAB BASE SHOULD BE FINISHED TO THE FOLLOWING TOLERANCES: +0/-3/4 in. WHEN MEASURED FROM BOTTOM OF SLAB ELEVATION.
19. AS A MINIMUM, THE GEOTECHNICAL ENGINEER'S SERVICES SHALL INCLUDE:
 - a. PRIOR TO COMMENCEMENT OF FILLING, THE GEOTECHNICAL ENGINEER SHALL MAKE OPTIMUM MOISTURE AND MAXIMUM DENSITY TESTS IN ACCORDANCE WITH ASTM D698.
 - b. THE GEOTECHNICAL ENGINEER SHALL APPROVE THE SUBGRADE FOR COMPACTED STRUCTURAL FILL OR CONCRETE FOOTING SUPPORT BEFORE ANY FILL IS PLACED OR CONCRETE IS POURED.
 - c. THE GEOTECHNICAL ENGINEER SHALL REVIEW THE COMPACTION OPERATIONS.
 - d. THE GEOTECHNICAL ENGINEER SHALL RENDER FULL-TIME FIELD INSPECTION DURING FILLING OPERATIONS AND SHALL DETERMINE THE DENSITY OF EACH LAYER OF FILL.
 - e. RECORDS OF ALL TESTING SHALL BE MAINTAINED BY THE GEOTECHNICAL ENGINEER AND COPIES OF THE SAME BE SENT DIRECTLY TO THE ENGINEER.
 - f. THE FOLLOWING IS THE MINIMUM COMPACTION TESTING FREQUENCIES:
 - ONE COMPACTION TEST SHALL BE MADE FOR EVERY 500 SQUARE FEET OF FILL AT EACH 12" LIFT.
 - ONE COMPACTION TEST SHALL BE MADE FOR EVERY 500 SQUARE FEET OF COMPACTED GRANULAR FILL.
 - COMPACTION TESTS SHOULD BE PERFORMED ON THE SUB-BASE BELOW STRIP FOOTINGS AT A MINIMUM OF EVERY 50'.

CLASS B SPICE LENGTHS		
BAR SIZE	SPICE LENGTH (3000 PSI CONCRETE)	SPICE LENGTH (4000 PSI CONCRETE)
#3	17"	16"
#4	23"	20"
#5	29"	25"
#6	35"	30"
#7	62"	54"
#8	72"	62"

MASONRY NOTES

1. MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 5) AND "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6) PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE, EXCEPT AS MODIFIED BY THE CONTRACTOR DOCUMENTS.
2. CONCRETE BLOCK UNITS SHALL BE NORMAL WEIGHT, HOLLOW, LOAD BEARING UNITS AND COMPLY WITH ASTM C90, GRADE N, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON THE NET AREA. ALL BLOCK SHALL BE TYPE 1, MOISTURE CONTROLLED UNITS.
3. BRICK UNITS SHALL COMPLY WITH ASTM C62, GRADE SW, 6000 PSI COMPRESSIVE STRENGTH.
4. ALL MORTAR IS TO BE IN ACCORDANCE WITH ASTM C270, UNLESS NOTED OTHERWISE. MORTAR FOR EXTERIOR AND FOUNDATION WALL MASONRY SHALL BE TYPE "S". PORTLAND CEMENT/LIME MORTAR. MORTAR FOR INTERIOR LOAD BEARING MASONRY SHALL BE TYPE S. MORTAR FOR INTERIOR NON-LOADBEARING MASONRY SHALL BE TYPE N OR S.
5. UNLESS SPECIFICALLY NOTED OTHERWISE, ALL CONCRETE MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (FM) OF 1500 PSI. ALL BRICK MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI. COMPRESSIVE STRENGTH SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD.
6. ALL REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A615 (S1). NEW BILLET STEEL DEFORMED BARS SHALL BE GRADE 60.
7. ALL MASONRY REINFORCING STEEL SHALL BE INSTALLED IN ACCORDANCE WITH ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCING".
8. MASONRY JOINT REINFORCEMENT IS TO COMPLY WITH ASTM A951, MILL GALVANIZED CARBON STEEL WIRE FOR INTERIOR WALLS AND HOT-DIP GALVANIZED CARBON STEEL WIRE FOR EXTERIOR WALLS. HORIZONTAL JOINT REINFORCEMENT IS TO BE EITHER LADDER OR TRUSS TYPE WITH SINGLE PAIR OF SIDE RODS AND CROSS RODS SPACED NO MORE THAN 16" O.C. LAP REINFORCEMENT A MINIMUM OF 6". PLACE MORTAR ON JOINT REINFORCEMENT WHERE THE CAVITY WALL IS 4" OR MORE. HORIZONTAL JOINT REINFORCEMENT SHALL BE PROVIDED WITHIN 16" OF THE TOP AND BOTTOM OF WALLS.
9. ALL ANCHOR BOLTS AND REINFORCING IN MASONRY SHALL BE COMPLETELY GROUTED. GROUT FOR COLLAR JOINTS IN WALLS SHALL BE MORTAR. GROUT FOR REINFORCING BARS AND ANCHOR BOLTS SHALL BE SAND AGGREGATE CEMENT GROUT. MASONRY CELLS TO BE GROUTED SHALL BE CLEAR OF MORTAR PINS, DEBRIS OR OTHER OBSTRUCTIONS, SUCH THAT THE FLOW OF GROUT IS NOT INHIBITED.
10. ALL GROUT FOR GROUTING MASONRY BLOCK CORES SHALL BE MIN. 3000 PSI SAND AGGREGATE CEMENT GROUT, MEETING ASTM C476. SLUMP FOR THIS GROUT SHALL BE 8". MAXIMUM GROUT LIFT TO BE 5 FT. ALLOW A MINIMUM OF 24 HOURS FOR MASONRY TO SET PRIOR TO GROUTING. ROD OR VIBRATE GROUT DURING PLACEMENT TO INSURE SOLID GROUTING. ALLOW AT LEAST 15 MINUTES BETWEEN SUCCESSIVE LIFTS.
11. ALL MASONRY AND COLLAR JOINTS BELOW FINISHED FLOOR ELEVATION ARE TO BE GROUTED SOLID.
12. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF MASONRY CONTROL JOINTS.
13. USE A FULL MORTAR BED FOR FIRST STARTER COURSE. USE FACE SHELL MORTAR BEDS ABOVE.
14. ALL REINFORCING BARS ARE TO BE LAPPED IN ACCORDANCE WITH ACI 309.26. SPICE LENGTH TO BE 48" X BAR DIAMETER BUT NOT LESS THAN 12".
15. DURING ERECTION, COVER TOP OF WALLS WITH WATERPROOF SHEETING AT END OF EACH DAY'S WORK. COVER PARTIALLY COMPLETED STRUCTURES WHEN WORK IS NOT IN PROGRESS.
16. DO NOT APPLY UNIFORM FLOOR OR ROOF LOADING FOR AT LEAST 12 HOURS AFTER BUILDING MASONRY WALLS.
17. DO NOT APPLY CONCENTRATED LOADS FOR AT LEAST 3 DAYS AFTER BUILDING MASONRY WALLS OR COLUMNS.
18. ALL CMU IS TO BE LAYED WITH RUNNING BONDS WITH VERTICAL JOINTS LOCATED AT CENTER OF MASONRY UNITS IN THE ALTERNATE COURSE BELOW.
19. PROVIDE CONTINUITY AT ALL WALL CORNERS BY CONFORMING TO ONE OF THE FOLLOWING:
 - a. FIFTY PERCENT OF THE MASONRY UNITS AT THE INTERFACE SHALL INTERLOCK.
 - b. WALLS SHALL BE REGULARLY TOOTHED WITH 8 IN. MAXIMUM OFFSETS AND ANCHORED BY STEEL CONNECTORS WITH A MINIMUM SIZE OF 1/2" x 12" x 36" INCLUDING 2" LONG 90 DEGREE BEND AT EACH END TO FORM A U OR Z SHAPE. ANCHORS ARE TO BE SPACED VERTICALLY AT 4' O.C.
20. THE ADHESIVE REQUIRED FOR ANCHORAGE INTO GROUT FILLED CONCRETE MASONRY UNITS SHALL BE HILTI HIT HY 150 OR EQUAL. THE ADHESIVE REQUIRED FOR ANCHORAGE INTO HOLLOW CONCRETE UNITS SHALL BE HILTI HIT HY 20 OR EQUAL. FOLLOW THE MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION PROCEDURES.
21. ANCHORS USED FOR MASONRY ATTACHMENT SHALL BE STANDARD HAS ROD MATERIAL, MEETING THE REQUIREMENTS OF ASTM A36. ANCHOR RODS SHALL BE FURNISHED WITH CHAMFERED ENDS.

STEEL LINTEL SCHEDULE

FOR EACH 4" THICKNESS OF WALL, THE FOLLOWING ITEMS SHALL BE USED:

STEEL (FOR BRICK WORK)

- 2'-0" - 5'-0" L4" x 3 1/2" x 3/4" LLV
- 5'-0" - 6'-0" L4" x 3 3/2" x 3/4" LLV
- 6'-0" - 7'-0" L5" x 3 1/2" x 3/4" LLV
- 7'-0" - 8'-0" L5" x 3 1/2" x 3/4" LLV
- 8'-0" - 9'-0" L6" x 3 1/2" x 3/4" LLV
- 9'-0" - 10'-0" L7" x 4" x 3/4" LLV

1. LINTELS SHALL BEAR 8" MINIMUM EACH END UNLESS NOTED OTHERWISE AT L7"x4" BEAR 12" MIN. EACH END.

2. GROUT SOLID BETWEEN BACK TO BRICK AND ANGLE.

3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DETAILS AT ALL STRUCTURAL SECTIONS AND DETAILS.

4. PROVIDE 12 ga. CLOSURE PLATE AS REQUIRED.

CONCRETE NOTES

1. ALL DETAILING, FABRICATION AND PLACEMENT OF REINFORCING STEEL, FORM WORK, MIXING, HANDLING, PLACING, FINISHING AND CURING OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI-315) AND ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI-318).
2. CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR REVIEW SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
3. THE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES.
4. CONCRETE SHALL CONFORM TO ASTM C94, MINIMUM STRENGTH AT 28 DAYS SHALL BE 3500 PSI FOR ALL CONCRETE, UNLESS NOTED OTHERWISE. MAXIMUM WATER-CEMENT RATIO SHALL BE 0.50, WITH MAXIMUM SLUMP 4 INCHES. MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE 1 1/2" INCH, AND ALL AGGREGATES SHALL CONFORM TO ASTM C33. NO FLY ASH WILL BE UTILIZED IN THE CONCRETE MIX DESIGN.
5. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.
6. CONCRETE SLABS ON GRADE SHALL BE FINISHED TO THE FOLLOWING TOLERANCES: COMPOSITE FLATNESS (Ft) = 25, COMPOSITE LEVELNESS (FL) = 20.
7. EXTERIOR CONCRETE SHALL BE AIR ENTRAINED. AIR CONTENT TO BE BETWEEN 4 AND 6 PERCENT BY VOLUME. MAXIMUM WATER-CEMENT RATIO SHALL BE 0.50.
8. ALL REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A615 (S1). NEW BILLET STEEL DEFORMED BARS SHALL BE GRADE 60. WELDED WIRE FABRIC (WWF) TO MEET ASTM A185. MINIMUM WWF LAP AT SPLICES TO BE 8 INCHES.
9. PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. SPLICE ONLY AS SHOWN OR APPROVED. STAGGER SPLICES WHERE POSSIBLE. UNLESS NOTED OTHERWISE, ALL REINFORCING BAR SPLICES SHALL BE ACI CLASS B TENSION LAP SPLICES.
10. THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT NEAREST THE DESCRIBED SURFACE, UNLESS OTHERWISE NOTED:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 IN. CONCRETE EXPOSED TO EARTH OR WEATHER: # 6 OR LARGER BARS: 2 IN. # 6 OR SMALLER BARS: 1 1/2 IN. CONCRETE NOT EXPOSED TO EARTH OR WEATHER: SLAB AND WALLS: 1" BEAM STIRRUPS AND COLUMN TIES: 1 1/2"
11. UNLESS OTHERWISE SHOWN ALL SLABS ON GRADE SHALL BE 4" CONCRETE WITH 6x6w/2x1w/2 1 WELDED WIRE FABRIC (1'-12" CLEAR FROM TOP OF SLAB). WWF SHEETS ARE REQUIRED IN LIEU OF ROLLS. WELDED WIRE FABRIC SHALL BE SET ON SUPPORTS DURING CONCRETE POURING. "PULL UP" AND "WALKED IN" METHODS ARE PROHIBITED.
12. UNLESS NOTED OTHERWISE, SLABS ON GRADE SHALL HAVE EITHER CONSTRUCTION JOINTS OR SAW CUT JOINTS SPACE SO THAT THE JOINTS FORM PANELS IN THE SLAB, WITH NO SLAB PANEL GREATER THAN 225 SQUARE FEET NOR MORE THAN 15 FEET IN ANY ONE DIRECTION. INSTALL SAW CUT CONTROL JOINTS AS SOON AS THE SLAB IS CAPABLE OF BEING SAWN WITHOUT RAVELING, BUT IN NO CASE LATER THAN 8 HOURS AFTER FINAL FINISHING HAS BEGUN.
13. INTERIOR SLAB CONCRETE SHALL RECEIVE A STEEL TROWEL FINISH. IMMEDIATELY FOLLOWING FINISHING THE CONCRETE SHALL BE PROTECTED FROM PREMATURE OR EXCESSIVE DRYING, TEMPERATURE EXTREMES AND INJURY. COORDINATE CURING PROCEDURES WITH FLOOR FINISH REQUIREMENTS. AIR CONTENT OF FLOORS TO RECEIVE TROWEL FINISH SHALL NOT EXCEED 3 PERCENT.
14. TAKE 5 CYLINDERS OF EACH CONCRETE POUR. TEST 2 AT 7 DAYS AND 2 AT 28 DAYS. HOLD ONE CYLINDER FOR POSSIBLE TEST UNTIL 60 DAYS, THEN DISPOSE OF TEST NOT REQUESTED. SEND REPORTS TO ARCHITECT AND STRUCTURAL ENGINEER.
15. THE PROPOSED MATERIALS SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE OWNER'S TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S.
16. ALL CONCRETE FORM WORK SHALL BE NEW PLYWOOD HAVING A FINISHED SURFACE AND THICKNESS SUFFICIENT TO PRODUCE STRAIGHT AND TRUE SURFACES. THE SIDES OF ALL FOOTINGS SHALL BE FORMED.
17. HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED OR APPROVED. THE LOCATION OF VERTICAL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. AT ALL COLD JOINTS SCARIFY AND APPLY AN EPOXY BONDING AGENT OVER OLDER CONCRETE.

METAL BUILDING NOTES

1. SEE ARCHITECTURAL DRAWINGS FOR ALL CENTER-LINE, SETBACK, VERTICAL AND CLEAR HEIGHTS. THESE SUPERCEDE ANY MANUFACTURER'S STANDARD DIMENSIONS.
2. SEE ARCHITECTURAL DRAWINGS FOR FRAME SHAPES AND OVERHANG REQUIREMENTS.
3. SEE STRUCTURAL DRAWINGS FOR PORTAL FRAME LONGITUDINAL WIND BRACING LOCATIONS.
4. METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR SIZING ALL ANCHOR BOLTS AND BASE PLATES.
5. NO OPENINGS SHALL BE CUT IN STRUCTURAL MEMBERS UNLESS SHOWN ON THE DRAWINGS AND APPROVED BY THE METAL BUILDING DESIGNER.
6. FIELD WELDS SHALL BE WITH E70XX ELECTRODES AND SHALL MEET AWS D1.1, WELDING SHALL BE PERFORMED BY APPROVED CERTIFIED WELDERS AND SHALL CONFORM TO THE PROVISIONS OF THE "STRUCTURAL WELDING CODE - STEEL" OF THE AMERICAN WELDING SOCIETY.
7. DO NOT USE GAS CUTTING TORCHES FOR CORRECTING FABRICATION ERRORS IN THE STRUCTURAL FRAMING.
8. CONTRACTOR TO VERIFY ALL CONDITIONS AND ALL DIMENSIONS PRIOR TO FABRICATION OF STEEL. NOTIFY THE ARCHITECT AND ENGINEER IMMEDIATELY IF ANY DISCREPANCIES EXIST.
9. ALL BOTTOM FLANGE BRACING TO BE CONSISTENT DEPTH ACROSS RIGID FRAME OF PRE-ENGINEERED METAL BUILDING UNLESS OTHERWISE APPROVED BY ARCH.
10. FOUNDATION SIZES ARE BASED UPON ESTIMATED REACTIONS. CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR EOR REVIEW PRIOR TO LAYOUT OR CONSTRUCTION OF FOUNDATIONS. FOUNDATION SIZES MAY BE ADJUSTED PENDING FINAL REACTIONS FROM MBM. SHOP DRAWINGS AND METAL BUILDING REACTIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF GEORGIA.
11. METAL BUILDING MANUFACTURER TO DESIGN PEMB FOR MAXIMUM LATERAL DRIFT OF 1/400 AND ALL DESIGNED COMPONENTS BACKING UP TO MASONRY TO L800.

FRAMING NOTES

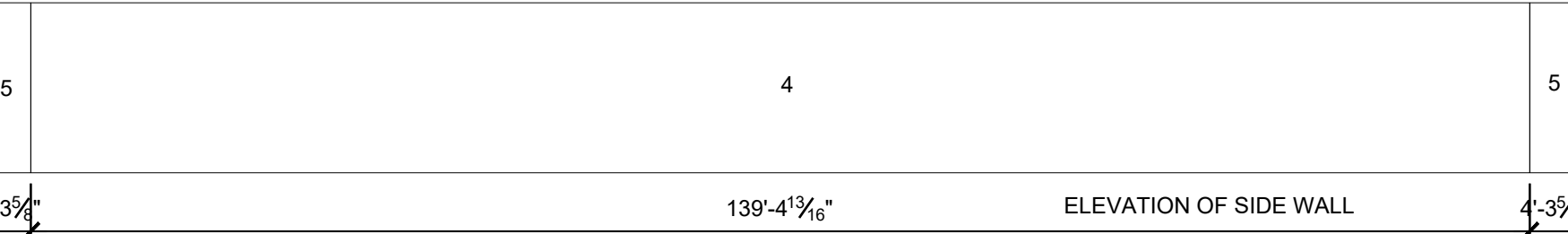
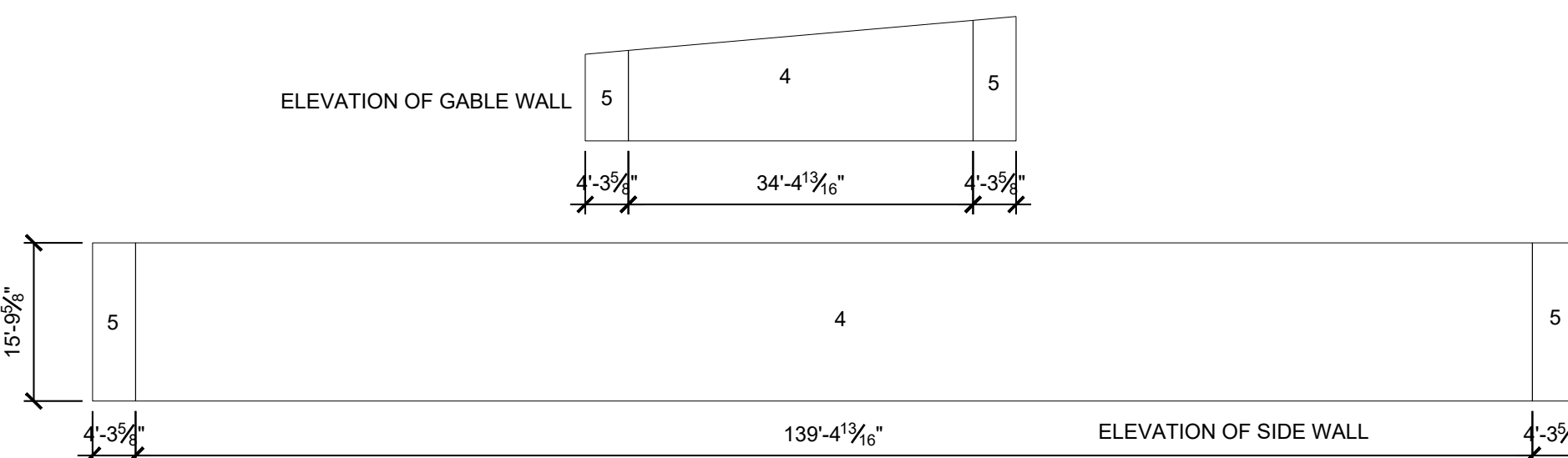
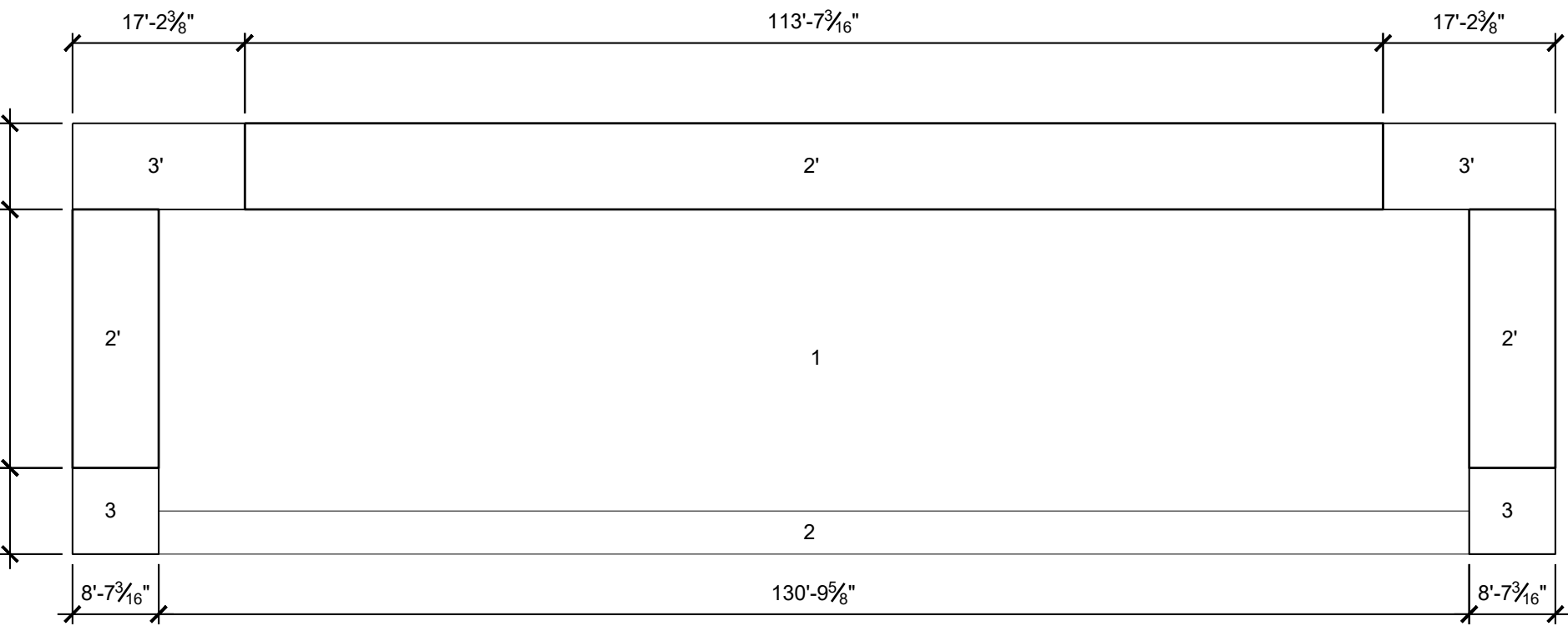
1. ALL WORK SHALL COMPLY WITH THE 2024 EDITION OF THE INTERNATIONAL BUILDING CODE, AND IN STRICT COMPLIANCE WITH GOVERNING MUNICIPAL CODES (CITY, STATE, AND FEDERAL).
- DESIGN LOADS:
 - LIVE LOADS (PER IRC)
 - ROOF = 20 PSF
 - WIND LOADS (PER ASCE)
 - CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR EOR REVIEW PRIOR TO BUILDING CATEGORY = IV
 - WIND EXPOSURE CATEGORY B
 - INTERNAL PRESSURE COEFFICIENT (GCP) = +0.18
 - WIND FORCES ARE CALCULATED USING THE DIRECTIONAL PROCEDURE
 - COMPONENTS & CLADDING WIND PRESSURES: (WITHOUT ANY LOAD COMB. REDUCTIONS)
 - WALLS - ZONE 4 = +20.0 PSF, -21.0 PSF
 - ZONE 5 = +20.0 PSF, -26.0 PSF
 - ROOF - ZONE 1 = +16.0 PSF, -23.0 PSF
 - ZONE 2 = +16.0 PSF, -26.0 PSF
 - ZONE 3 = +16.0 PSF, -36.0 PSF
 - ZONE 2' = +16.0 PSF, -32.0 PSF
 - ZONE 3' = +16.0 PSF, -30.0 PSF
 - EDGE STRIP (a) = SEE DIAGRAM
 - SEISMIC LOADS (PER IRC)
 - RISK CATEGORY IV
 - SEISMIC IMPORTANCE FACTOR = 1.5
 - SPECTRAL RESPONSE COEFFICIENTS
 - Ss = 0.27g
 - Sl = 0.052g
 - Sds = 0.22g
 - Sd1 = 0.13g
 - SITE CLASS D
 - SEISMIC DESIGN CATEGORY C
 - BLOCK SHEAR WALLS:
 - SEISMIC RESPONSE COEFFICIENTS (Cs) = 0.116
 - RESPONSE MODIFICATION FACTOR (R) = 2
 - SEISMIC DESIGN CATEGORY C
 - GROUND SNOW LOAD (Pg) = 5 psf
 - SNOW IMPORTANCE FACTOR = 1.2
 - ICE IMPORTANCE FACTOR = 1.25

SEISMIC FORCE-RESISTING SYSTEMS ARE ORDINARY REINFORCED MASONRY SHEAR WALLS.

EQUIVALENT LATERAL FORCE PROCEDURE WAS UTILIZED TO OBTAIN SEISMIC FORCES.

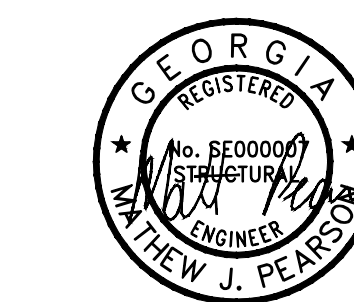
TRUSS DESIGN LOADS TO BE AS FOLLOWS:

ROOF TRUSSES	
TOP CHORD LIVE LOADS	20 PSF
BOTTOM CHORD DEAD LOAD	10 PSF
TOP CHORD DEAD LOAD	10 PSF
UPLIFT LOAD	PER ASCE 7-16
ALL TRUSS TOP CHORDS TO BE 2x6 TYP.	



11 Ninth Street
Suite 120
Columbus, GA 31901
P. (706) 571-6923
F. (706) 571-6928

RFP 2027-3500-01



FIRE STATION MADISON, GA

1100 MARY MAGNAN BLVD
MADISON, GA 30650

BID DOCUMENTS

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NOTES

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