

Stormwater Management Report for

North Oconee Heritage Park

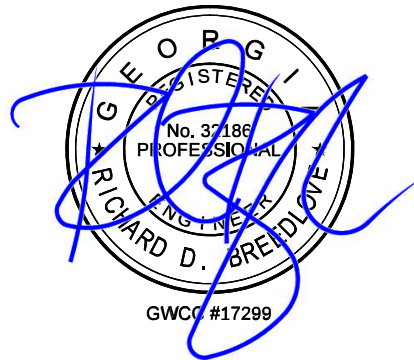
PROJECT LOCATION

0 Cabin Creek Road
Jefferson, Georgia
Jackson County

OWNER/DEVELOPER

Jackson County Government
4965 Jackson Parkway
Jefferson, Georgia 30549

June 11, 2026



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DCD Project #JKS002
Permit Set

TABLE OF CONTENTS

	<u>Page</u>
Introduction	2
Drainage Basins	2
Runoff Reduction and TSS	2
Water Quality	2
Channel Protection	3
Detention	3
10% Study	3
Downstream Analysis	3

APPENDICES

Appendix A: Calculations

- Peak Flow Rate Summary
- Dry Extended Detention Pond 1 Summary
- Stage Storage Pond 1 Summary
- Water Quality Calculations Pond 1
- Channel Protection Calculations Pond 1
- Dry Extended Detention Pond 2 Summary
- Stage Storage Pond 2 Summary
- Water Quality Calculations Pond 2
- Channel Protections Calculations Pond 2
- Drainage Basin Summary
- Drainage Basin Calculations
- GSMM Stormwater Quality Site Development Review Tool
- NOAA Rainfall Chart

Appendix B: Exhibits

- Vicinity Map
- Grading Plan
- 10% Study Basin
- Predeveloped Basins
- Postdeveloped Basins
- Detention Pond Details
- Outlet Control Structure Details

Appendix C: Hydraflow Model Results

Introduction

The purpose of this report is to provide an adequate stormwater management design for the proposed North Oconee Heritage Park, located at 0 Cabin Creek Road. The existing site is 23.25 acres and is heavily wooded. The proposed development will be a public park with active and passive recreational areas such as a multi-use lawn, playground, outdoor classroom, running trails and a dog park. Stormwater management will be provided by two above-ground ponds working in series. The discharge will be released through an outlet control structure and flow into the natural channels along the southwest portion of the property.

All applicable environmental permits (none required) have been acquired for the site.

Drainage Basins

There is an existing ridge onsite that splits the runoff primarily in two directions. The southern portion of the site drains towards the south and southwest. The southernmost basin will be considered Point of Analysis (POA) A. This basin collects 2.07 acres of onsite runoff and 2.63 acres of offsite runoff. The two basins that discharge along the southwest property line are POA B and POA C. POA B collects 6.60 acres of onsite runoff, and POA C collects 1.36 acres of onsite runoff. The remaining northernmost portion of the site drains towards a valley that leaves the site along the western property line. This basin will be considered POA D, and it collects 13.22 acres of onsite runoff and 17.29 acres of offsite runoff. The predeveloped onsite and offsite basins are assumed to have a curve number of 55.

The postdeveloped conditions of the site include 1.35 acres of onsite area and 1.78 acres of offsite area draining to Pond 1 with an overall curve number of 59, respectively. Pond 2 collects 6.53 acres of onsite area with a curve number of 69. Postdeveloped onsite bypass includes 0.77 acres at POA A, 0.94 acres at POA C and 12.95 acres at POA D. There is additional postdeveloped offsite bypass that includes 0.84 acres at POA A and 17.29 acres at POA D. Please refer to the calculations and exhibits for specific drainage basin information.

Runoff Reduction and TSS

The proposed development has an onsite impervious area of 2.39 acres. The required runoff storage volume is 5,329 cf. Due to the significantly large amount of bypass, runoff reduction could not be achieved. Therefore, water quality treatment was based on the removal rate of total suspended solids (TSS). The stormwater management model is designed to handle the removal of the total suspended solids for the first 1.2 inches of rainfall. The GSMM Water Quality Performance tool was used to calculate TSS for the proposed development. The proposed dry extended detention ponds and 12.95 acres of natural conservation area will provide 100% TSS removal, which meets the requirement of 80%. No additional measures are required or proposed. Please refer to the attachments for more information.

Water Quality

Water quality for this development will be provided by two dry extended detention ponds in series. All the necessary water quality, channel protection and detention volumes will be collected in both ponds. Pond 1 will have a 1.5" diameter water quality orifice placed at an invert elevation of 681.50, which is the bottom of the pond. The top of the water quality storage for Pond 1 is at an elevation of 684.00. Pond 2 will have a 2.5" diameter water quality orifice placed at an invert elevation 693.50 at the bottom of the pond. The top of the water quality storage for Pond 2 is at an elevation of 695.50. Two (2) rip-rap berms will be provided, one at each inlet headwall, to create forebays, each with a storage volume of 2,000 cf.

Per the GSMM Water Quality Performance Review Tool, the target water quality storage volume for this site is 6,394 cf. The total storage volume below the water quality orifice that is provided is 11,177 cf.

Channel Protection

Channel protection is provided by the attenuation of the 1-year peak flow rate at each point of analysis. Pond 1 will have a 2.5" diameter channel protection orifice set at an invert elevation of 684.00, which is the top of the proposed water quality storage. The top of the channel protection storage is at 685.00, which is above the 1-year ponding elevation at 684.18. Pond 2 will have a 3.5" diameter channel protection orifice set at an invert elevation of 695.50, which is the top of the proposed water quality storage. The top of the channel protection storage is 697.00, which is above the 1-year ponding elevation at 695.62 for Pond 2.

Detention

Peak flow detention will be met for the 1-, 2-, 5-, 10-, 25-, 50- and 100-year storm events at POA A, POA B and POA C. There is a slight increase in the peak flow rate at POA D due to the small increase in curve number from predeveloped to postdeveloped conditions for the trail. With the large amount of undisturbed area and existing swales that can account for this slight increase in impervious area, this change is negligible. Please refer to the Peak Flow Rate Summary chart for information regarding the 100-year ponding elevations.

10% Study

An analysis of the 10% study basin was not performed for this site. The discharge from the site splits westward and southward and does not converge until approximately 1,360 feet south of the property where the total drainage basin is 84,050 acres. This basin contains many variable conveyance systems and unknown conditions. Given those circumstances, along with the fact that this site makes up less than 1% of the total ten percent basin, the analysis was deemed unnecessary and not performed for this project.

Downstream Analysis

The proposed stormwater management design will reduce the runoff and attenuate the peak flow rates at all points of analysis. No other properties or waters will be adversely affected by the development. No additional measures are required or proposed for this development.

Appendix A: Calculations

PEAK FLOW RATE SUMMARY

Note: Peak flow rates and maximum ponding elevations shown here are taken directly from hydrograph models.

Point of Analysis A

Storm Frequency (yrs)	Existing PFR (cfs)	Postdeveloped PFR (cfs)	PFR Reduction	Pond 1 Max Ponding Elevation (ft)	Pond 2 Max Ponding Elevation (ft)
1	0.94	0.76	18.89%	684.52	695.46
2	2.00	1.22	38.81%	685.02	695.89
5	4.34	2.20	49.25%	685.16	696.60
10	6.80	3.19	53.11%	685.30	697.15
25	10.88	4.88	55.15%	685.71	697.58
50	14.48	9.75	32.69%	686.16	698.07
100	18.50	17.04	7.89%	686.67	698.52

Point of Analysis B

Storm Frequency (yrs)	Predeveloped PFR (cfs)	Postdeveloped PFR (cfs)	PFR Reduction
1	1.25	0.22	82.81%
2	2.64	0.44	83.26%
5	5.84	0.97	83.38%
10	9.23	1.51	83.69%
25	14.70	2.37	83.90%
50	19.51	3.12	84.00%
100	24.88	3.96	84.08%

Point of Analysis C

Storm Frequency (yrs)	Predeveloped PFR (cfs)	Postdeveloped PFR (cfs)	PFR Reduction
1	0.26	0.14	47.67%
2	0.54	0.28	47.79%
5	1.20	0.63	47.84%
10	1.90	0.99	47.79%
25	3.03	1.58	47.79%
50	4.02	2.10	47.80%
100	5.13	2.68	47.80%

Point of Analysis D

Storm Frequency (yrs)	Predeveloped PFR (cfs)	Postdeveloped PFR (cfs)	PFR Reduction
1	4.56	5.53	-21.13%
2	9.53	10.92	-14.56%
5	21.34	23.12	-8.34%
10	33.79	35.86	-6.13%
25	54.68	57.21	-4.63%
50	73.29	76.00	-3.70%
100	94.09	96.95	-3.04%

DRY EXTENDED DETENTION POND SUMMARY

Project name: North Oconee Heritage Park
Pond description: Dry Extended Detention Pond #1
Date: 5/21/2026

Onsite area draining to pond (acres):	1.47	The GSWMM (4.6) recommends a
Onsite impervious area draining to pond (acres):	0.20	maximum depth of 10 feet for dry
Overall area draining to pond (acres):	3.26	ponds and a maximum drainage basin
Overall curve number draining to pond (Cn):	75	of 100 acres. There is no permanent
Shared WQV for channel protection storage (%):	0	pool.

Pond Storage

Stage (ft)	Elevation (ft)	Surface Area (sf)	Incremental Storage (cf)	Cumulative Storage (cf)
0.00	681.50	0	0	0
0.50	682.00	2,257	564	564
2.50	684.00	3,909	6,165	6,730
4.50	686.00	5,787	9,696	16,425
6.50	688.00	7,891	13,678	30,104
7.50	689.00	9,029	8,460	38,564

Water Quality Design

	Required	Proposed	Compliant?
WQ elevation (ft):	682.22	684.00	Yes
WQ volume (cf):	1,104	6,730	Yes
WQ surface area (sf):	640	3,909	Yes
WQ orifice invert elevation (ft):	681.50	681.50	Yes
WQ orifice diameter (in):	1.94	1.50	Yes

Channel Protection Design

CP begin routing elevation (ft):	684.00	684.00	Yes
1-year ponding elevation (ft):	(n/a)	684.18	Yes
CP elevation (ft):	684.18	685.00	Yes
CP storage + water quality volume (cf):	7,467	11,127	Yes
CP orifice invert elevation (ft):	684.00	684.00	Yes
CP orifice diameter (in):	2.76	2.50	Yes

Pond Parameters

Forebay volume (cf):	110	3,000	Yes
100-year ponding elevation (ft):	(n/a)	686.67	Yes
Freeboard (ft):	1.00	2.33	Yes

STAGE STORAGE SUMMARY

Pond description: Dry Extended Detention Pond #1

Stage (ft)	Elevation (ft)	Surface Area (sf)	Storage Volume (cf)	Comments
0.00	681.50	0	0	1.5" WQ orifice at 681.50, bottom of pond
0.10	681.60	451	23	
0.20	681.70	903	90	
0.30	681.80	1,354	203	
0.40	681.90	1,805	361	
0.50	682.00	2,257	564	
0.90	682.40	2,587	1,533	
1.30	682.80	2,918	2,634	
1.70	683.20	3,248	3,867	
2.10	683.60	3,578	5,232	
2.50	684.00	3,909	6,730	2.5" CP orifice at 684.00
2.90	684.40	4,284	8,368	
3.30	684.80	4,660	10,157	
3.70	685.20	5,036	12,096	2' detention weir at 685.00
4.10	685.60	5,411	14,186	
4.50	686.00	5,787	16,425	
4.90	686.40	6,208	18,824	
5.30	686.80	6,629	21,392	100-yr ponding at 686.67
5.70	687.20	7,050	24,127	Emergency overflow weir at 687.00
6.10	687.60	7,471	27,031	
6.50	688.00	7,891	30,104	
6.70	688.20	8,119	31,705	
6.90	688.40	8,346	33,351	
7.10	688.60	8,574	35,043	
7.30	688.80	8,801	36,781	
7.50	689.00	9,029	38,564	Top of pond at 689.00

WATER QUALITY CALCULATIONS

Pond description: Dry Extended Detention Pond #1

Water Quality Storage Volume (GSWMM 2.2.4.1)

Onsite impervious area draining to pond (Ai):	0.20 acres
Onsite area draining to pond (At):	1.47 acres
Volumetric runoff coefficient (Rv):	0.172
$R_v = 0.050 + (0.009) (A_i / A_t)$	
Required water quality volume (WQv):	1,104 cf
$WQ_v = (1.2/12) (R_v) (A_t)$	
Required forebay volume (FBv):	110 cf
$FB_v = (WQ_v) (10\%)$	
Required water quality surface area (Aw):	640 sf
$A_w = (1\%) (A_t)$	

Water Quality Orifice Sizing (using maximum head and maximum discharge method, GSWMM 3.4.3.2)

Proposed WQ orifice invert elevation:	681.50 ft
Proposed WQ storage elevation:	684.00 ft
Drawdown duration:	24 hours
Discharge coefficient (C):	0.6
Gravitational acceleration (g):	32.2 ft/s/s
Maximum orifice flow discharge (Qmax):	0.156 cfs
$Q_{max} = (2) (ED \text{ storage} / \text{drawdown duration})$	
Maximum hydraulic head (H):	2.50 ft
$H = \text{WQ storage elev} - \text{WQ orifice ie}$	
Maximum orifice cross-sectional area (A):	0.0205 sf
$A = Q_{max} / [C \sqrt{2gH}]$	
Maximum orifice diameter (D):	1.94 in
$D = \sqrt{4A / \pi}$	

**Alternatively, when using the average head and average discharge method (GSWMM 3.4.3.3), the maximum orifice diameter (D) becomes 1.63 inches.*

Water Quality Summary

	<u>Required</u>	<u>Proposed</u>	<u>Compliant?</u>
WQ elevation:	682.22 ft	684.00 ft	Yes
WQ volume:	1,104 cf	6,730 cf	Yes
WQ surface area:	640 sf	3,909 sf	Yes
WQ orifice invert elevation:	681.50 ft	681.50 ft	Yes
WQ forebay storage volume:	110 cf	3,000 cf	Yes
WQ orifice diameter:	1.94 in	1.50 in	Yes

CHANNEL PROTECTION CALCULATIONS

Pond description: Dry Extended Detention Pond #1

Assumed Channel Protection Storage Volume (for orifice sizing only)

Overall area draining to pond (At):	3.26 acres
Overall curve number for drainage basin (Cn):	75
Accumulated rainfall for a 1-yr storm event (P):	3.36 in
Potential maximum soil retention (S):	3.33 in
$S = 1000/Cn - 10$	
Accumulated direct runoff (Qa):	1.20 in
$Qa = (P - 0.2S)^2 / (P + 0.8S)$	
Required channel protection volume (CPv):	14,244 cf*
$CPv = (Qa) (At)$	

**Actual channel protection storage volume may be based on 1-year routed storm results.*

Channel Protection Orifice Sizing (using maximum head and maximum discharge method, GSWMM 3.4.3.2)

Proposed WQ elevation:	684.00 ft
Shared percentage of WQ volume for CP storage:	0 %
Required WQ volume:	1,104 cf
Shared WQ volume:	0 cf
Additional CP volume needed:	14,244 cf
Proposed WQ volume:	6,730 cf
Combined volume needed for WQ and CP:	20,973 cf
Combined elevation needed for WQ and CP:	686.73 ft
Drawdown duration:	24 hours
Discharge coefficient (C):	0.6
Gravitational acceleration (g):	32.2 ft/s/s
Maximum orifice flow discharge (Qmax):	0.330 cfs
$Qmax = (2) (CPv \text{ above orifice} / \text{drawdown})$	
Maximum hydraulic head (H):	2.73 ft
$H = \text{CP storage elev} - \text{orifice invert elev}$	
Maximum orifice cross-sectional area (A):	0.0414 sf
$A = Qmax / [C \sqrt{2gH}]$	
Maximum orifice diameter (D):	2.76 in**
$D = \sqrt{4A / \pi}$	

***Alternatively, when using the average head and average discharge method (GSWMM 3.4.3.3), the maximum orifice diameter (D) becomes 2.32 inches.*

Channel Protection Summary

	<u>Required</u>	<u>Proposed</u>	<u>Compliant?</u>
CP begin routing elevation:	684.00 ft	684.00 ft	Yes
CP max elevation based on routed 1-yr ponding:	(n/a) ft	684.18 ft	Yes
CP actual elevation:	684.18 ft	685.00 ft	Yes
CP storage + water quality volume:	7,467 cf	11,127 cf	Yes
CP orifice invert elevation:	684.00 ft	684.00 ft	Yes
CP orifice diameter:	2.76 in	2.50 in	Yes

DRY EXTENDED DETENTION POND SUMMARY

Project name: North Oconee Heritage Park
Pond description: Dry Extended Detention Pond #2
Date: 5/21/2026

Onsite area draining to pond (acres):	6.69	<i>The GSWMM (4.6) recommends a maximum depth of 10 feet for dry ponds and a maximum drainage basin of 100 acres. There is no permanent pool.</i>
Onsite impervious area draining to pond (acres):	1.47	
Overall area draining to pond (acres):	6.69	
Overall curve number draining to pond (Cn):	75	
Shared WQV for channel protection storage (%):	0	

Pond Storage

Stage (ft)	Elevation (ft)	Surface Area (sf)	Incremental Storage (cf)	Cumulative Storage (cf)
0.00	693.50	0	0	0
0.50	694.00	5,124	1,281	1,281
2.50	696.00	7,983	13,107	14,388
4.50	698.00	11,068	19,050	33,439
6.50	700.00	14,378	25,446	58,885

Water Quality Design

	<u>Required</u>	<u>Proposed</u>	<u>Compliant?</u>
WQ elevation (ft):	695.01	695.50	Yes
WQ volume (cf):	7,220	10,597	Yes
WQ surface area (sf):	2,914	7,268	Yes
WQ orifice invert elevation (ft):	693.50	693.50	Yes
WQ orifice diameter (in):	2.57	2.50	Yes

Channel Protection Design

CP begin routing elevation (ft):	695.50	695.50	Yes
1-year ponding elevation (ft):	(n/a)	695.62	Yes
CP elevation (ft):	695.62	697.00	Yes
CP storage + water quality volume (cf):	11,464	23,173	Yes
CP orifice invert elevation (ft):	695.50	695.50	Yes
CP orifice diameter (in):	3.84	3.50	Yes

Pond Parameters

Forebay volume (cf):	722	3,000	Yes
100-year ponding elevation (ft):	(n/a)	698.52	Yes
Freeboard (ft):	1.00	1.48	Yes

STAGE STORAGE SUMMARY

Pond description: Dry Extended Detention Pond #2

Stage (ft)	Elevation (ft)	Surface Area (sf)	Storage Volume (cf)	Comments
0.00	693.50	0	0	2.5" WQ orifice at 693.50, bottom of pond
0.10	693.60	1,025	51	
0.20	693.70	2,050	205	
0.30	693.80	3,075	461	
0.40	693.90	4,100	820	
0.50	694.00	5,124	1,281	
0.90	694.40	5,696	3,445	
1.30	694.80	6,268	5,838	
1.70	695.20	6,840	8,459	
2.10	695.60	7,411	11,310	3.5" CP orifice at 695.50
2.50	696.00	7,983	14,388	
2.90	696.40	8,600	17,705	
3.30	696.80	9,217	21,268	
3.70	697.20	9,834	25,078	2' detentinon weir at 697.00
4.10	697.60	10,451	29,135	
4.50	698.00	11,068	33,439	
4.90	698.40	11,730	37,998	Emergency overflow weir at 698.50
5.30	698.80	12,392	42,823	100-yr ponding at 698.52
5.70	699.20	13,054	47,912	
6.10	699.60	13,716	53,266	
6.50	700.00	14,378	58,885	Top of pond at 700.00

WATER QUALITY CALCULATIONS

Pond description: Dry Extended Detention Pond #2

Water Quality Storage Volume (GSWMM 2.2.4.1)

Onsite impervious area draining to pond (Ai):	1.47 acres
Onsite area draining to pond (At):	6.69 acres
Volumetric runoff coefficient (Rv):	0.248
$R_v = 0.050 + (0.009) (A_i / A_t)$	
Required water quality volume (WQv):	7,220 cf
$WQ_v = (1.2/12) (R_v) (A_t)$	
Required forebay volume (FBv):	722 cf
$FB_v = (WQ_v) (10\%)$	
Required water quality surface area (Aw):	2,914 sf
$A_w = (1\%) (A_t)$	

Water Quality Orifice Sizing (using maximum head and maximum discharge method, GSWMM 3.4.3.2)

Proposed WQ orifice invert elevation:	693.50 ft
Proposed WQ storage elevation:	695.50 ft
Drawdown duration:	24 hours
Discharge coefficient (C):	0.6
Gravitational acceleration (g):	32.2 ft/s/s
Maximum orifice flow discharge (Qmax):	0.245 cfs
$Q_{max} = (2) (ED \text{ storage} / \text{drawdown duration})$	
Maximum hydraulic head (H):	2.00 ft
$H = \text{WQ storage elev} - \text{WQ orifice ie}$	
Maximum orifice cross-sectional area (A):	0.0360 sf
$A = Q_{max} / [C \sqrt{2gH}]$	
Maximum orifice diameter (D):	2.57 in
$D = \sqrt{4A / \pi}$	

**Alternatively, when using the average head and average discharge method (GSWMM 3.4.3.3), the maximum orifice diameter (D) becomes 2.16 inches.*

Water Quality Summary

	<u>Required</u>	<u>Proposed</u>	<u>Compliant?</u>
WQ elevation:	695.01 ft	695.50 ft	Yes
WQ volume:	7,220 cf	10,597 cf	Yes
WQ surface area:	2,914 sf	7,268 sf	Yes
WQ orifice invert elevation:	693.50 ft	693.50 ft	Yes
WQ forebay storage volume:	722 cf	3,000 cf	Yes
WQ orifice diameter:	2.57 in	2.50 in	Yes

CHANNEL PROTECTION CALCULATIONS

Pond description: Dry Extended Detention Pond #2

Assumed Channel Protection Storage Volume (for orifice sizing only)

Overall area draining to pond (At):	6.69 acres
Overall curve number for drainage basin (Cn):	75
Accumulated rainfall for a 1-yr storm event (P):	3.36 in
Potential maximum soil retention (S):	3.33 in
$S = 1000/Cn - 10$	
Accumulated direct runoff (Qa):	1.20 in
$Qa = (P - 0.2S)^2 / (P + 0.8S)$	
Required channel protection volume (CPv):	29,230 cf*
$CPv = (Qa) (At)$	

**Actual channel protection storage volume may be based on 1-year routed storm results.*

Channel Protection Orifice Sizing (using maximum head and maximum discharge method, GSWMM 3.4.3.2)

Proposed WQ elevation:	695.50 ft
Shared percentage of WQ volume for CP storage:	0 %
Required WQ volume:	7,220 cf
Shared WQ volume:	0 cf
Additional CP volume needed:	29,230 cf
Proposed WQ volume:	10,597 cf
Combined volume needed for WQ and CP:	39,828 cf
Combined elevation needed for WQ and CP:	698.55 ft
Drawdown duration:	24 hours
Discharge coefficient (C):	0.6
Gravitational acceleration (g):	32.2 ft/s/s
Maximum orifice flow discharge (Qmax):	0.677 cfs
$Qmax = (2) (CPv \text{ above orifice} / \text{drawdown})$	
Maximum hydraulic head (H):	3.05 ft
$H = \text{CP storage elev} - \text{orifice invert elev}$	
Maximum orifice cross-sectional area (A):	0.0804 sf
$A = Qmax / [C \sqrt{2gH}]$	
Maximum orifice diameter (D):	3.84 in**
$D = \sqrt{4A / \pi}$	

***Alternatively, when using the average head and average discharge method (GSWMM 3.4.3.3), the maximum orifice diameter (D) becomes 3.23 inches.*

Channel Protection Summary

	<u>Required</u>	<u>Proposed</u>	<u>Compliant?</u>
CP begin routing elevation:	695.50 ft	695.50 ft	Yes
CP max elevation based on routed 1-yr ponding:	(n/a) ft	695.62 ft	Yes
CP actual elevation:	695.62 ft	697.00 ft	Yes
CP storage + water quality volume:	11,464 cf	23,173 cf	Yes
CP orifice invert elevation:	695.50 ft	695.50 ft	Yes
CP orifice diameter:	3.84 in	3.50 in	Yes

DRAINAGE BASIN SUMMARY

Individual Drainage Basins

Basin	Description	Area (ac)	Curve Number	Time of Concentration (min)*
1	Predeveloped Onsite to POA A	2.07	55	9
2	Predeveloped Offsite to POA A	2.63	55	12
3	Predeveloped Onsite to POA B	6.60	55	11
4	Predeveloped Onsite to POA C	1.36	55	11
5	Predeveloped Onsite to POA D	13.22	55	15
6	Predeveloped Offsite to POA D	17.29	55	17
7	Postdeveloped Onsite to Pond 1	1.35	65	7
8	Postdeveloped Offsite to Pond 1	1.78	55	12
9	Postdeveloped Onsite to Pond 2	6.53	69	5
10	Postdeveloped Onsite Bypass to POA A	0.77	63	8
11	Postdeveloped Offsite Bypass to POA A	0.84	55	11
12	Postdeveloped Onsite Bypass to POA B	0.94	55	8
13	Postdeveloped Onsite Bypass to POA C	0.71	55	11
14	Postdeveloped Onsite Bypass to POA D	12.95	57	15
15	Postdeveloped Offsite Bypass to POA D	17.29	55	17

* The actual times of concentration are rounded up to a minimum of 5 minutes for modeling purposes.

Combined Drainage Basins

Description	Combined Acreage	Overall Cn	Overall Tc (min)	Contributing Basins (mark with "X"):														
				1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
Predeveloped Onsite	23.25	55	15	X		X	X	X										
Postdeveloped Onsite	23.25	61	15							X		X	X		X	X	X	
Total to Pond 1	3.13	59	12							X	X							
Total to Pond 2	6.53	69	3									X						

DRAINAGE BASIN CALCULATIONS

Reference Table: Surface Types for Times of Concentration Calculations

A - Sheet flow over pavement or hard soil (n=0.01)	F - Shallow flow in paved channel (n=0.01)
B - Sheet flow over cultivated or soft soil (n=0.06)	G - Shallow flow in unpaved channel (n=0.03)
C - Sheet flow over pasture or lawns (n=0.15)	H - Open flow in ephemeral waterway (n=0.06)
D - Sheet flow over light underbrush (n=0.40)	I - Open flow in perennial waterway (n=0.04)
E - Sheet flow over dense underbrush (n=0.80)	J - Open flow in pipe or culvert (n=0.01)

User Input: Basin Description, Curve Numbers, Times of Concentration

Basin #1 Description: [Predeveloped Onsite to POA A](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	2.07	1	50	3	D	6.0	0.12	435	7.2
Landscaped	61		2	440	33	G	7.5	4.42	100	1.7
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	2.07		490	36				534	8.9

Basin #2 Description: [Predeveloped Offsite to POA A](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	2.63	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	835	46	G	5.5	3.79	221	3.7
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	2.63		885	48				732	12.2

Basin #3 Description: [Predeveloped Onsite to POA B](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	6.60	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	644	35	G	5.4	3.76	171	2.9
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	6.60		694	37				682	11.4

Basin #4 Description: [Predeveloped Onsite to POA C](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	1.36	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	362	12	G	3.3	2.94	123	2.1
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	1.36		412	14				634	10.6

Basin #5 Description: [Predeveloped Onsite to POA D](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	13.22	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	1,094	32	G	2.9	2.76	397	6.6
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	13.22		1,144	34				908	15.1

Basin #6 Description: [Predeveloped Offsite to POA D](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	17.29	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	1,657	70	G	4.2	3.32	500	8.3
Wooded Upland	55		3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	17.29		1,707	72				1,011	16.8

Basin #7 Description: [Postdeveloped Onsite to Pond 1](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	0.16	1	50	4	D	8.0	0.13	387	6.5
Landscaped	61	1.19	2	109	9	G	8.2	4.63	24	0.4
Wooded Upland	55		3	89	3.5	F	3.9	4.03	22	0.4
(Other, if needed)	0		4	76	11.5	J	15.2	45.68	2	0.0
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	65	1.35		324	28				435	7.2

Basin #8 Description: [Postdeveloped Offsite to Pond 1](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	1.78	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	600	31	G	5.2	3.67	164	2.7
Wooded Upland	55		3	81	3	F	3.7	3.91	21	0.3
(Other, if needed)	0		4	173	8	J	4.6	25.23	7	0.1
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	1.78		904	44				702	11.7

Basin #9 Description: [Postdeveloped Onsite to Pond 2](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	1.48	1	50	6	C	12.0	0.33	150	2.5
Landscaped	61	5.05	2	26	3	G	11.5	5.48	5	0.1
Wooded Upland	55		3	108	3	F	2.8	3.39	32	0.5
(Other, if needed)	0		4	443	20	J	4.5	24.93	18	0.3
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	69	6.53		627	32				205	3.4

Basin #10 Description: [Postdeveloped Onsite Bypass to POA A](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	0.04	1	50	4	D	8.0	0.13	387	6.5
Landscaped	61	0.73	2	104	9	G	8.7	4.75	22	0.4
Wooded Upland	55		3	92	2.5	F	2.7	3.35	27	0.5
(Other, if needed)	0		4	100	6.5	G	6.5	4.11	24	0.4
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	63	0.77		346	22				461	7.7

Basin #11 Description: [Postdeveloped Offsite Bypass to POA A](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	0.84	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	387	31.5	G	8.1	4.60	84	1.4
Wooded Upland	55		3	73	2	F	2.7	3.36	22	0.4
(Other, if needed)	0		4	100	6.5	G	6.5	4.11	24	0.4
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	0.84		611	42				641	10.7

Basin #12 Description: [Postdeveloped Onsite Bypass to POA B](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98		1	50	1	C	2.0	0.16	308	5.1
Landscaped	61		2	571	21.5	G	3.8	3.13	182	3.0
Wooded Upland	55	0.94	3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	0.94		621	22.5				490	8.2

Basin #13 Description: [Postdeveloped Onsite Bypass to POA C](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98		1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	362	12	G	3.3	2.94	123	2.1
Wooded Upland	55	0.71	3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	0.71		412	14				634	10.6

Basin #14 Description: [Postdeveloped Onsite Bypass to POA D](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98	0.71	1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	1,094	32	G	2.9	2.76	397	6.6
Wooded Upland	55	12.24	3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	57	12.95		1,144	34				908	15.1

Basin #15 Description: [Postdeveloped Offsite Bypass to POA D](#)

Ground Cover	Cn	Area (ac)	Segment Leg	Length (ft)	Δ Elev (ft)	Surface Type	Slope (%)	Velocity (ft/s)	Tc (sec)	Tc (min)
Impervious	98		1	50	2	D	4.0	0.10	511	8.5
Landscaped	61		2	1,657	70	G	4.2	3.32	500	8.3
Wooded Upland	55	17.29	3							
(Other, if needed)	0		4							
Commercial District	92		5							
Residential District	72		6							
Agriculture District	65		7							
Basin Total	55	17.29		1,707	72				1,011	16.8

Georgia Stormwater Management Manual

Stormwater Quality Site Development Review Tool

Version 2.2

General Information

Name of Developer:	Jackson County Government	Date Submitted:	5/21/2026
Development Name:	North Oconee Heritage Park	Permit Number:	
Site Location / Address:	0 Cabin Creek Road	Developer Contact:	Barry Vickery
	Jefferson, GA 30549	Phone Number:	(706) 286-6353
		Name of Engineer(s):	Dovetail Civil Design, Inc.
Development Type:	Parks, Recreation & Conservation Areas	Maintenance Responsibility:	Owner

Site Summary

Total Pre-Development Area (ac): 23.25
 Total Post-Development Area (ac): 23.25
 Total Treated Area (ac): 8.16
 Total Untreated Area (ac): 15.09

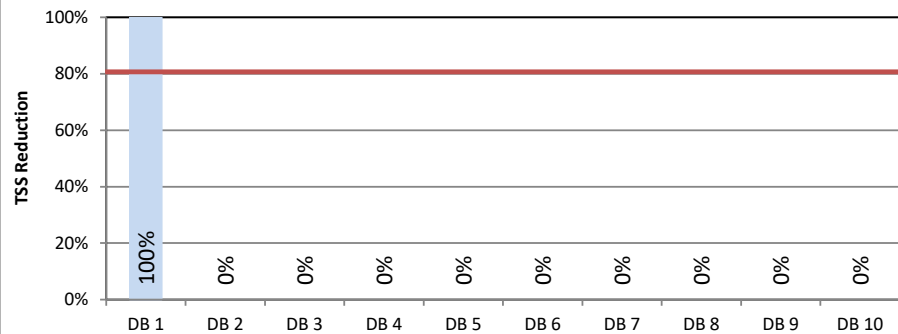
		I (ac)	P (ac)	CA (ac)
Total Onsite	DB 1	2.39	7.91	12.95
Drainage Basin 2	DB 2	0.00	0.00	0.00
Drainage Basin 3	DB 3	0.00	0.00	0.00
Drainage Basin 4	DB 4	0.00	0.00	0.00
Drainage Basin 5	DB 5	0.00	0.00	0.00
Drainage Basin 6	DB 6	0.00	0.00	0.00
Drainage Basin 7	DB 7	0.00	0.00	0.00
Drainage Basin 8	DB 8	0.00	0.00	0.00
Drainage Basin 9	DB 9	0.00	0.00	0.00
Drainage Basin 10	DB 10	0.00	0.00	0.00
TOTAL		2.39	7.91	12.95

I = Impervious Area, P = Pervious Area, CA = Conservation Area

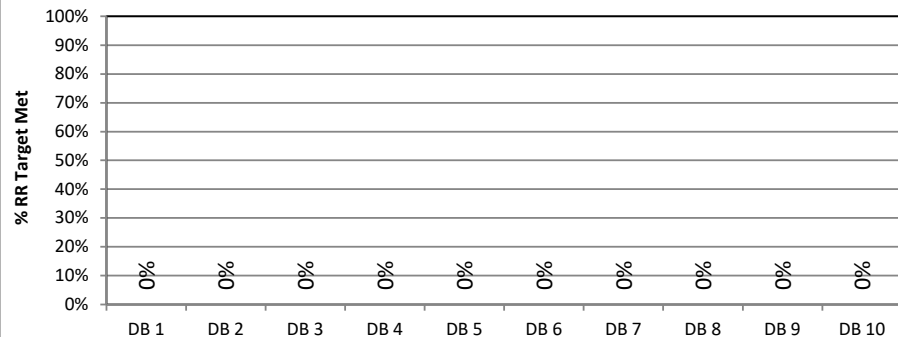
Target Runoff Reduction Volume Achieved? **No**
 Target TSS Removal Achieved? **Yes**

Total Target Runoff Reduction Volume (cf) 6,394
 Runoff Reduction Volume Achieved (cf) 0
 Total Target Water Quality Volume (cf) 6,394
 % TSS Removal Achieved 100%

Total Suspended Solids (TSS) Removal



Runoff Reduction (RR)



Official Use Only

Tracking #: _____
 Reviewed By: _____
 Date Approved: _____

Conditions of Approval:

A RECORDED CONSERVATION EASEMENT OR SIMILAR FORM OF PROTECTION IS REQUIRED FOR THIS PROJECT

Georgia Stormwater Management Manual

Stormwater Quality Site Development Review Tool, v2.2

Development Name: **North Oconee Heritage Park**
 Drainage Basin Name: **Total Onsite**

data input cells
 calculation cells
 constant values

Site Data

Indicate Pre-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed Area

Cover Type	HSG* A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Impervious		98		98		98		98	0.00	0%
Open space - Good condition (grass cover > 75%)		39		61		74		80	0.00	0%
Woods - Good Condition		30	23.25	55		70		77	23.25	100%
Select a land cover type...		0		0		0		0	0.00	0%
Select a land cover type...		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other									0.00	0%
Total	0.00		23.25		0.00		0.00		23.25	100%

*HSG = hydrologic soil group

Impervious (ac) 0.00
 Weighted CN 55
 Potential Max Soil Retention, S_{pre} (in) 8.18

Indicate Post-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed Area

Cover Type	HSG A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Impervious		98	2.39	98		98		98	2.39	10%
Pasture, grassland, or range - continuous forage for grazing - Good Condition		39	6.97	61		74		80	6.97	30%
Woods - Good Condition		30	13.89	55		70		77	13.89	60%
Select a land cover type...		0		0		0		0	0.00	0%
Select a land cover type...		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other									0.00	0%
Total	0.00		23.25		0.00		0.00		23.25	100%

Impervious (ac) 2.39
 Rv 0.14
 Weighted CN 61
 Potential Max Soil Retention, S_{post} (in) 6.33

Conservation Area Credits

Scenario 1: Natural Conservation Area *See the GSMM Volume 2, Section 2.3.3.3 for more information.

☒ Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection.

12.95 Area (ac) of development protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 1 box above is checked

Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information.

☐ Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection.

Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 2 box above is checked

Scenario 3: Soil Restoration *See the GSMM Volume 2, Section 4.23 for more information.

☐ Check the box if a portion of the post-developed area employs soil restoration and is protected by a conservation easement or equivalent form of protection.

Area (ac) of development with restored soils and protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 3 box above is checked

Scenario 4: Site Reforestation/Revegetation & Soil Restoration

**See the GSMM Volume 2, Section 4.22 and 4.23 for more information.*

☐ Check the box if the same portion of the post-developed area employs site reforestation/revegetation and soil restoration, and is protected by a conservation easement or equivalent form of protection.

Area (ac) with restored soils in a reforested & revegetated area and protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 4 box above is checked

Total Conservation Area Credit (acres) 12.95

Georgia Stormwater Management Manual

Stormwater Quality Site Development Review Tool, v2.2

Development Name: **North Oconee Heritage Park**
 Drainage Basin Name: **Total Onsite**

data input cells
 calculation cells
 constant values

Water Quality Goals

Target Runoff Reduction Storm (in) **1.20**

Total Site Area for Water Quality Volume (acres) **10.30**
 Target Runoff Reduction Volume (cf) **6,394**
 Target Water Quality Volume (cf) **6,394**

Select BMPs for Runoff Reduction and Water Quality

		Area Draining to Each BMP			Storage Volume Provided by BMP (cf)	RR Conveyance Volume Provided by BMP (cf)	Down-stream BMP	Runoff Reduction Calculations						WQ Calculations	
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)				RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runoff Reduction %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ, from Direct Drainage (cf)	Effective TSS Removal %
BMP 1	Dry Extended Detention Basin	1.27	0.20	1.79	6,730			1,104	7,220	8,324	0%	0	8,324	1,104	60%
BMP 2	Dry Extended Detention Basin	5.22	1.47		10,597		BMP 1	7,220	0	7,220	0%	0	7,220	7,220	60%
BMP 3	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 4	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 5	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 6	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 7	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 8	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 9	Select a BMP...							0	0	0	N/A	0	0	0	N/A
BMP 10	Select a BMP...							0	0	0	N/A	0	0	0	N/A
TOTAL		6.49	1.67	1.79				8,324				0		8,324	
UNTREATED AREA (acres)		14.37	0.72												

Target Runoff Reduction Volume (cf)	6,394
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	6,394

Target Water Quality Volume (cf)	6,394
% TSS Removal Achieved	100%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

Channel and Flood Protection Calculations

1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
3.33	3.78	6.22	7.84

Target Rainfall Event (in)

1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
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Georgia Stormwater Management Manual

Stormwater Quality Site Development Review Tool, v2.2

Development Name: North Oconee Heritage Park
Drainage Basin Name: Total Onsite

data input cells
calculation cells
constant values

Pre-Development Runoff Volume (in)	0.29	0.45	1.65	2.68
Post Development Runoff Volume (in) with no BMPs	0.51	0.71	2.17	3.35
Post-Development Runoff Volume (in) with BMPs	0.51	0.71	2.17	3.35
Adjusted CN	61	61	61	61

*See Stormwater Management Standards to Determine Detention Requirements.

Comments

A RECORDED CONSERVATION EASEMENT OR SIMILAR FORM OF PROTECTION IS REQUIRED FOR THIS PROJECT



NOAA Atlas 14, Volume 9, Version 2
Location name: Nicholson, Georgia, USA*
Latitude: 34.1179°, Longitude: -83.4715°
Elevation: m/ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffrey Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.418 (0.335-0.512)	0.476 (0.382-0.583)	0.570 (0.456-0.700)	0.648 (0.517-0.797)	0.756 (0.591-0.942)	0.838 (0.647-1.05)	0.920 (0.697-1.17)	1.00 (0.741-1.29)	1.11 (0.804-1.45)	1.19 (0.851-1.57)
10-min	0.612 (0.491-0.750)	0.697 (0.559-0.854)	0.835 (0.668-1.02)	0.949 (0.757-1.17)	1.11 (0.865-1.38)	1.23 (0.947-1.54)	1.35 (1.02-1.71)	1.47 (1.09-1.89)	1.63 (1.18-2.13)	1.75 (1.25-2.30)
15-min	0.746 (0.599-0.914)	0.850 (0.681-1.04)	1.02 (0.815-1.25)	1.16 (0.924-1.42)	1.35 (1.06-1.68)	1.50 (1.16-1.88)	1.64 (1.24-2.09)	1.79 (1.32-2.30)	1.98 (1.44-2.59)	2.13 (1.52-2.81)
30-min	1.04 (0.838-1.28)	1.20 (0.959-1.47)	1.44 (1.15-1.77)	1.64 (1.31-2.02)	1.91 (1.49-2.38)	2.12 (1.63-2.66)	2.32 (1.76-2.94)	2.52 (1.86-3.24)	2.79 (2.01-3.63)	2.98 (2.12-3.93)
60-min	1.34 (1.08-1.64)	1.53 (1.23-1.88)	1.86 (1.49-2.28)	2.13 (1.70-2.62)	2.52 (1.98-3.16)	2.83 (2.19-3.56)	3.14 (2.39-4.01)	3.47 (2.57-4.48)	3.91 (2.84-5.12)	4.26 (3.04-5.61)
2-hr	1.64 (1.33-1.98)	1.87 (1.52-2.27)	2.28 (1.84-2.76)	2.62 (2.12-3.20)	3.13 (2.49-3.90)	3.54 (2.78-4.44)	3.97 (3.05-5.03)	4.42 (3.31-5.68)	5.04 (3.69-6.57)	5.53 (3.98-7.24)
3-hr	1.83 (1.50-2.21)	2.09 (1.71-2.52)	2.54 (2.07-3.07)	2.95 (2.39-3.57)	3.55 (2.85-4.42)	4.05 (3.20-5.06)	4.58 (3.54-5.80)	5.15 (3.89-6.60)	5.95 (4.38-7.74)	6.60 (4.76-8.60)
6-hr	2.24 (1.85-2.68)	2.55 (2.10-3.04)	3.09 (2.54-3.69)	3.58 (2.94-4.30)	4.33 (3.53-5.36)	4.97 (3.97-6.16)	5.64 (4.42-7.10)	6.38 (4.86-8.13)	7.43 (5.52-9.60)	8.28 (6.02-10.7)
12-hr	2.77 (2.31-3.27)	3.13 (2.62-3.70)	3.77 (3.14-4.46)	4.35 (3.61-5.16)	5.21 (4.28-6.36)	5.93 (4.78-7.28)	6.69 (5.28-8.33)	7.52 (5.78-9.49)	8.68 (6.50-11.1)	9.61 (7.05-12.4)
24-hr	3.33 (2.82-3.89)	3.78 (3.20-4.42)	4.56 (3.84-5.34)	5.24 (4.40-6.14)	6.22 (5.14-7.48)	7.01 (5.70-8.50)	7.84 (6.24-9.64)	8.72 (6.75-10.9)	9.94 (7.50-12.6)	10.9 (8.06-13.9)
2-day	3.89 (3.33-4.50)	4.44 (3.80-5.14)	5.36 (4.58-6.22)	6.15 (5.23-7.15)	7.28 (6.07-8.65)	8.17 (6.70-9.79)	9.09 (7.29-11.1)	10.1 (7.84-12.4)	11.4 (8.63-14.3)	12.4 (9.23-15.7)
3-day	4.29 (3.70-4.93)	4.85 (4.18-5.58)	5.80 (4.99-6.68)	6.62 (5.67-7.65)	7.79 (6.55-9.22)	8.73 (7.21-10.4)	9.70 (7.83-11.7)	10.7 (8.41-13.2)	12.1 (9.25-15.2)	13.2 (9.88-16.7)
4-day	4.63 (4.01-5.30)	5.20 (4.50-5.95)	6.16 (5.32-7.07)	7.00 (6.02-8.05)	8.20 (6.93-9.66)	9.17 (7.61-10.9)	10.2 (8.25-12.3)	11.2 (8.84-13.8)	12.7 (9.72-15.8)	13.8 (10.4-17.4)
7-day	5.47 (4.78-6.20)	6.10 (5.33-6.92)	7.16 (6.24-8.13)	8.07 (7.00-9.19)	9.36 (7.97-10.9)	10.4 (8.71-12.2)	11.5 (9.37-13.7)	12.6 (9.99-15.3)	14.1 (10.9-17.5)	15.3 (11.6-19.2)
10-day	6.19 (5.45-6.98)	6.88 (6.05-7.76)	8.03 (7.04-9.07)	9.00 (7.87-10.2)	10.4 (8.88-12.0)	11.5 (9.65-13.4)	12.6 (10.3-15.0)	13.7 (10.9-16.6)	15.3 (11.9-18.9)	16.5 (12.5-20.6)
20-day	8.29 (7.38-9.24)	9.13 (8.13-10.2)	10.5 (9.32-11.7)	11.6 (10.3-13.0)	13.2 (11.4-15.0)	14.4 (12.2-16.6)	15.5 (12.9-18.2)	16.7 (13.4-20.0)	18.3 (14.3-22.3)	19.4 (14.9-24.1)
30-day	10.2 (9.14-11.3)	11.2 (10.0-12.4)	12.7 (11.4-14.1)	14.0 (12.4-15.6)	15.6 (13.5-17.7)	16.9 (14.4-19.3)	18.0 (15.0-21.0)	19.2 (15.5-22.8)	20.7 (16.2-25.1)	21.8 (16.8-26.8)
45-day	12.8 (11.5-14.0)	13.9 (12.6-15.3)	15.7 (14.2-17.4)	17.2 (15.4-19.0)	19.0 (16.5-21.2)	20.2 (17.3-23.0)	21.4 (17.9-24.7)	22.5 (18.2-26.5)	23.8 (18.7-28.7)	24.7 (19.1-30.3)
60-day	15.1 (13.7-16.5)	16.5 (14.9-18.0)	18.5 (16.8-20.3)	20.1 (18.1-22.1)	22.0 (19.2-24.5)	23.3 (20.0-26.2)	24.4 (20.5-28.0)	25.4 (20.7-29.8)	26.5 (20.9-31.8)	27.1 (21.1-33.2)

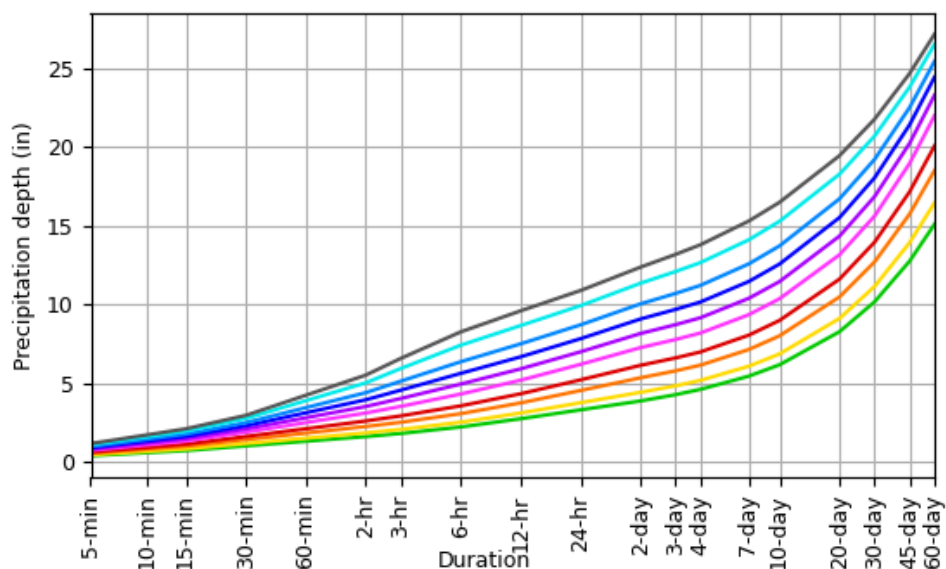
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
 Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
 Please refer to NOAA Atlas 14 document for more information.

[Back to Top](#)

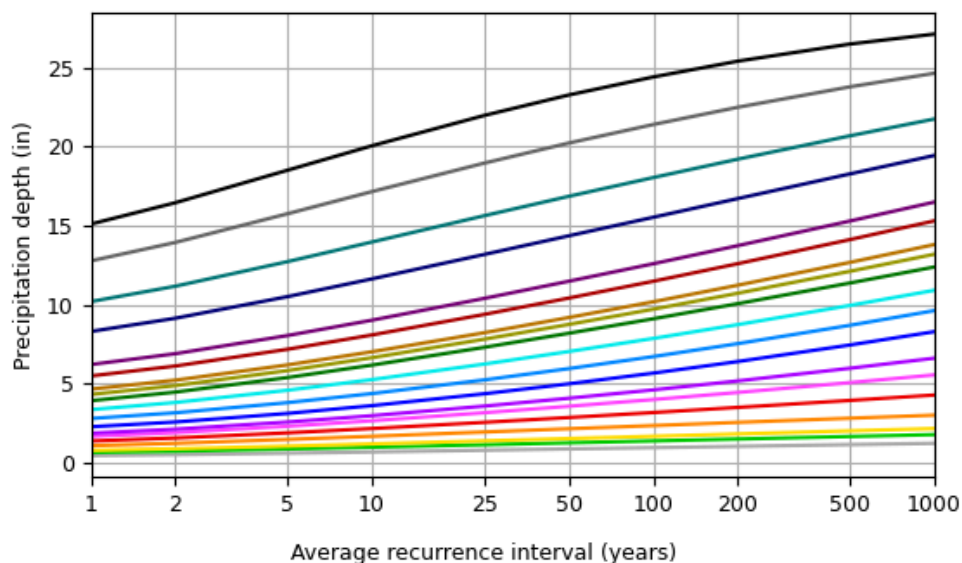
PF graphical

PDS-based depth-duration-frequency (DDF) curves

Latitude: 34.1179°, Longitude: -83.4715°



Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000



Duration	
5-min	2-day
10-min	3-day
15-min	4-day
30-min	7-day
60-min	10-day
2-hr	20-day
3-hr	30-day
6-hr	45-day
12-hr	60-day
24-hr	

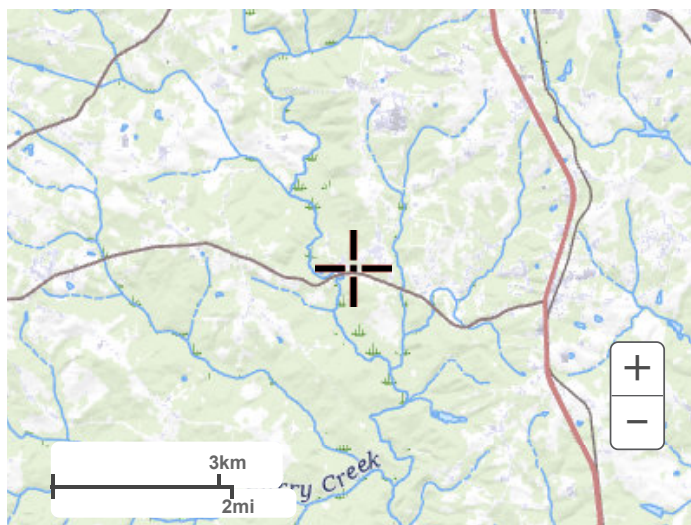
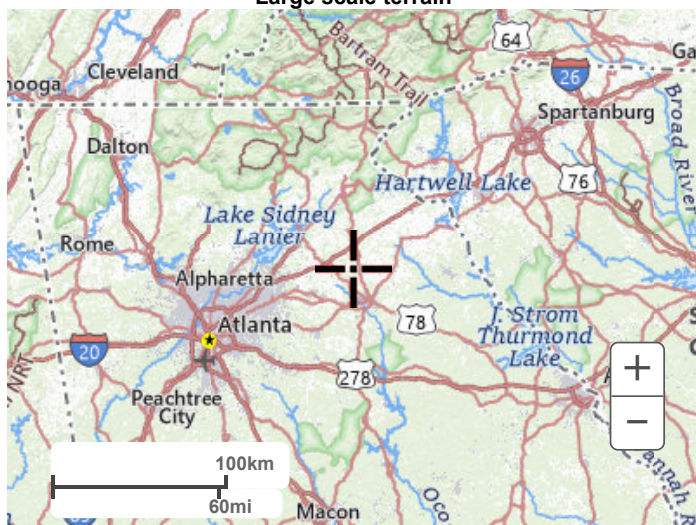
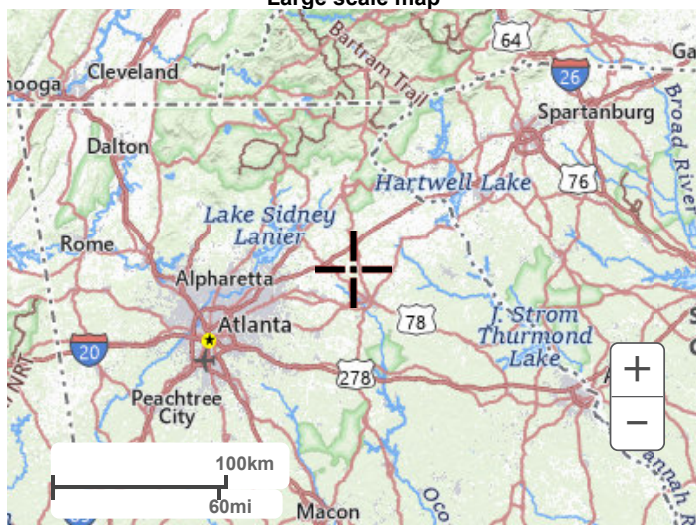
NOAA Atlas 14, Volume 9, Version 2

Created (GMT): Thu Apr 9 15:53:53 2026

[Back to Top](#)

Maps & aerials

Small scale terrain

**Large scale terrain****Large scale map****Large scale aerial**

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1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

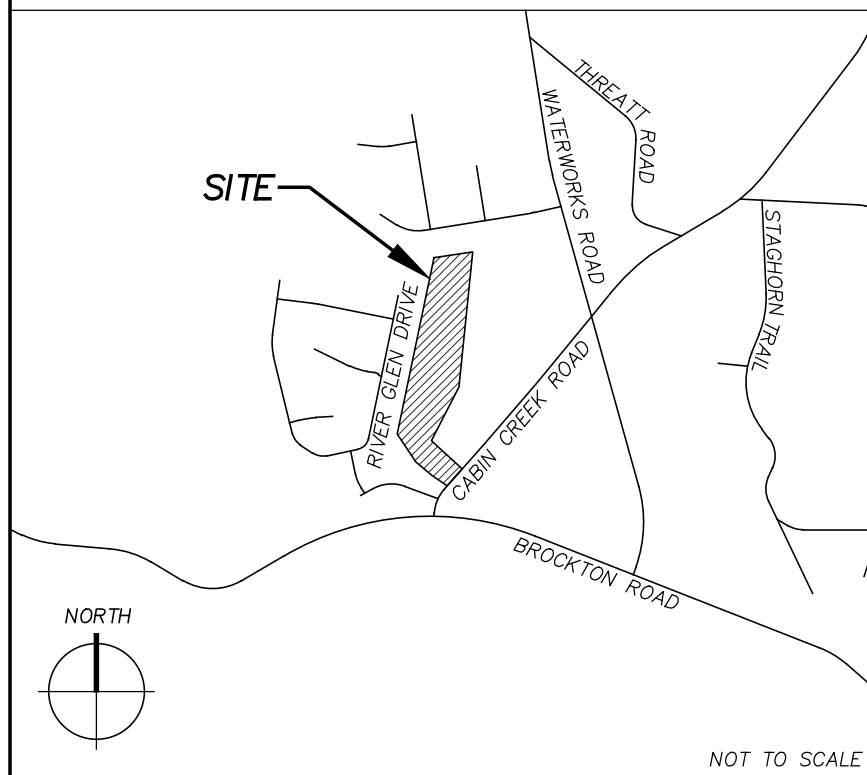
[Disclaimer](#)

Appendix B:

Exhibits

SITE ADDRESS:
0 CABIN CREEK ROAD
JEFFERSON, GA 30549

SITE ENTRANCE COORDINATES:
34.1187° LAT
-83.4707° LON



VICINITY MAP



GRADING ALONG BUILDING PERIMETER

1. Structures such as buildings, slabs, walls, amenities and monuments may require additional provisions for compaction, elevations, utilities and other elements not shown on these drawings. The contractor is responsible for meeting the specifications of the approved architectural, structural and MEP drawings and notifying the engineer if a conflict exists.

2. Unless otherwise noted, finished grades around the building slab shall be a minimum of 6" below the finished floor elevation or as required by the architect or building material specifications. If this condition cannot be met, additional provisions such as waterproofing, foundation walls, trench drains, handscaping and/or building redesign may be required.

3. Unpaved surfaces shall slope away from the building at 5% typical, but no less than 2% and maintain positive drainage at all times.

4. Paved surfaces shall slope away from the building at a minimum of 0.5% and a maximum of 2% unless otherwise noted.

5. The maximum vertical drop or offset at any point along an ADA route shall be 0.25". Door thresholds may be as high as 0.5" if beveled properly. Refer to architectural drawings for specifications and elevations around the building perimeter and at all access points.

JACKSON COUNTY INSPECTIONS

Notify Jackson County Inspector at least 24 hours before beginning any phase of construction. (706) 367-6335

UTILITY CONSTRUCTION NOTE

The installation, relocation or removal of any utility lines or structures must be performed at the supervision and discretion of the appropriate utility department. Instructions and requirements issued by the utility departments shall supersede those shown on these plans if they exceed the recommendations included herein.

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SITE

0 CABIN CREEK ROAD
JEFFERSON, GA 30549

SITE ENTRANCE COORDINATES:
34.1167° LAT
-83.4707° LONG

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VICINITY MAP

NOT TO SCALE

EXISTING FEATURES DISCLAIMER

The existing utilities, topography and other site features shown on these drawings may be comprised of field surveys by others, local GIS data, previous design plans and other sources. The engineer assumes no responsibility for the completeness or accuracy of the existing conditions.

It is the responsibility of the contractor to locate and verify all existing utilities and features prior to construction to ensure proper horizontal and vertical connections and positive drainage as needed. The contractor shall notify the owner and engineer immediately if any discrepancies occur.

FINISHED FLOOR ELEVATIONS

The building floor elevations shown (denoted as FFE, MFE, GAR, BSMT) indicate the minimum elevations allowable based on topography and nearby stormwater ponding. The actual elevations of the finished floor, basement, garage and other areas of the structure shall be based on the proposed building plans by others. It is strongly recommended that the actual elevations either match or exceed those shown on these plans. The architect or contractor shall notify the engineer if conflicts occur.

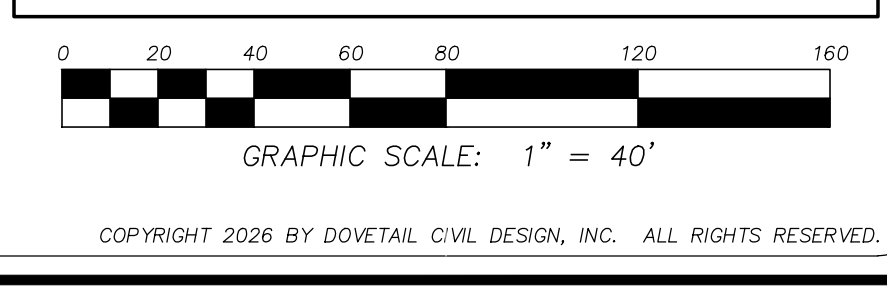
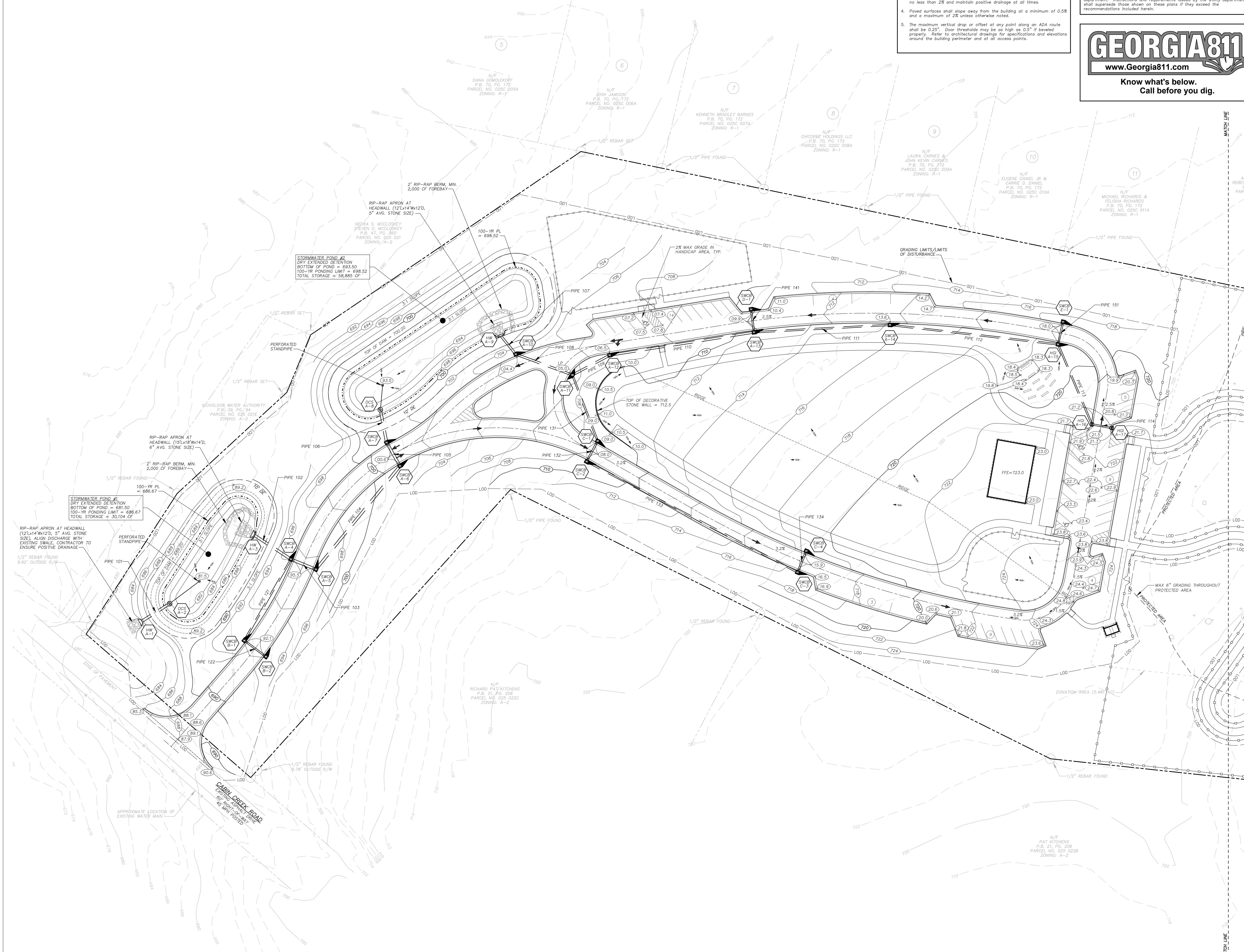
EARTHWORK ESTIMATES

ESTIMATED SURFACE CUT:	15,769 CY
ESTIMATED SURFACE FILL:	16,763 CY
NET BALANCE:	4,008 CY CUT
PROPOSED AREA OF DISTURBANCE:	12.00 AC
VERTICAL DIFFERENTIAL:	0.21 FT

The contractor is responsible for the verification of earthwork quantities and shall not rely on estimates provided by the engineer. The earthwork cut and fill estimates included in these plans are based on computer model comparisons of the existing and proposed surfaces and may not accurately account for unobtainable soils, demolition, piling thickness, compaction, trenching and other significant elements. The contractor may prefer to adjust finished floor elevations or other grades, but must notify the engineer for approval prior to making these field changes.

GENERAL GRADING NOTES

- The contractor shall verify all proposed and existing dimensions, utilities, grading slopes and elevations prior to construction and notify the owner or engineer of any discrepancies.
- Additional land development provisions beyond those shown on the permitted plans may be required during construction due to site conditions, weather or requests by the inspector.
- The contractor shall make a reasonable effort at all times to avoid any excessive disturbance to this property, neighboring properties, buffer areas, utilities and traffic flow. The contractor shall work in a manner that minimizes damage to existing site features and vegetation unless otherwise indicated on these drawings.
- No land disturbing activity shall occur outside of the limits of disturbance and no trees shall be damaged outside of the tree protection fencing as shown on the permitted plans without written consent of the property owner and approved by the governing authorities.
- Clearing and grubbing shall consist of removing all vegetation, organic material, unsuitable soils and debris from the delineated areas as shown on the plans. Upon completion of grubbing, these areas shall be left in a clean, compacted and stabilized condition and shall be leveled to eliminate any erodible holes or ridges of two feet or greater. Voids shall be filled or refilled with acceptable material and properly compacted in layers by tamper, roller or other appropriate construction equipment.
- Topsoil shall be stripped from any area receiving fill dirt or any area which requires compaction such as buildings, utilities, structures, pond dams or paving. Top soil may be temporarily stockpiled for use in landscape areas if cleaned of debris and approved by the owner. Materials and operations shall be evaluated by a geotechnical engineer. Exporting or disposal of soils and other materials shall be performed in accordance with local solid waste regulations and may require separate permitting.
- All areas receiving fill dirt or compaction shall be prepared by stripping and completely removing all vegetation, organic material, unsuitable soils and debris. Filling materials and compaction operations shall be evaluated by a geotechnical engineer.
- Proof rolling is required throughout all exposed areas to evaluate soil conditions and stability. Any soft, yielding, excessively wet, residual or otherwise unsuitable soils discovered during proof rolling or otherwise present during any phase of construction shall be undercut and replaced with acceptable fill material and properly compacted.
- If temporary or permanent groundwater is discovered, the contractor shall be responsible for dewatering the affected areas with trenching, pumps, mechanical pumping and other solutions. Permanent groundwater may require additional trenching and bedding measures or more significant action such as a redesign of the site and grading plans. The contractor shall consult the geotechnical engineer for evaluation of such conditions.
- Fill dirt shall meet the minimum criteria of ASTM D2487. Fill dirt shall not include vegetation, organic material, frozen material, debris, polluted soils or other unsuitable material. Fill dirt shall not include stones larger than 6 inches for utility trenches or the upper 6 inches of surface grading. Structural fill dirt shall not include stones larger than 3 inches. Otherwise, fill dirt shall not include stones larger than 6 inches. Fill dirt may be generated by onsite excavation if it meets the criteria described herein.
- Fill material shall be placed in successive horizontal layers, or lifts, not to exceed 8 inches thick and compacted to at least 88% of the maximum laboratory dry density according to standard practice. Fill slopes steeper than 4:1V shall be stepped or benched. Each lift shall be proof rolled with a vibrating sheepsfoot compactor roller, loaded rubber tired tandem axle dump truck, scraper, loader or other appropriate heavy equipment. Compaction equipment shall make overlapping passes longitudinally and perpendicularly throughout the lift area until minimum criteria are met. Moisture content of each lift shall be within 1% of the optimum moisture content. Watering trucks, spreaders or spray hoses may be used if necessary to bring soils up to the proper moisture range. In such cases, water shall be thoroughly mixed and evenly distributed prior to compaction.
- Compaction, moisture and laboratory testing shall meet the minimum criteria of ASTM D698 and AASHTO T98.
- The contractor shall follow the recommendations of the geotechnical engineer or soils inspection reports if they exceed the specifications included herein.
- If rock is encountered, the owner shall be notified and written instructions shall be provided by the geotechnical engineer prior to excavation.
- The site should be graded during each phase of construction to maintain positive drainage away from buildings, laydown areas and other significant features of the construction site to prevent erosion or stormwater ponding.
- The maximum slope for maintained areas, pond dams and stormwater conveyance channels is 3H:1V. All other slopes may not exceed 2H:1V unless otherwise noted and evaluated by a geotechnical engineer.
- The proposed grading shown on these plans represents the finished surface elevations unless otherwise noted.
- All drainage elements and gravity utility lines shall be sloped in an effective and safe manner away from buildings and toward the appropriate destinations as shown on these plans. If site conditions present a conflict, the contractor shall notify the owner or engineer immediately.
- All grading and paving tie-points shall have a smooth and aesthetic transition between the new construction and existing elements.
- Structures such as buildings, slabs, walls, amenities and other elements not shown on these drawings. The contractor is responsible for meeting the specifications of the approved architectural, structural and MEP drawings and notifying the engineer if a conflict exists.
- Stormwater inlet structures shall have their flow lines adjusted to finished grade or slightly lower to accept positive drainage from surface runoff. Please note that flow lines are not necessarily the same as rim elevations for some structures such as pedestal drop inlets and curb inlets. The contractor shall consult the engineer for clarity if needed.
- All stormwater structures shall be traffic-rated if installed within roads, drives, parking lots and other areas designated for vehicular traffic.
- Stormwater grate inlets shall be ADA compliant if installed within sidewalks, plazas and other areas designated for pedestrian traffic.



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JKS002
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6/11/26

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DATE	DESCRIPTION
4/20/26	DESIGN DEVELOPMENT SET FOR OWNER'S REVIEW
6/11/26	LDP SUBMITTAL TO JACKSON COUNTY

North Oconee Heritage Park

SITE DEVELOPMENT PLANS

SITE INFORMATION
0 CABIN CREEK ROAD
JEFFERSON, GA 30549
UNINCORPORATED JACKSON COUNTY
G.M.D. 253
A-2 ZONING

DEVELOPER INFORMATION
JEFFERSON, GA 30549
PHONE: (706) 286-6353
CONTACT PERSON: BARRY VICKERY

24-HR CONTACT: BARRY VICKERY, (706) 286-6353

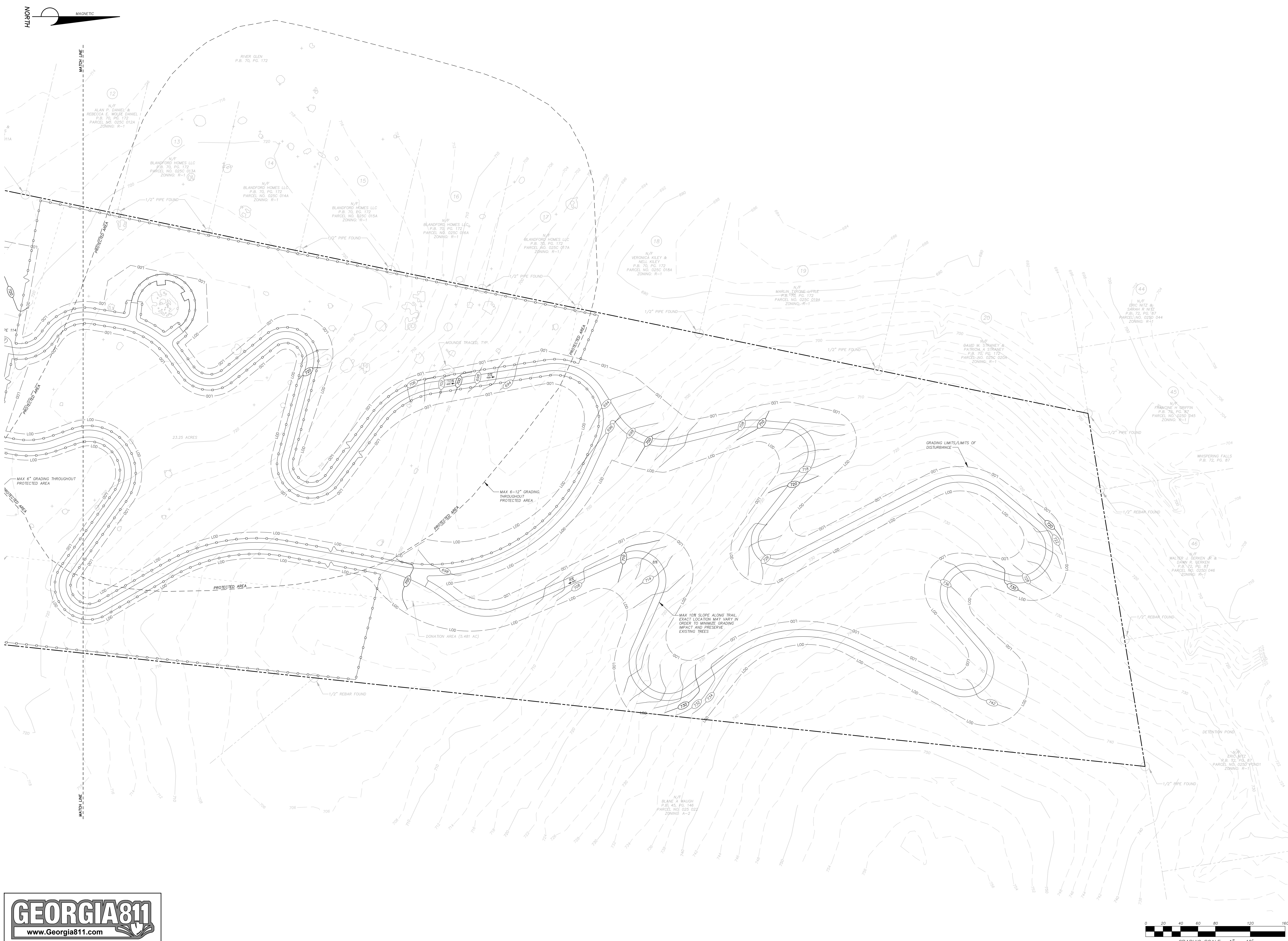
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GRADING AND DRAINAGE PLAN (SOUTH)

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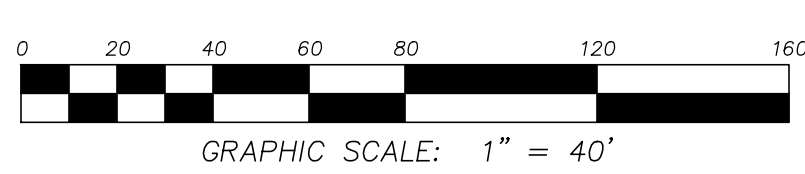
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ISSUE	DATE	DESCRIPTION
1	4/20/26	DESIGN DEVELOPMENT SET FOR OWNER'S REVIEW
2	6/11/26	LDP SUBMITTAL TO JACKSON COUNTY

North Oconee Heritage Park
SITE DEVELOPMENT PLANS

SITE INFORMATION

JACKSON COUNTY, GEORGIA

4800 JEFFERSON ROAD

UNINCORPORATED JACKSON COUNTY

G.M.D. 253

A-2 ZONING

DEVELOPER INFORMATION

JACKSON COUNTY, GEORGIA

4800 JEFFERSON ROAD

UNINCORPORATED JACKSON COUNTY

G.M.D. 253

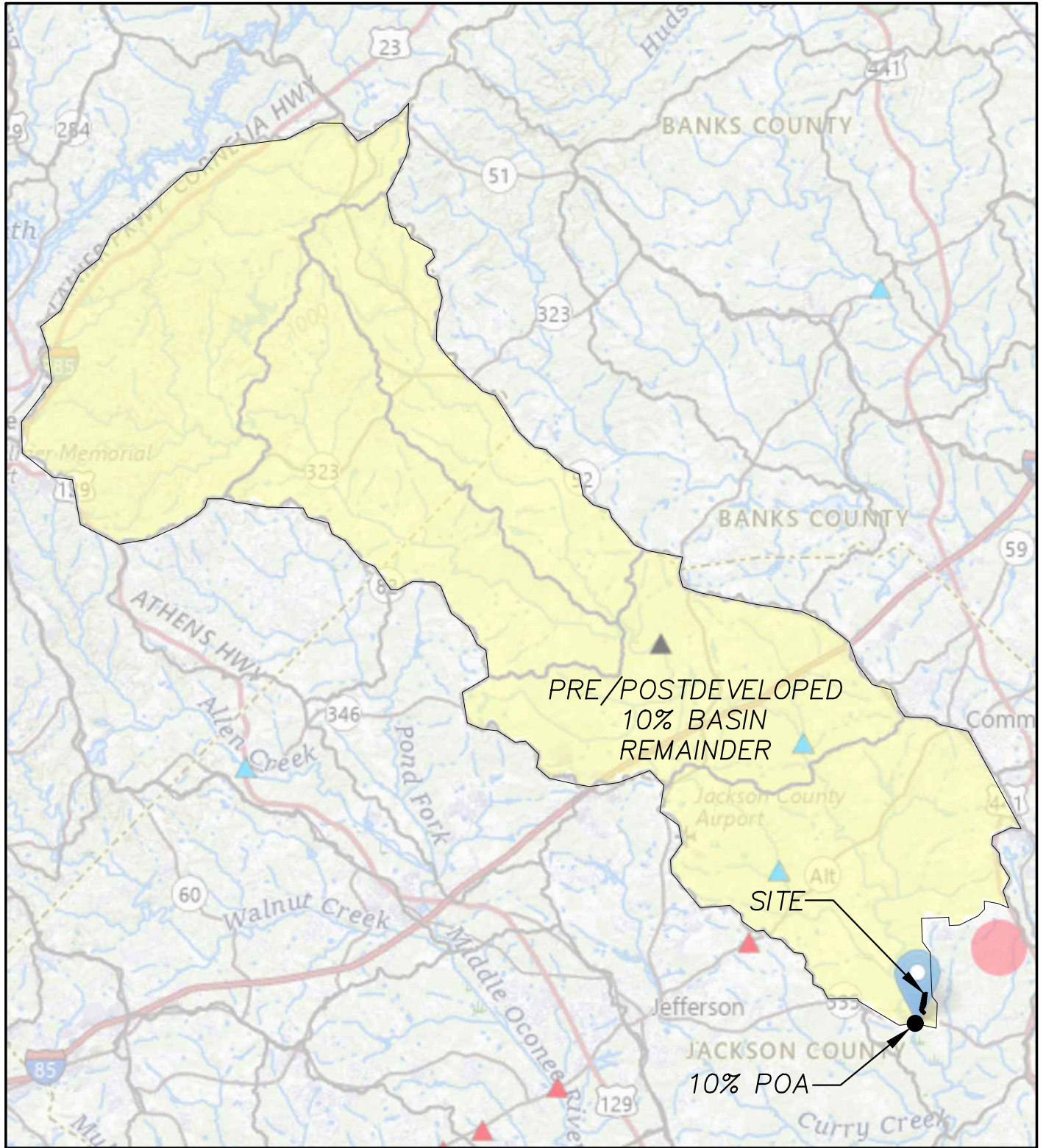
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24-HR CONTACT: BARRY VICKERY, (706) 286-6353

PROJECT NUMBER:	JKS002
DATE:	6/11/26
ISSUE NUMBER:	2
CHECKED BY:	RDB
SHEET TITLE:	GRADING AND DRAINAGE PLAN (NORTH)
SHEET NUMBER:	C4

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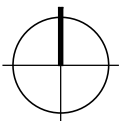
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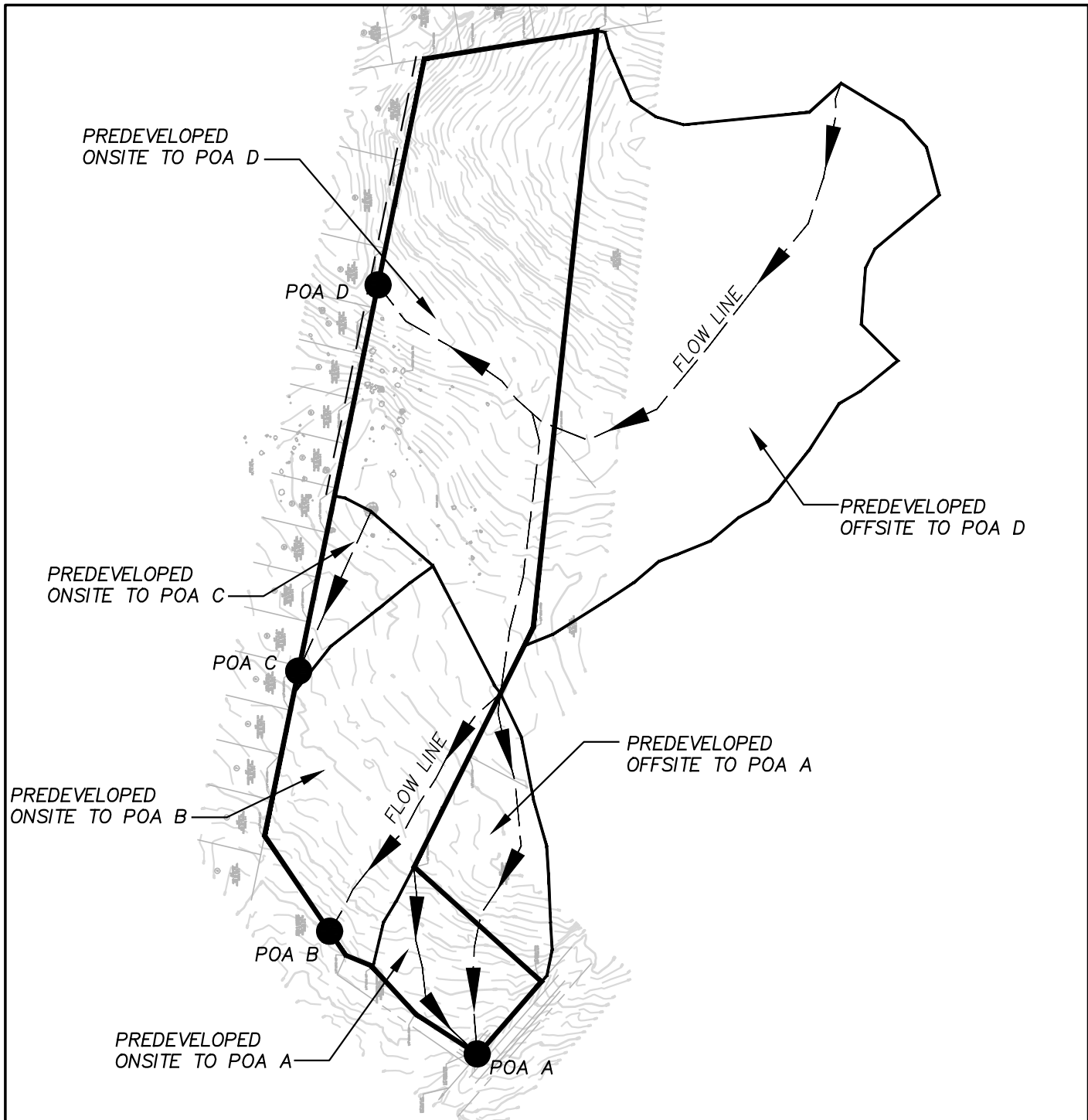
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SITE	23.25		
PRE/POSTDEVELOPED 10% BASIN REMAINDER	84,027		
10% BASIN TOTAL	84,050		

NORTH



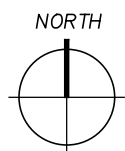
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10% STUDY BASIN



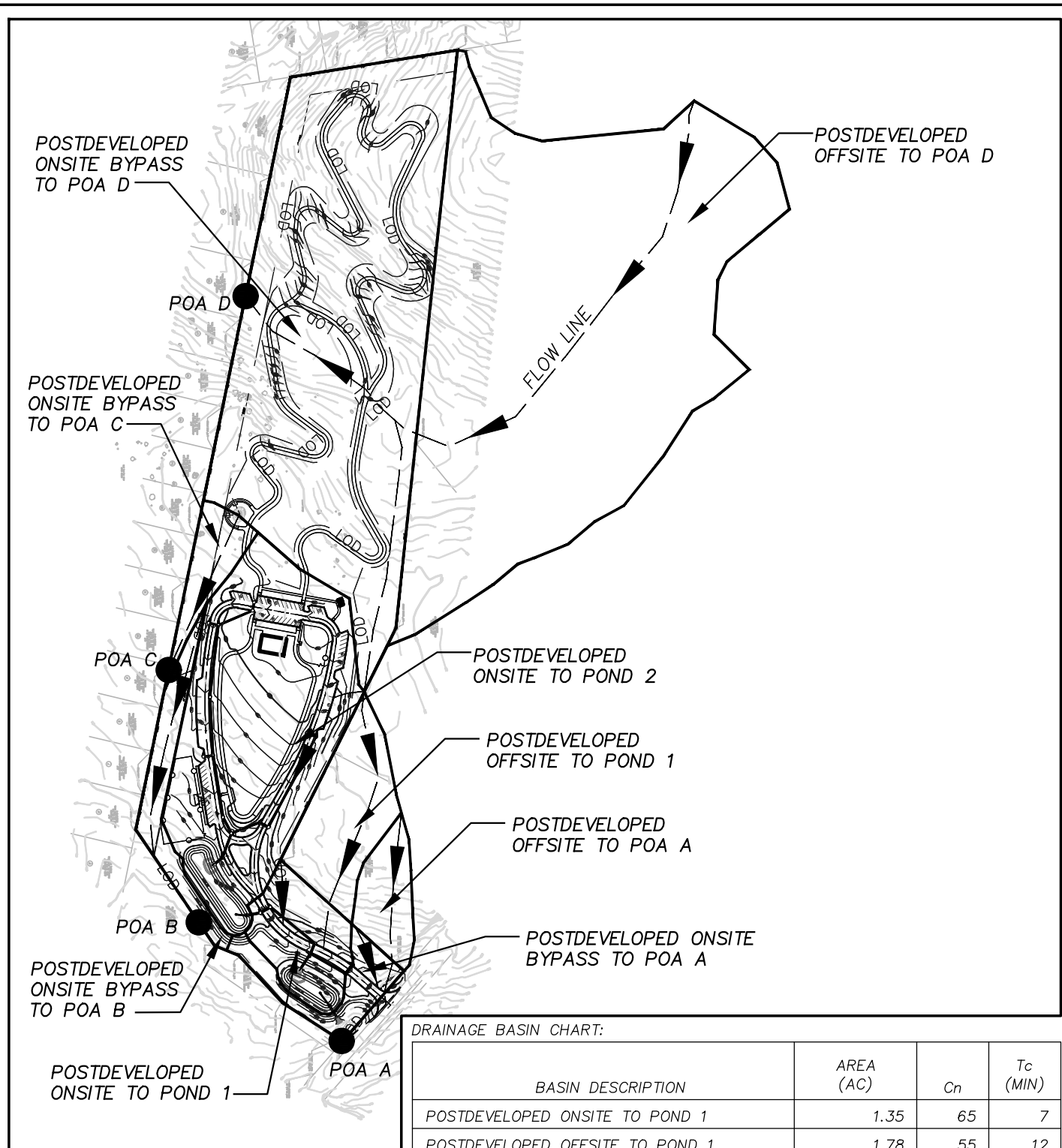
DRAINAGE BASIN CHART:

BASIN DESCRIPTION	AREA (AC)	C _n	T _c (MIN)
PREDEVELOPED ONSITE TO POA A	2.07	55	9
PREDEVELOPED OFFSITE TO POA A	2.63	55	12
PREDEVELOPED ONSITE TO POA B	6.60	55	11
PREDEVELOPED ONSITE TO POA C	1.36	55	11
PREDEVELOPED ONSITE TO POA D	13.22	55	15
PREDEVELOPED OFFSITE TO POA D	17.29	55	17



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PREDEVELOPED BASINS



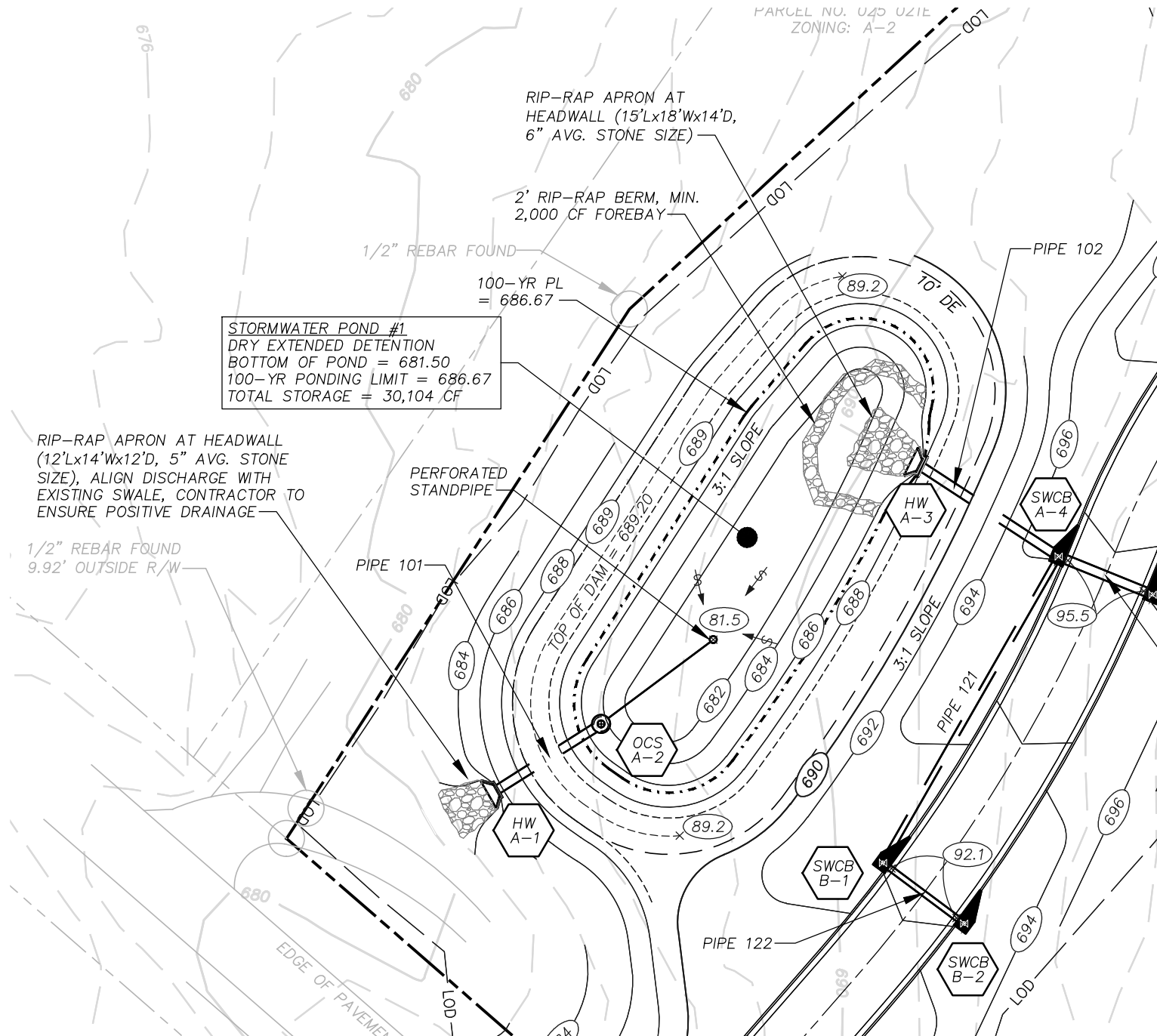
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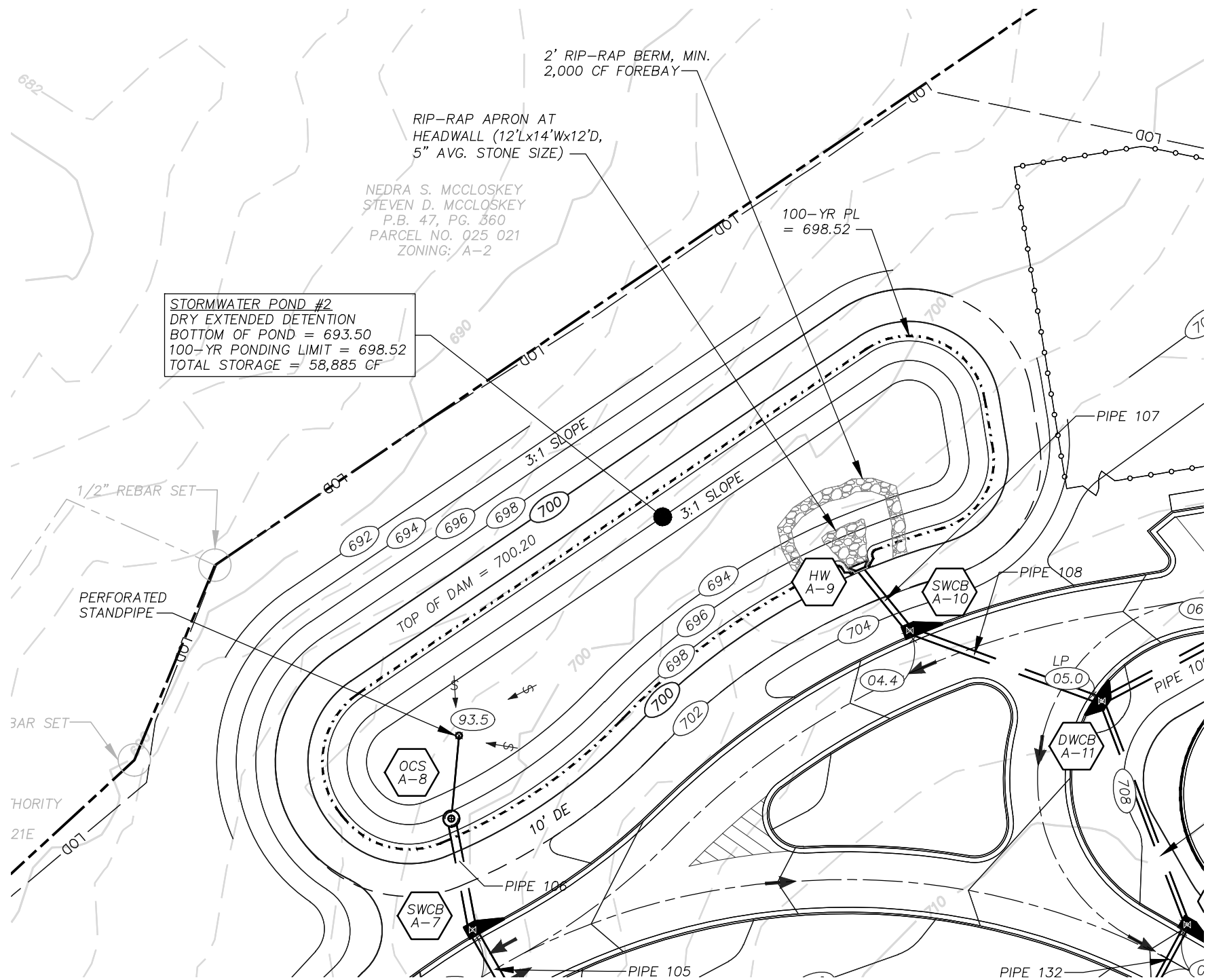
BASIN DESCRIPTION	AREA (AC)	C _n	T _c (MIN)
POSTDEVELOPED ONSITE TO POND 1	1.35	65	7
POSTDEVELOPED OFFSITE TO POND 1	1.78	55	12
POSTDEVELOPED ONSITE TO POND 2	6.53	69	5
POSTDEVELOPED ONSITE BYPASS TO POA A	0.77	63	8
POSTDEVELOPED OFFSITE BYPASS TO POA A	0.84	55	11
POSTDEVELOPED ONSITE BYPASS TO POA B	0.94	55	8
POSTDEVELOPED ONSITE BYPASS TO POA C	0.71	55	11
POSTDEVELOPED ONSITE BYPASS TO POA D	12.95	57	15
POSTDEVELOPED OFFSITE BYPASS TO POA D	17.29	55	17

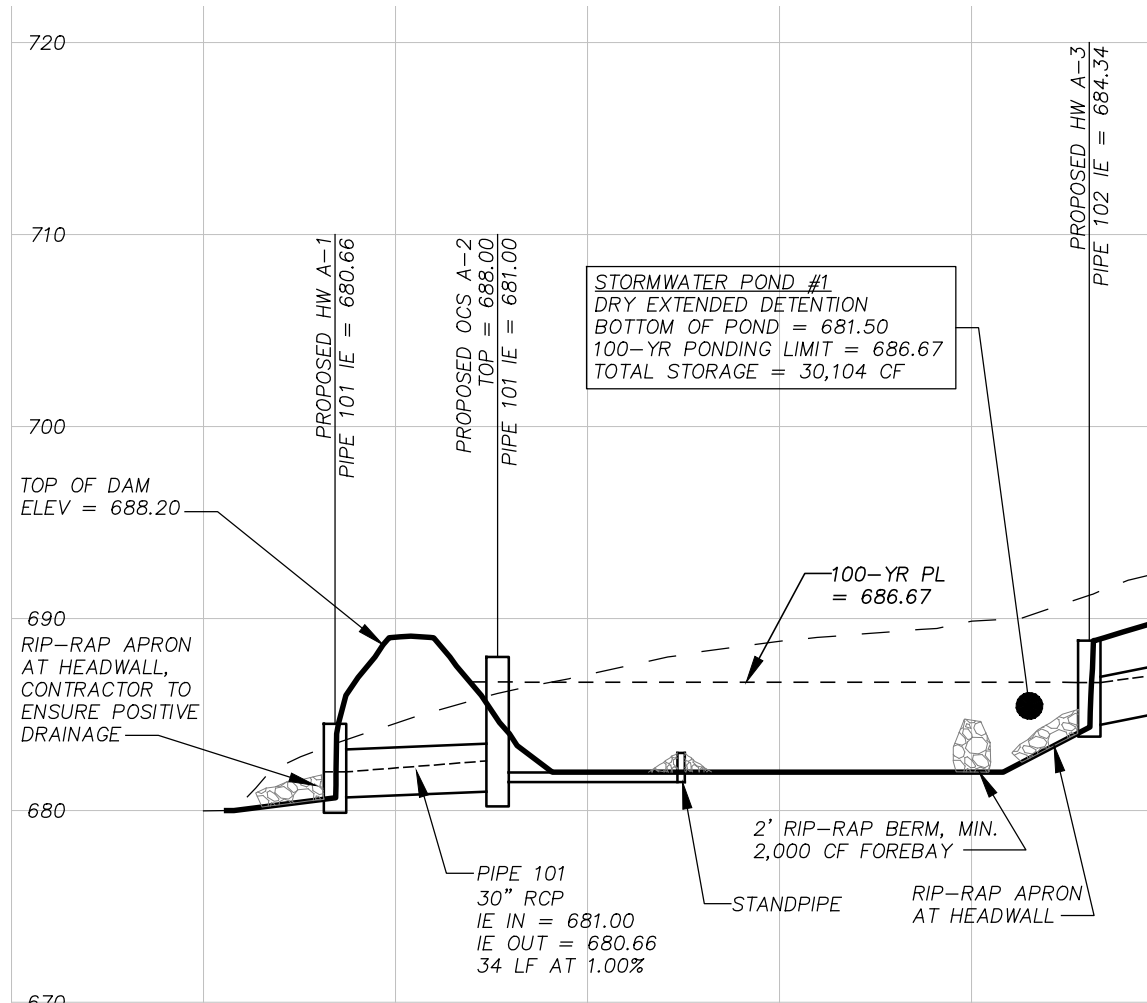


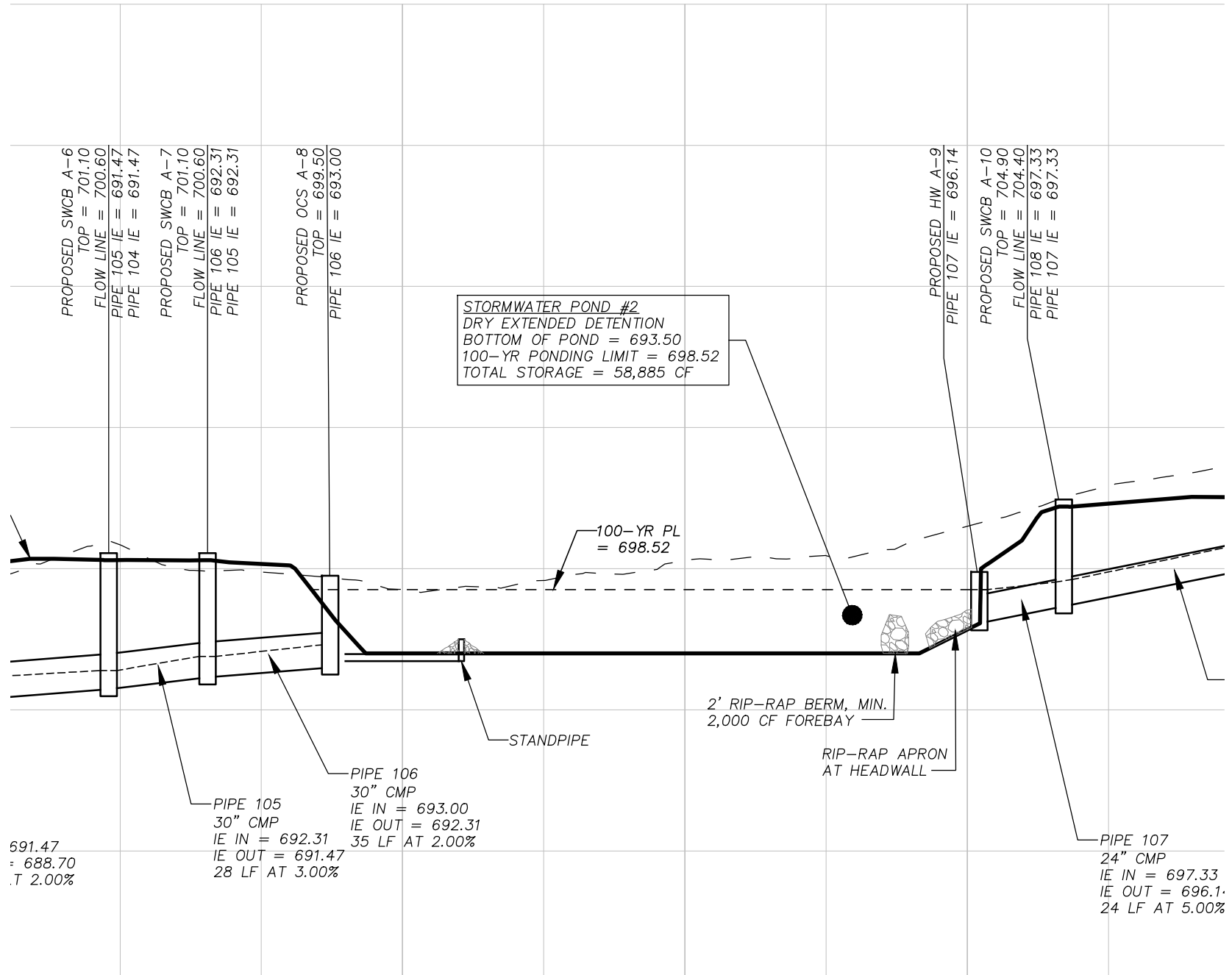
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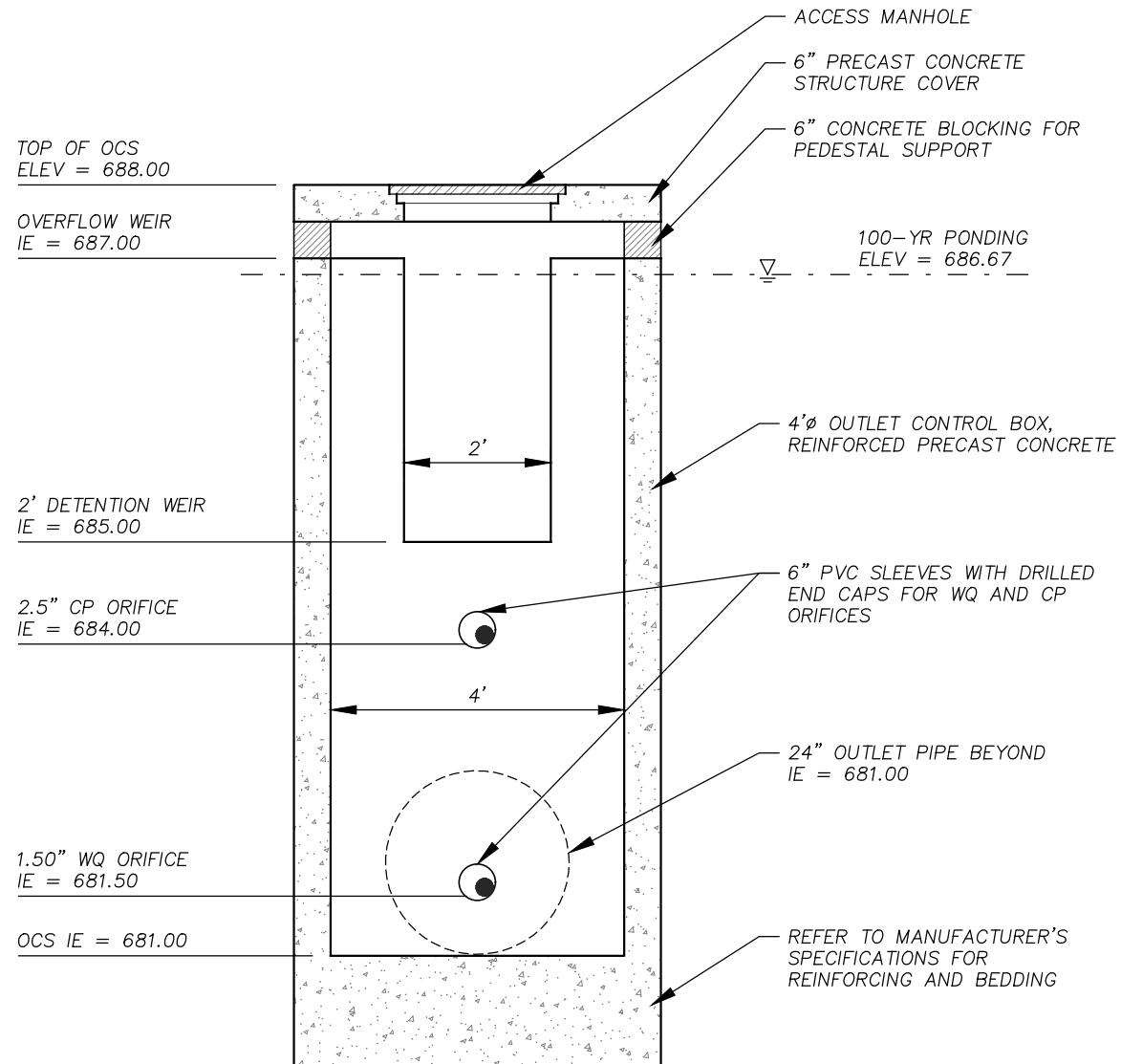
POSTDEVELOPED BASINS





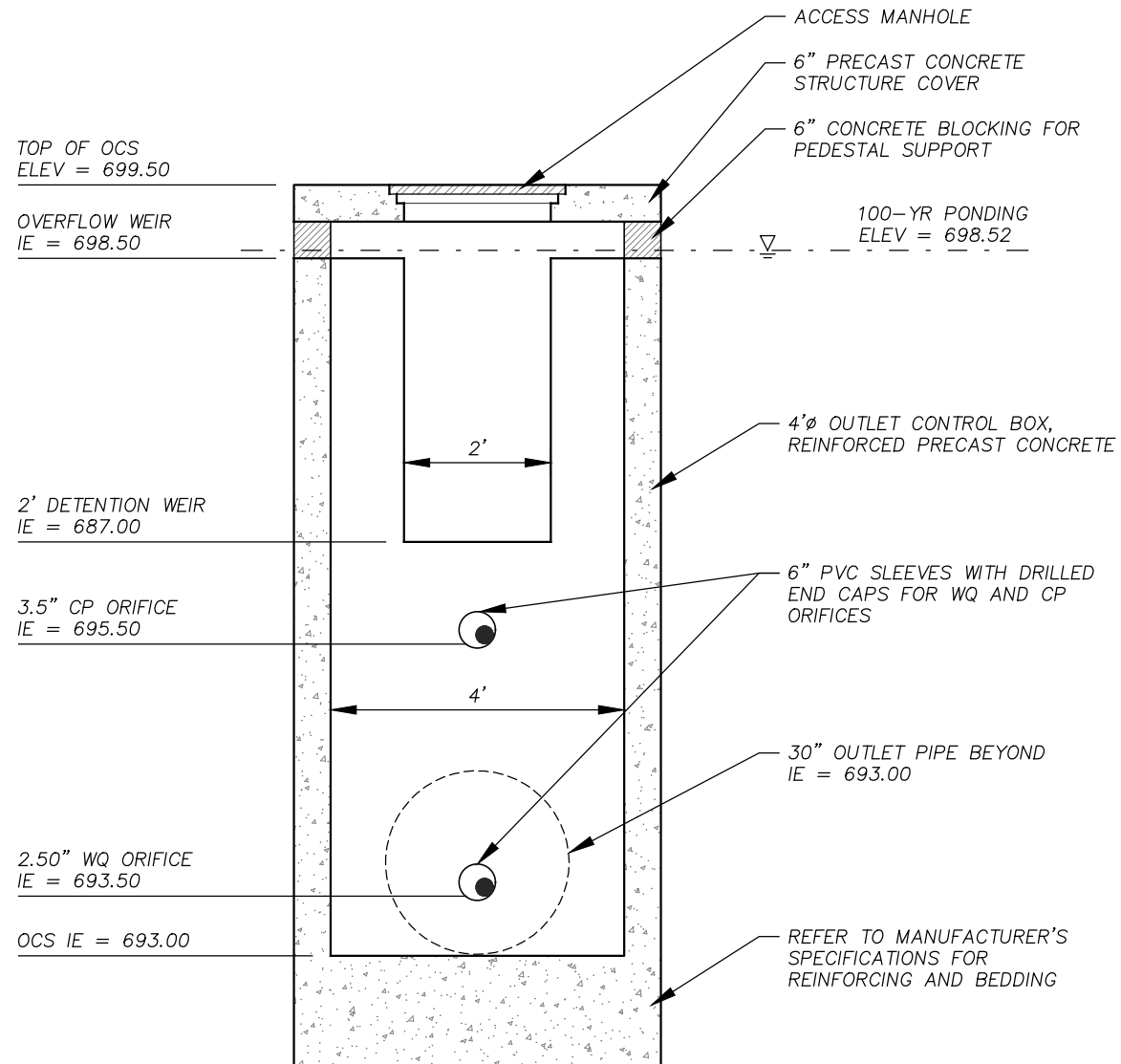






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OUTLET CONTROL STRUCTURE A-2



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OUTLET CONTROL STRUCTURE A-8

Appendix C:

Hydraflow Model Results

Hydrograph Return Period Recap

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	SCS Runoff	-----	0.473	0.974	-----	2.138	3.317	5.211	6.873	8.720	Pre Onsite to POA A
2	SCS Runoff	-----	0.498	1.052	-----	2.328	3.679	5.859	7.776	9.913	Pre Offsite to POA A
3	Combine	1, 2	0.937	1.997	-----	4.341	6.795	10.88	14.48	18.50	Pre Total to POA A
4	SCS Runoff	-----	1.251	2.640	-----	5.842	9.233	14.70	19.51	24.88	Pre Onsite to POA B
5	SCS Runoff	-----	0.258	0.544	-----	1.204	1.902	3.030	4.021	5.126	Pre Onsite to POA C
6	SCS Runoff	-----	2.126	4.487	-----	9.963	15.84	25.34	33.71	43.04	Pre Onsite to POA D
7	SCS Runoff	-----	2.475	5.208	-----	11.55	18.55	30.02	40.14	51.44	Pre Offsite to POA D
8	Combine	6, 7	4.562	9.532	-----	21.34	33.79	54.68	73.29	94.09	Pre Total to POA D
10	SCS Runoff	-----	9.299	12.43	-----	18.27	23.67	31.81	38.75	46.26	Post Onsite to Pond 2
11	Reservoir	10	0.230	0.405	-----	0.598	1.142	3.757	8.251	13.80	Dry ED Pond 2
12	SCS Runoff	-----	1.284	1.830	-----	2.875	3.862	5.378	6.657	8.062	Post Onsite to Pond 1
13	SCS Runoff	-----	0.337	0.712	-----	1.576	2.490	3.965	5.263	6.709	Post Offsite to Pond 1
14	Combine	11, 12, 13	1.683	2.609	-----	4.622	6.743	10.76	18.50	26.82	Post Total to Pond 1
15	Reservoir	14	0.207	0.341	-----	0.752	1.416	4.344	8.679	14.77	Dry ED Pond 1
16	SCS Runoff	-----	0.606	0.898	-----	1.464	2.005	2.841	3.552	4.324	Post Onsite Bypass to POA A
17	SCS Runoff	-----	0.159	0.336	-----	0.744	1.175	1.871	2.484	3.166	Post Offsite Bypass to POA A
18	Combine	15, 16, 17	0.757	1.222	-----	2.203	3.186	4.881	9.747	17.04	Post Total to POA A
19	SCS Runoff	-----	0.215	0.442	-----	0.971	1.506	2.367	3.121	3.960	Post Onsite Bypass to POA B
20	SCS Runoff	-----	0.135	0.284	-----	0.628	0.993	1.582	2.099	2.676	Post Onsite Bypass to POA C
21	SCS Runoff	-----	3.185	5.871	-----	11.93	18.06	27.86	36.42	45.92	Post Onsite Bypass to POA D
22	SCS Runoff	-----	2.475	5.208	-----	11.55	18.55	30.02	40.14	51.44	Post Offsite Bypass to POA D
23	Combine	21, 22	5.526	10.92	-----	23.12	35.86	57.21	76.00	96.95	Post Total to POA D
Proj. file: JKS002 5-21-26.gpw										Wednesday, 06 / 3 / 2026	

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

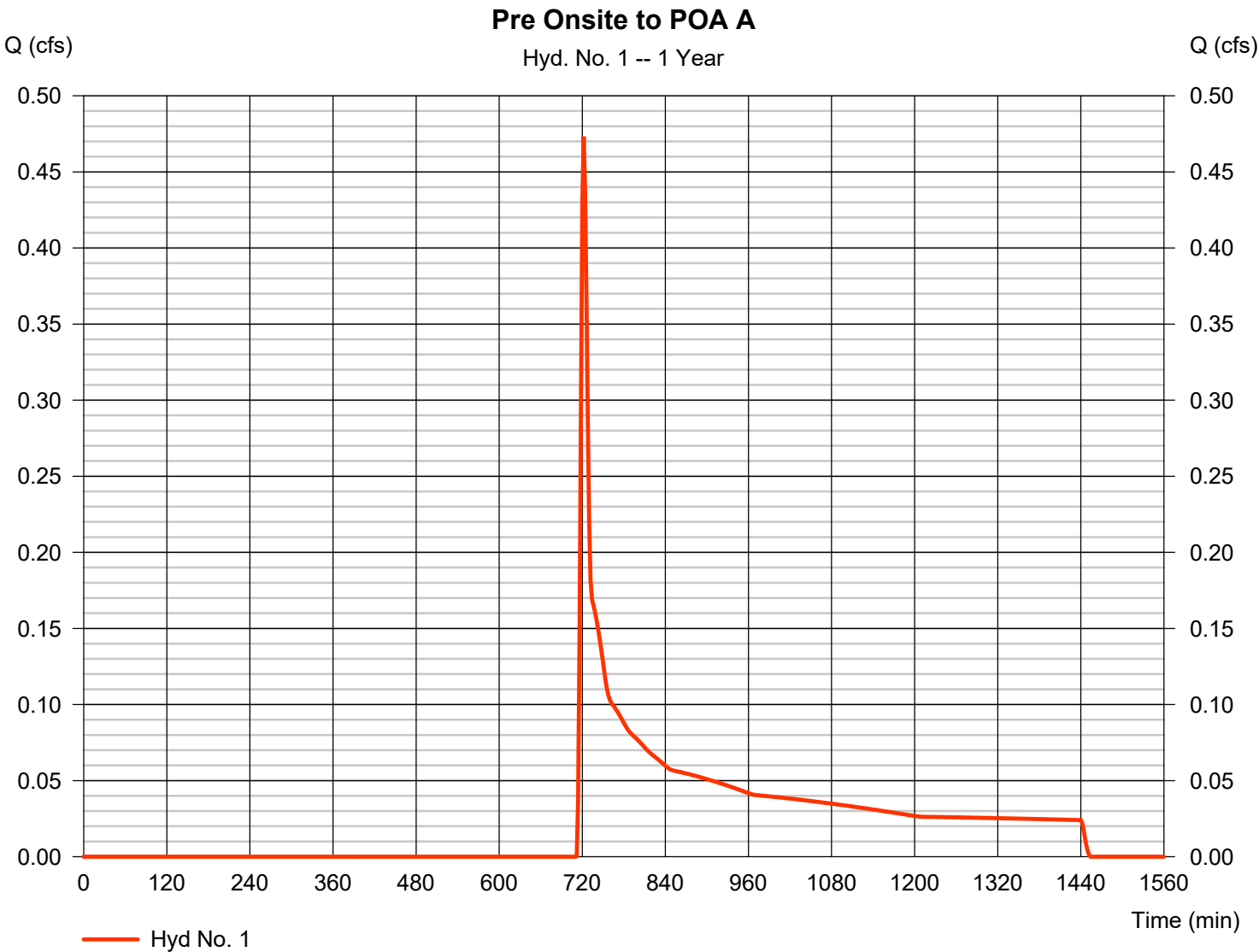
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.473	2	722	2,177	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	0.498	2	724	2,853	-----	-----	-----	Pre Offsite to POA A
3	Combine	0.937	2	724	5,030	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	1.251	2	724	7,159	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	0.258	2	724	1,475	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	2.126	2	726	13,558	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	2.475	2	730	18,187	-----	-----	-----	Pre Offsite to POA D
8	Combine	4.562	2	728	31,746	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	9.299	2	718	18,974	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	0.230	2	1012	18,948	10	695.46	10,862	Dry ED Pond 2
12	SCS Runoff	1.284	2	720	3,257	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	0.337	2	724	1,931	-----	-----	-----	Post Offsite to Pond 1
14	Combine	1.683	2	720	24,135	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	0.207	2	1468	24,120	14	684.52	9,252	Dry ED Pond 1
16	SCS Runoff	0.606	2	720	1,617	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	0.159	2	724	911	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	0.757	2	720	26,648	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	0.215	2	722	989	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	0.135	2	724	770	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	3.185	2	726	16,216	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	2.475	2	730	18,187	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	5.526	2	728	34,403	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 1 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.473 cfs
Storm frequency	=	1 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	2,177 cuft
Drainage area	=	2.070 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	9.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

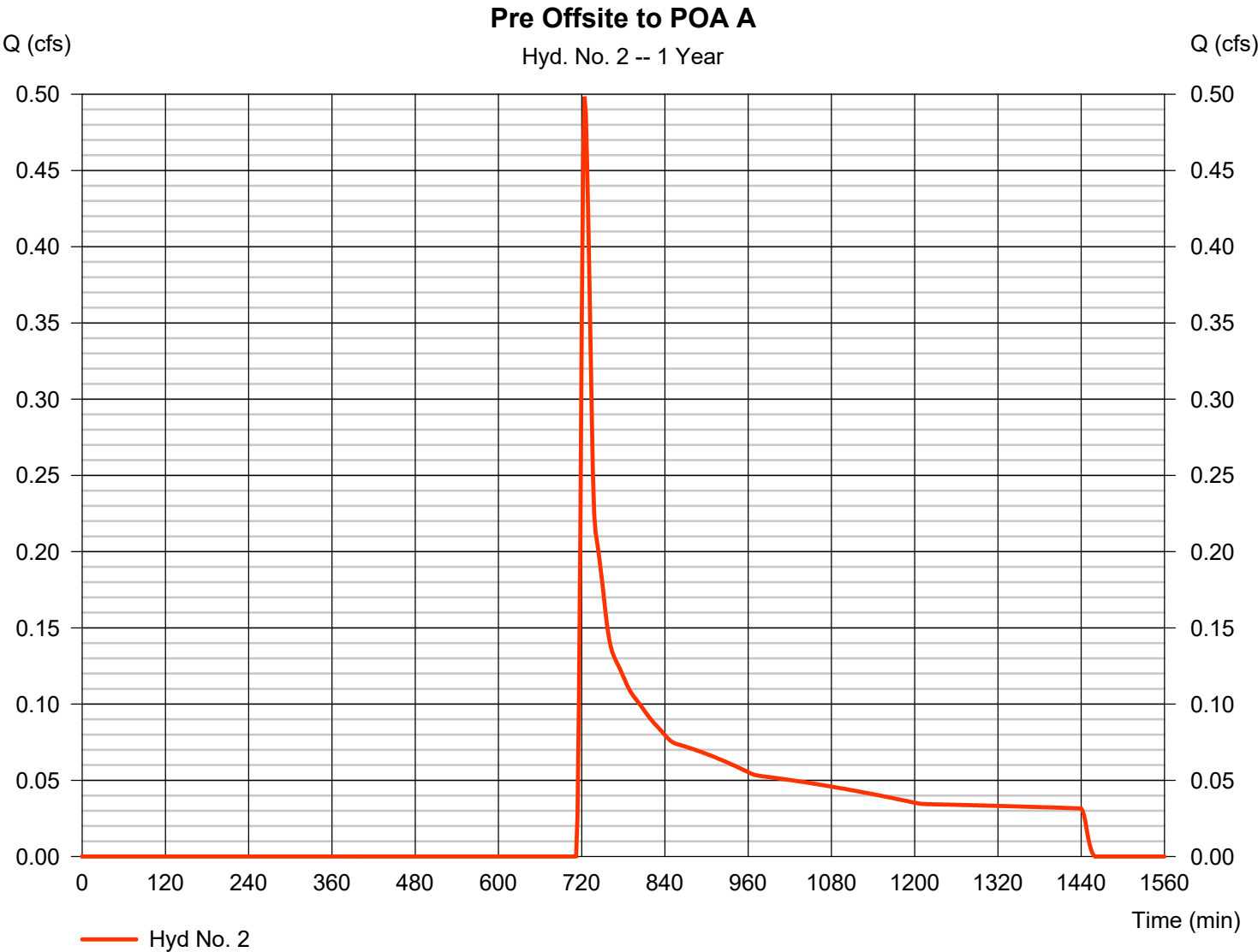


Hydrograph Report

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.498 cfs
Storm frequency	=	1 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	2,853 cuft
Drainage area	=	2.630 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

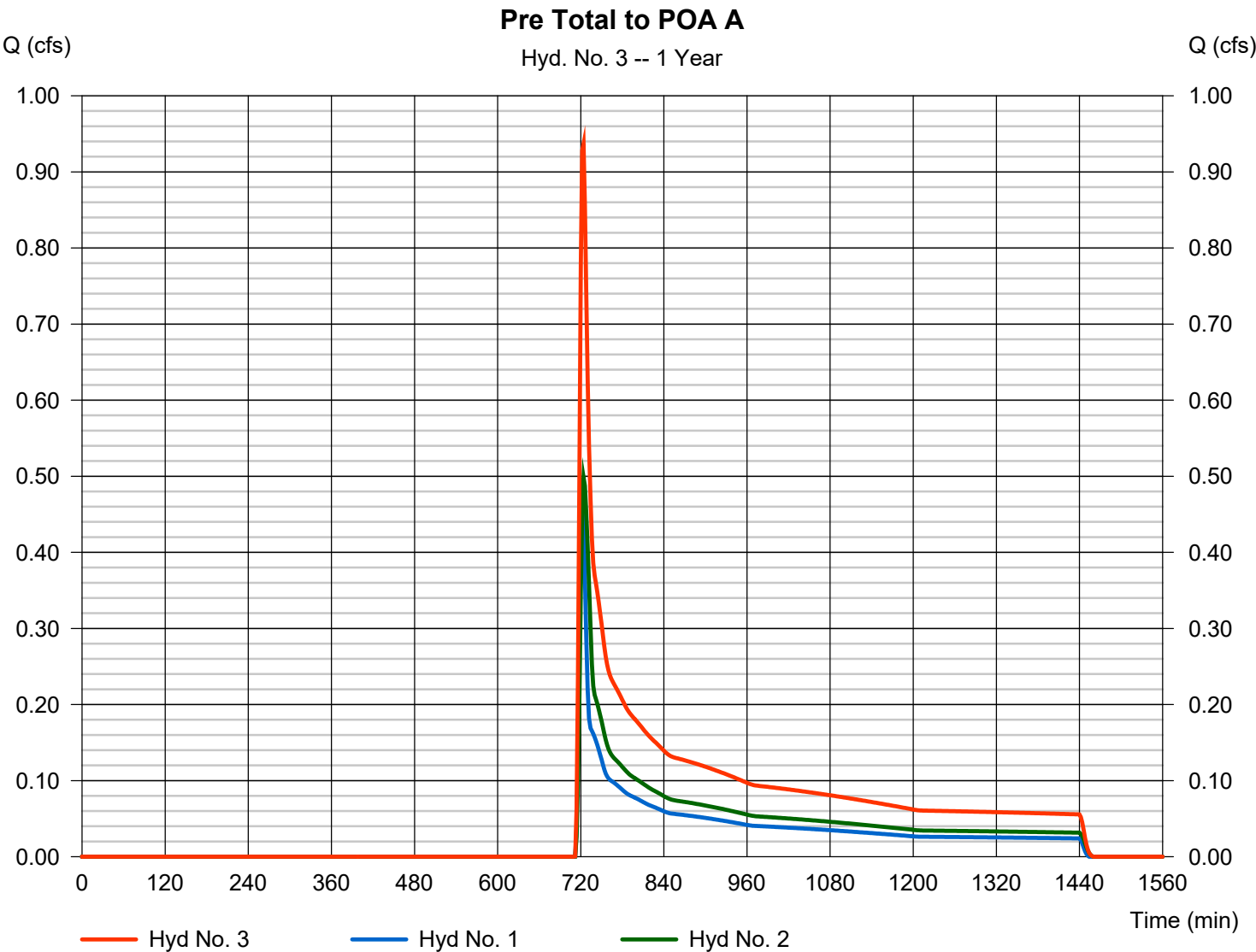


Hydrograph Report

Hyd. No. 3

Pre Total to POA A

Hydrograph type	= Combine	Peak discharge	= 0.937 cfs
Storm frequency	= 1 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 5,030 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 4.700 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

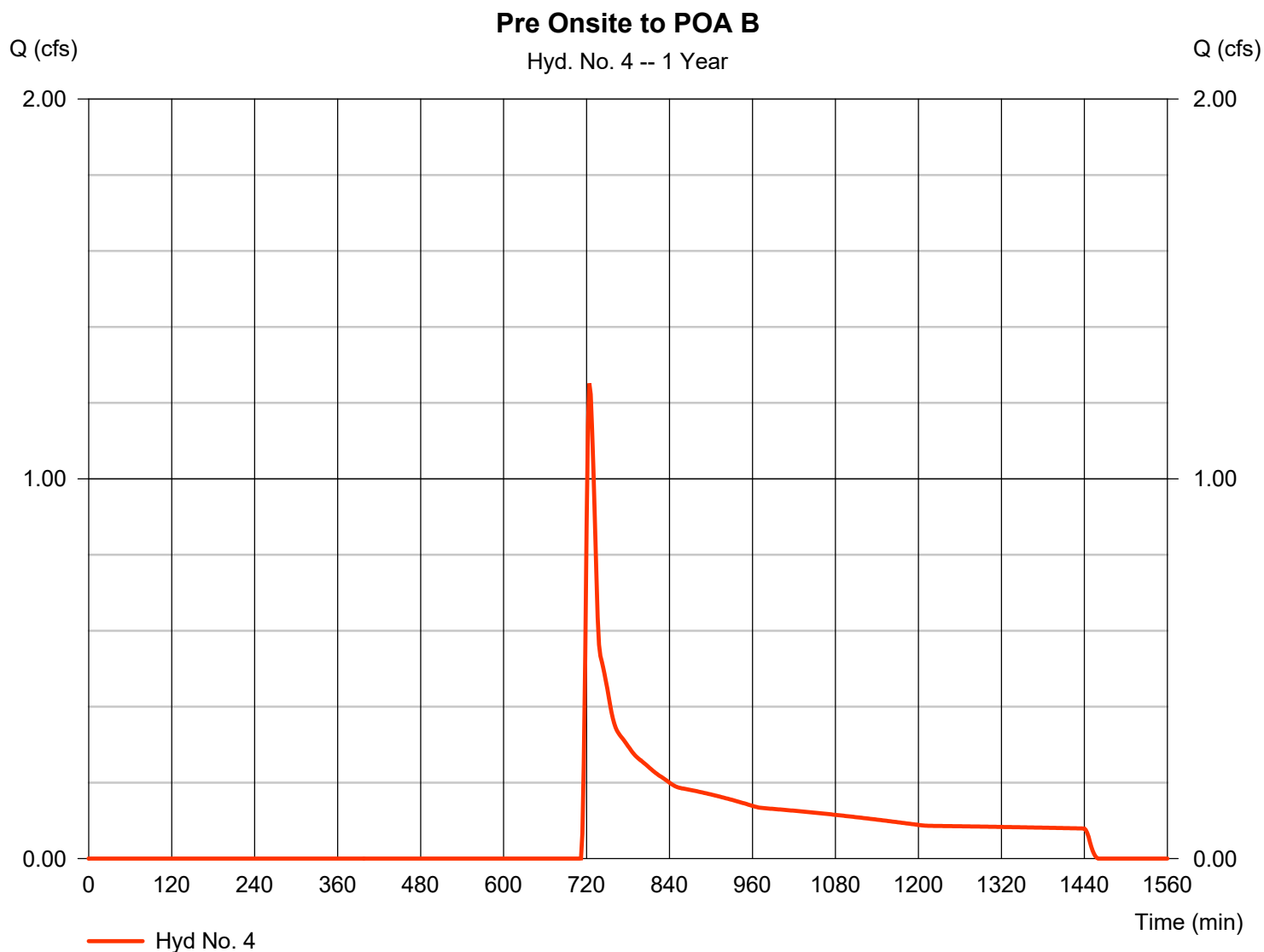
Wednesday, 06 / 3 / 2026

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type = SCS Runoff
 Storm frequency = 1 yrs
 Time interval = 2 min
 Drainage area = 6.600 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 3.33 in
 Storm duration = 24 hrs

Peak discharge = 1.251 cfs
 Time to peak = 724 min
 Hyd. volume = 7,159 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 11.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

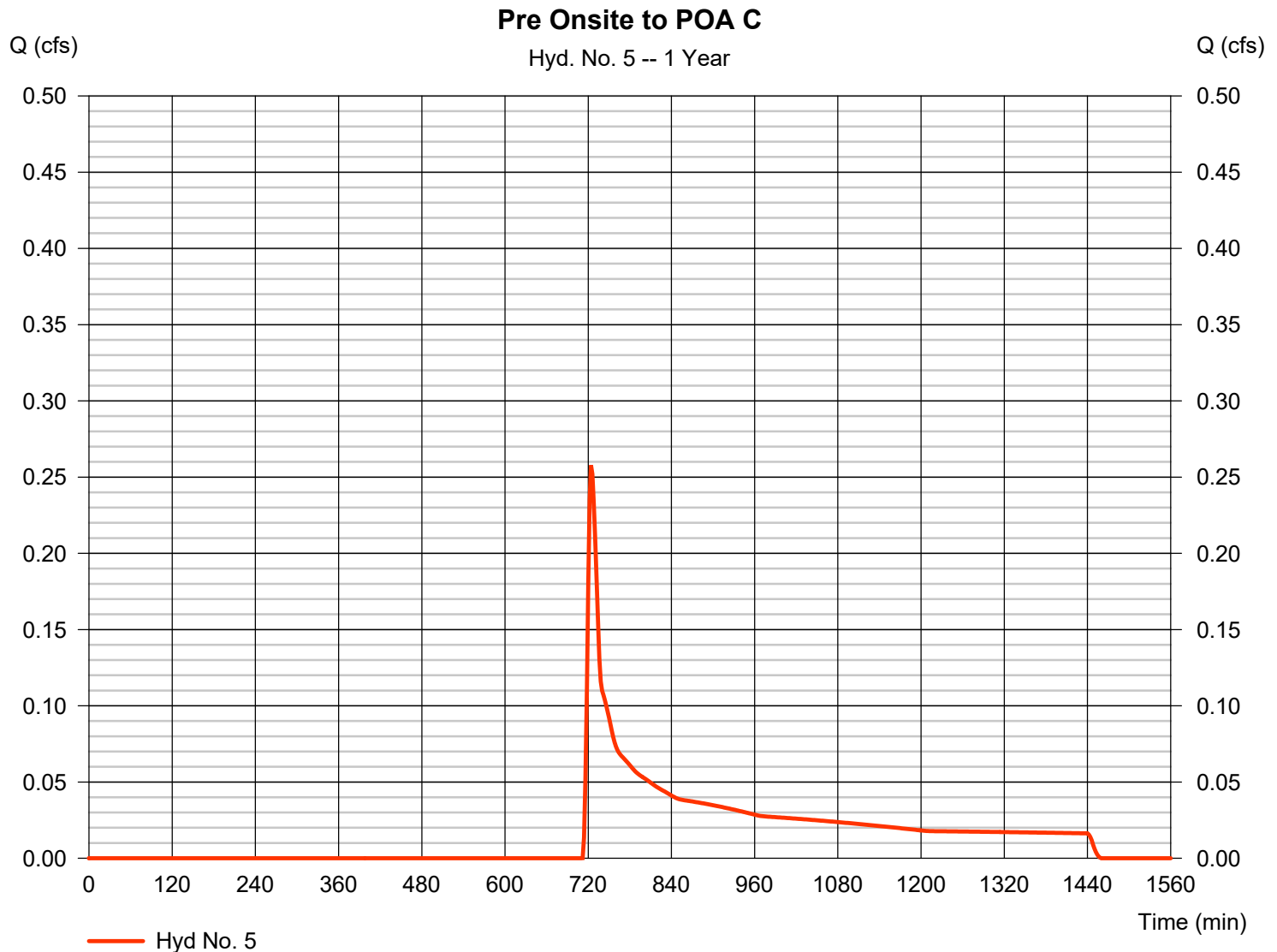
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Wednesday, 06 / 3 / 2026

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 0.258 cfs
Storm frequency	= 1 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 1,475 cuft
Drainage area	= 1.360 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

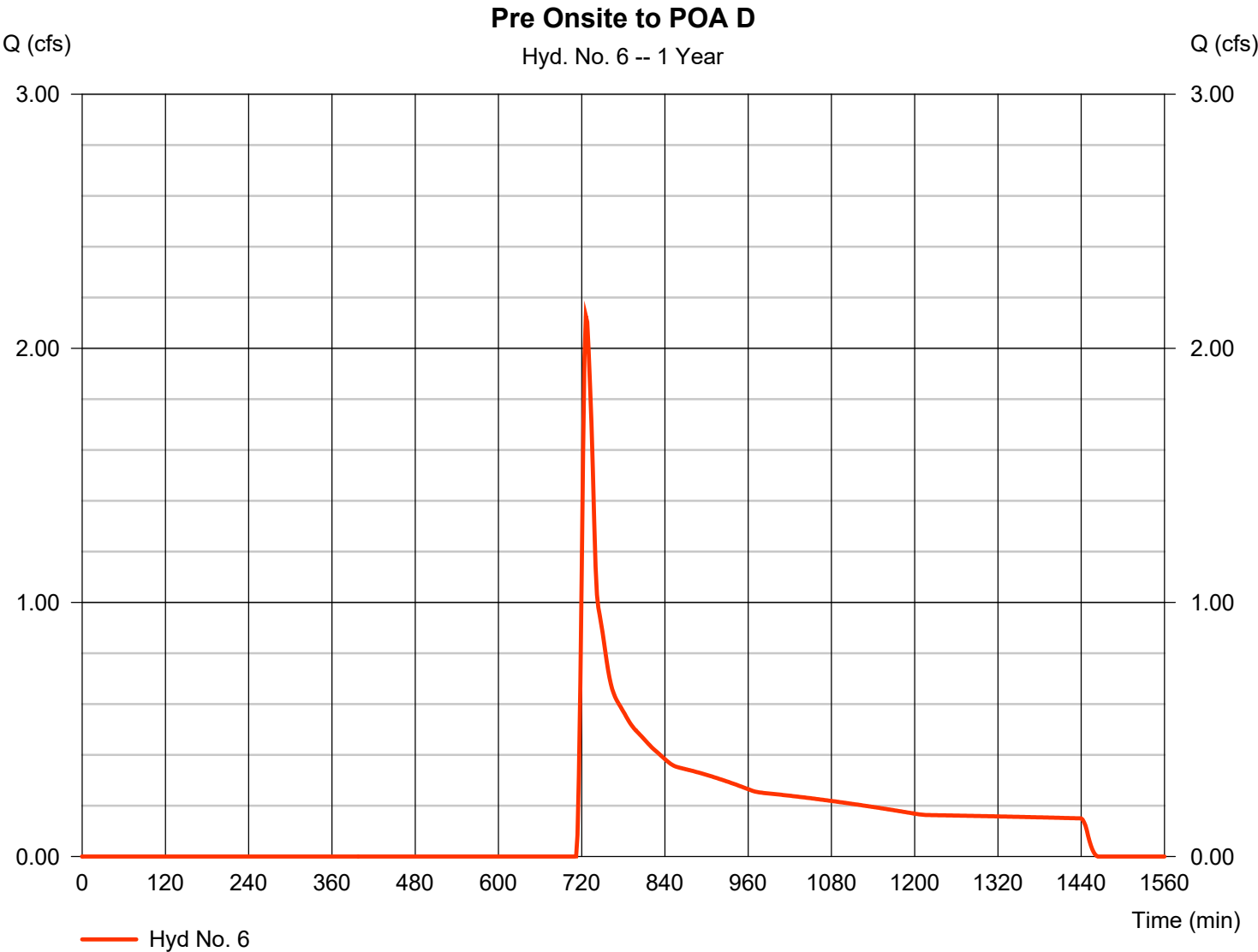


Hydrograph Report

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.126 cfs
Storm frequency	=	1 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	13,558 cuft
Drainage area	=	13.220 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	15.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

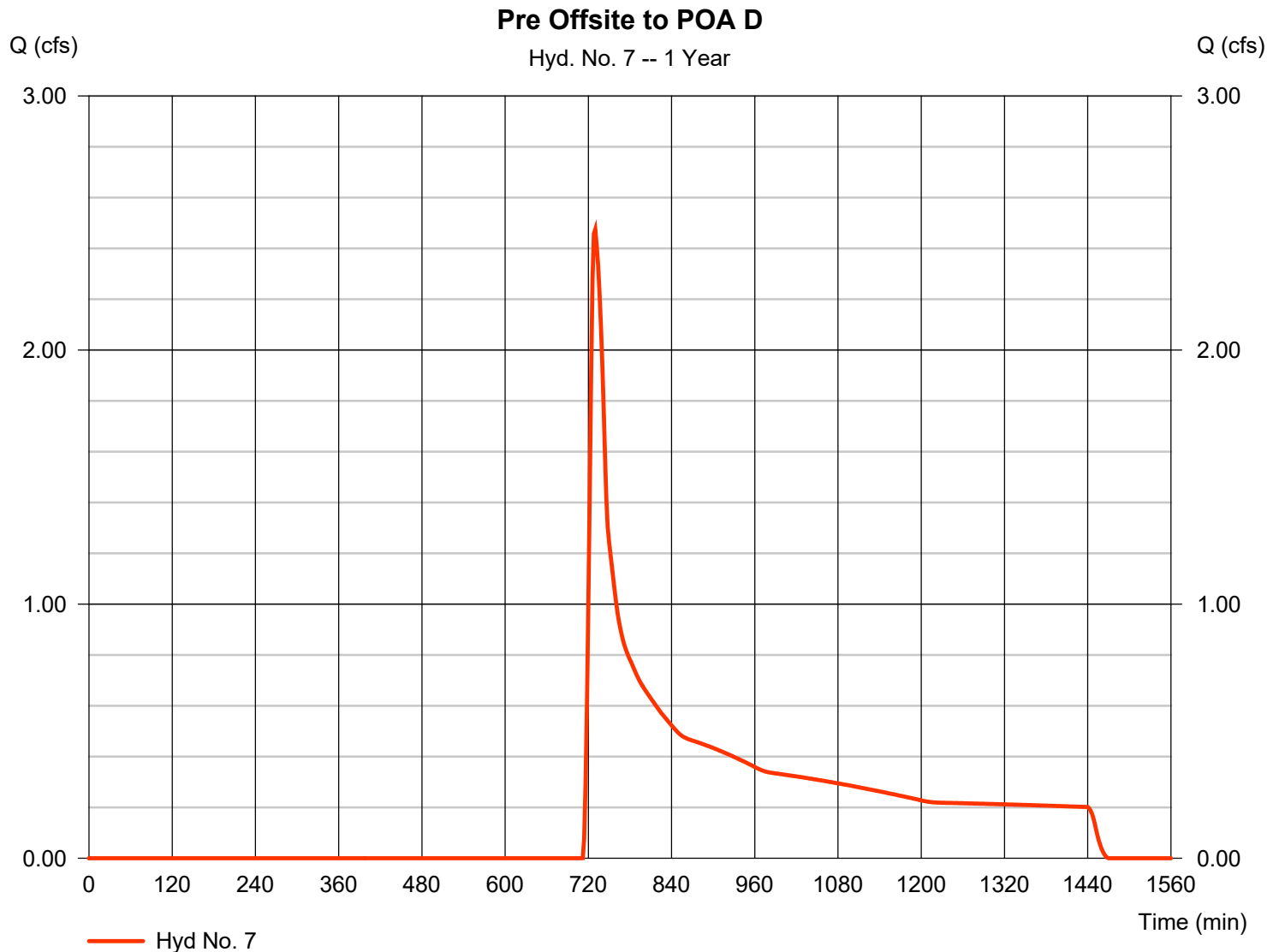
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Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 2.475 cfs
Storm frequency	= 1 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 18,187 cuft
Drainage area	= 17.290 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 17.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

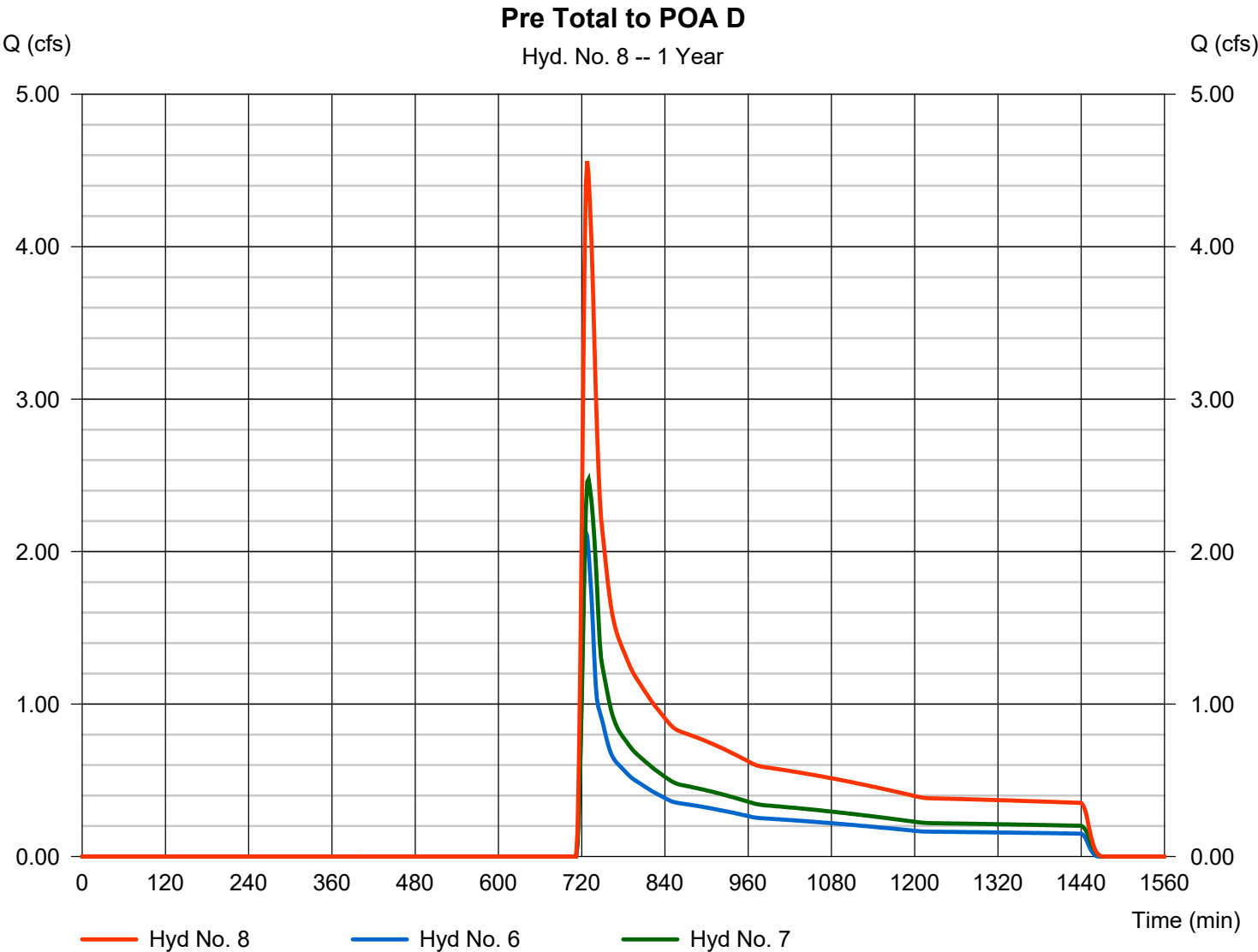


Hydrograph Report

Hyd. No. 8

Pre Total to POA D

Hydrograph type	= Combine	Peak discharge	= 4.562 cfs
Storm frequency	= 1 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 31,746 cuft
Inflow hyds.	= 6, 7	Contrib. drain. area	= 30.510 ac



Hydrograph Report

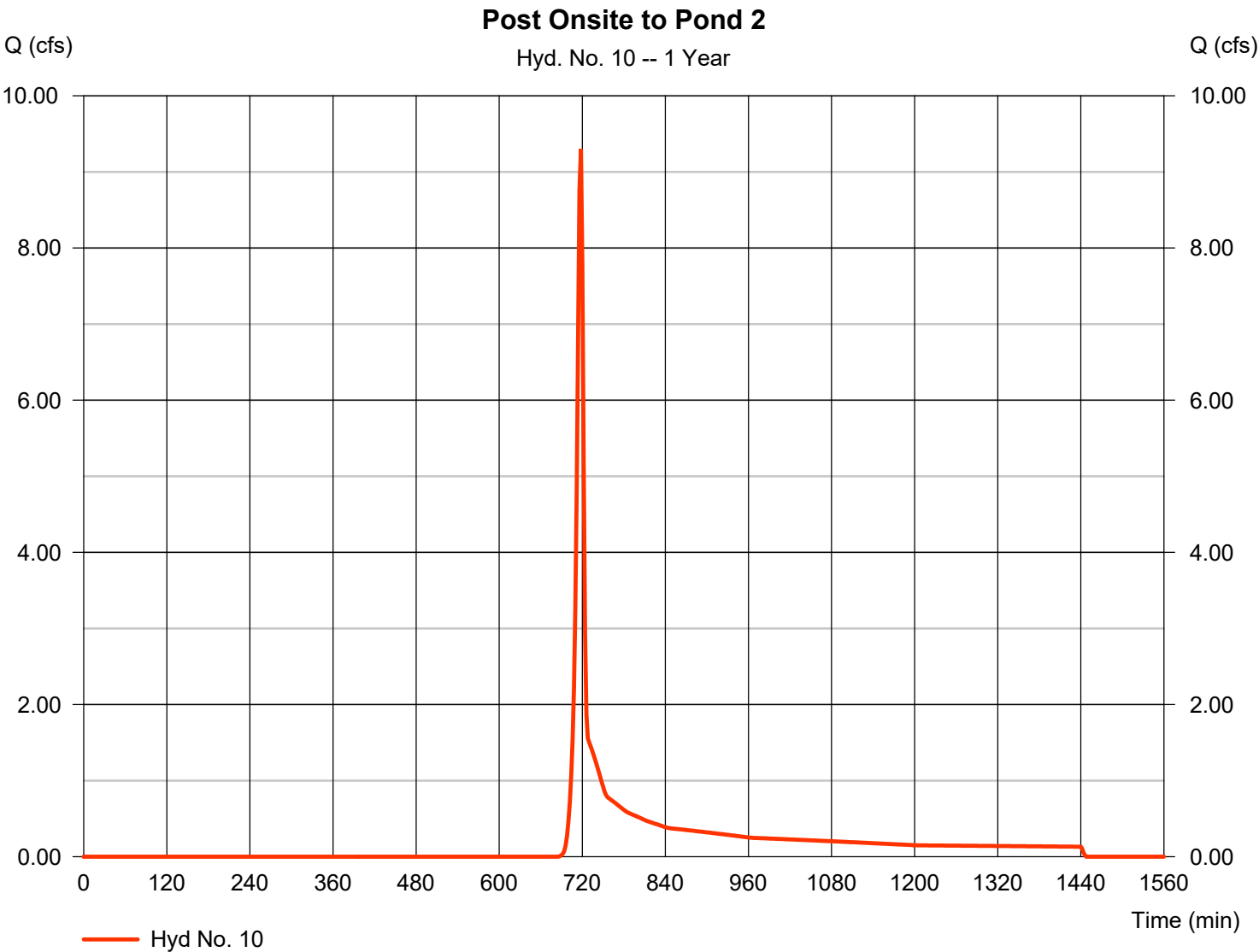
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Wednesday, 06 / 3 / 2026

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	=	SCS Runoff	Peak discharge	=	9.299 cfs
Storm frequency	=	1 yrs	Time to peak	=	718 min
Time interval	=	2 min	Hyd. volume	=	18,974 cuft
Drainage area	=	6.530 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	5.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

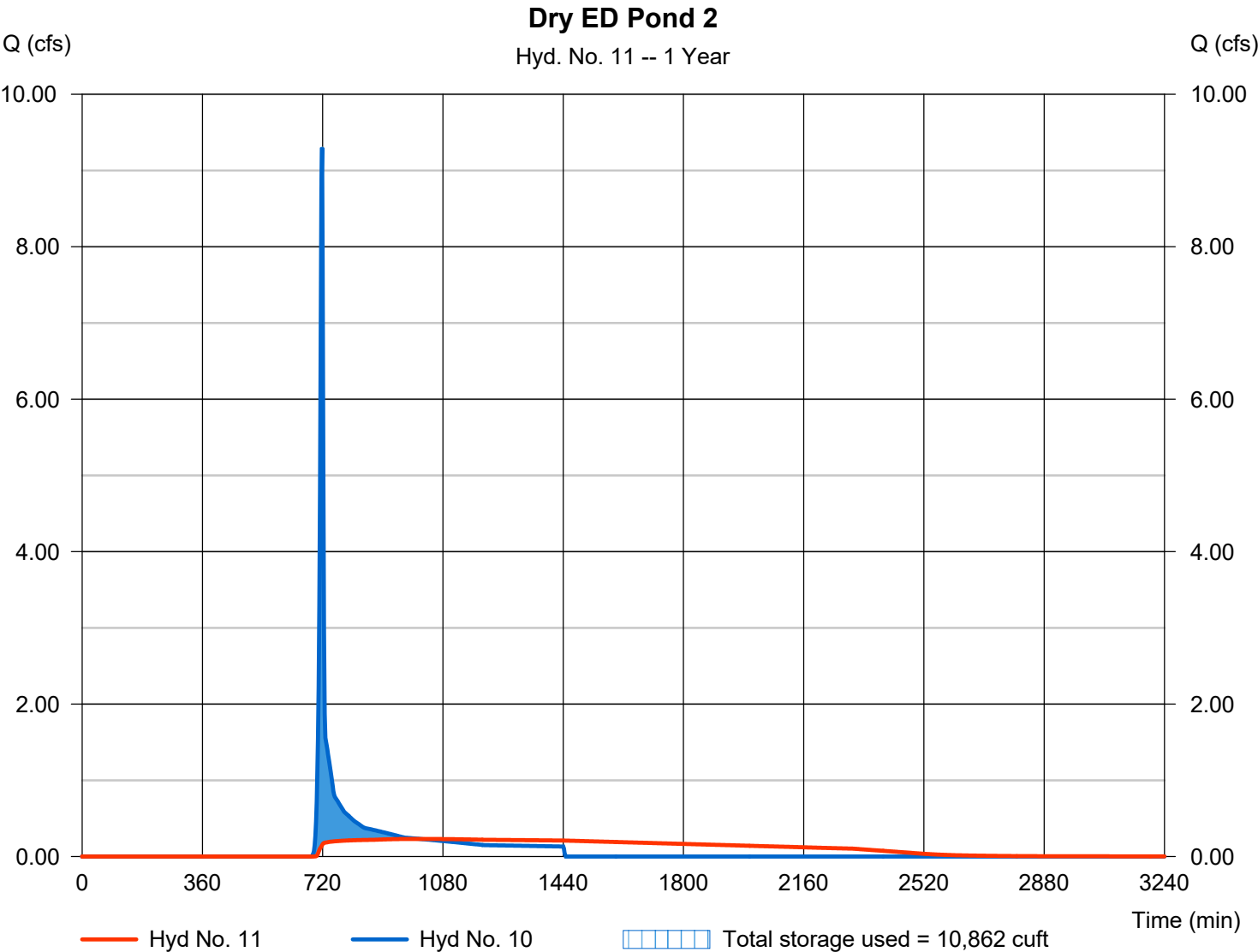
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 0.230 cfs
Storm frequency	= 1 yrs	Time to peak	= 1012 min
Time interval	= 2 min	Hyd. volume	= 18,948 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 695.46 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 10,862 cuft

Storage Indication method used.



Pond No. 2 - Dry ED Pond 2

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Beginning Elevation = 693.50 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	693.50	00	0	0
0.50	694.00	5,124	1,281	1,281
2.50	696.00	7,983	13,107	14,388
4.50	698.00	11,068	19,051	33,439
6.50	700.00	14,378	25,446	58,885

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 30.00	2.50	3.50	0.00
Span (in)	= 30.00	2.50	3.50	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 693.00	693.50	695.50	0.00
Length (ft)	= 50.00	0.00	0.00	0.00
Slope (%)	= 1.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 2.00	6.57	0.00	0.00
Crest El. (ft)	= 697.00	698.50	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	Rect	---	---
Multi-Stage	= Yes	Yes	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

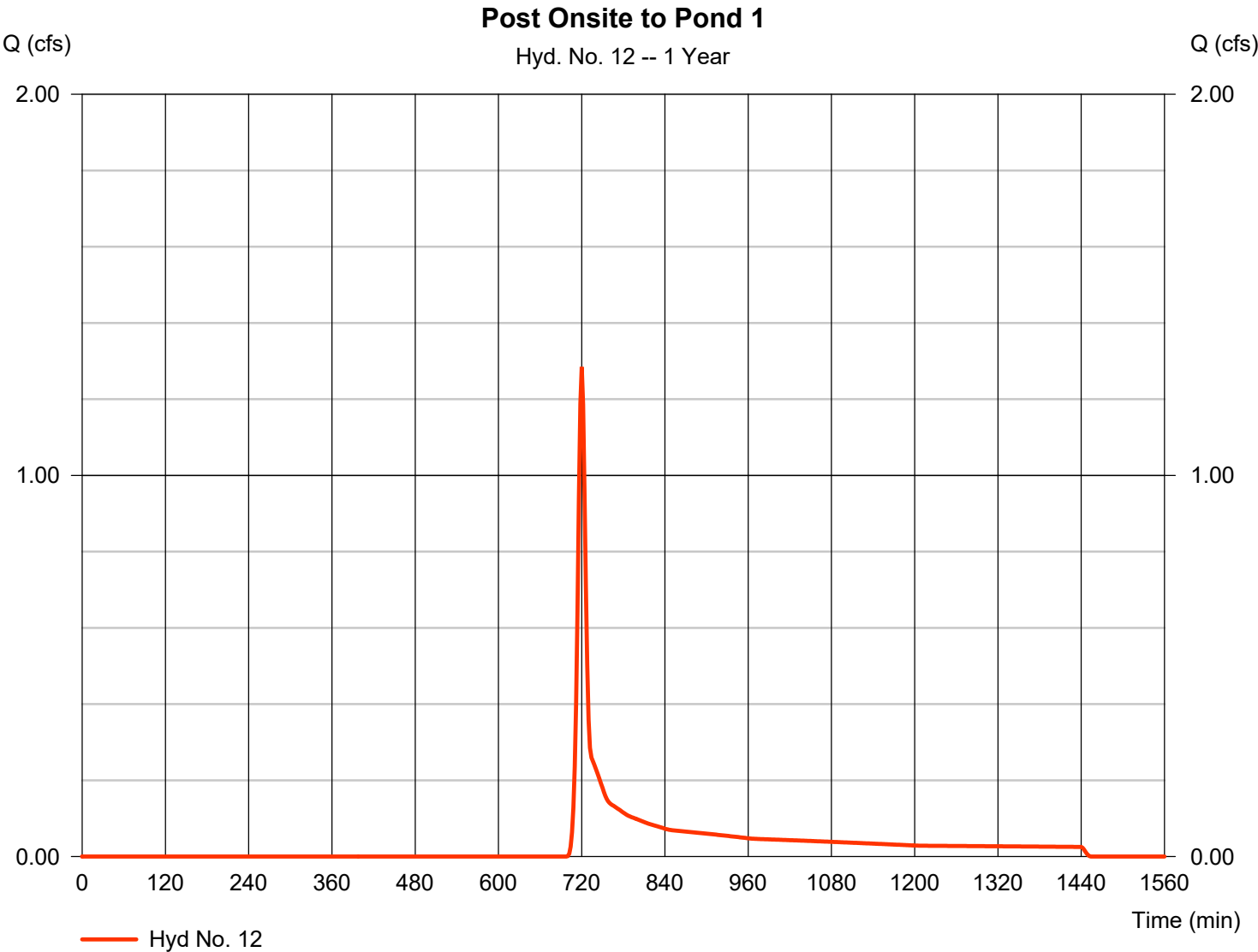
Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	693.50	0.00	0.00	0.00	---	0.00	0.00	---	---	---	---	0.000
0.50	1,281	694.00	1.76 ic	0.10 ic	0.00	---	0.00	0.00	---	---	---	---	0.103
2.50	14,388	696.00	1.76 ic	0.25 ic	0.19 ic	---	0.00	0.00	---	---	---	---	0.445
4.50	33,439	698.00	7.52 ic	0.32 ic	0.49 ic	---	6.66	0.00	---	---	---	---	7.478
6.50	58,885	700.00	53.58 ic	0.13 ic	0.25 ic	---	21.45 s	31.76 s	---	---	---	---	53.58

Hydrograph Report

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.284 cfs
Storm frequency	= 1 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 3,257 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

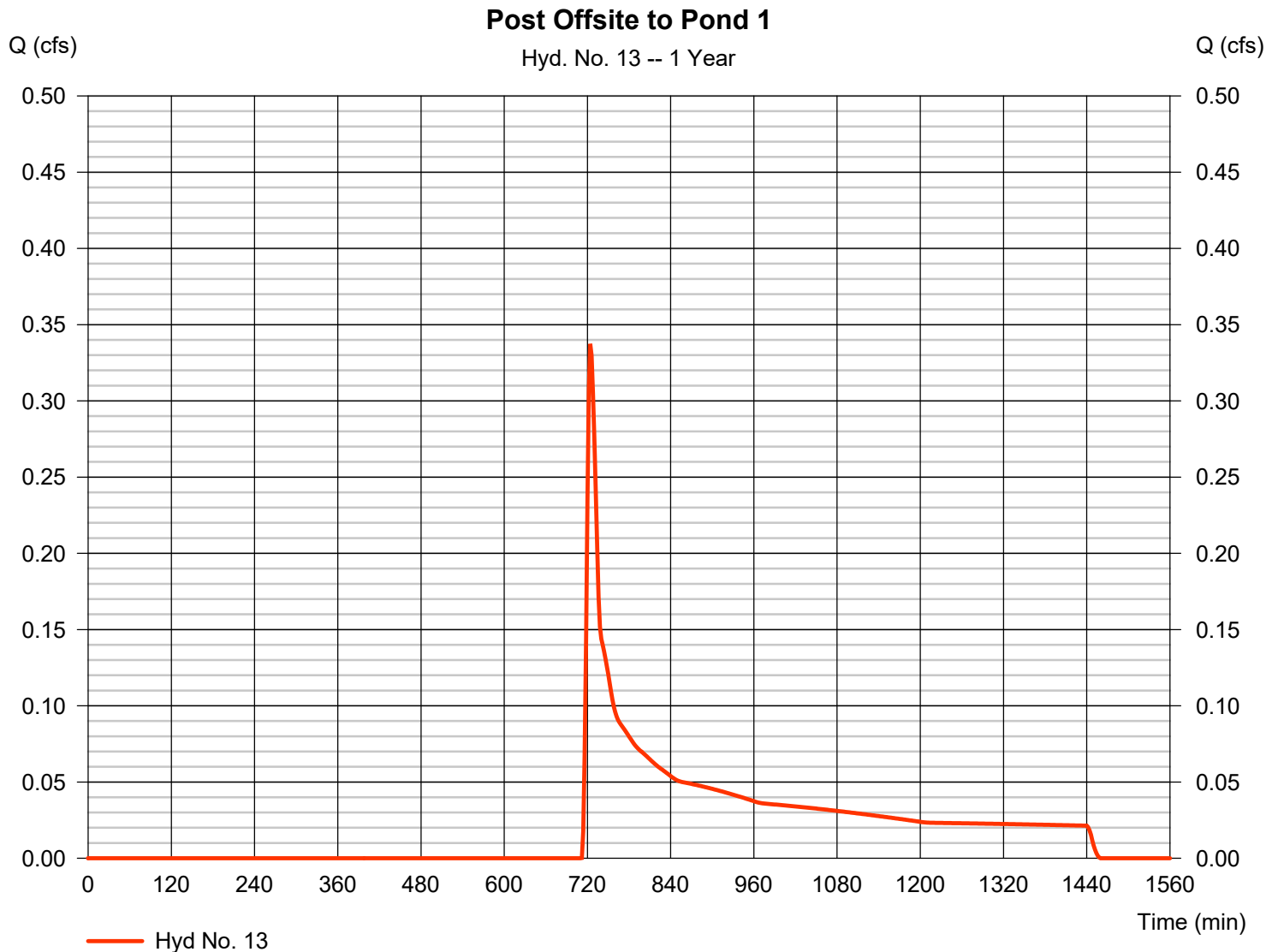
Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type = SCS Runoff
 Storm frequency = 1 yrs
 Time interval = 2 min
 Drainage area = 1.780 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 3.33 in
 Storm duration = 24 hrs

Peak discharge = 0.337 cfs
 Time to peak = 724 min
 Hyd. volume = 1,931 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 12.00 min
 Distribution = Type II
 Shape factor = 484

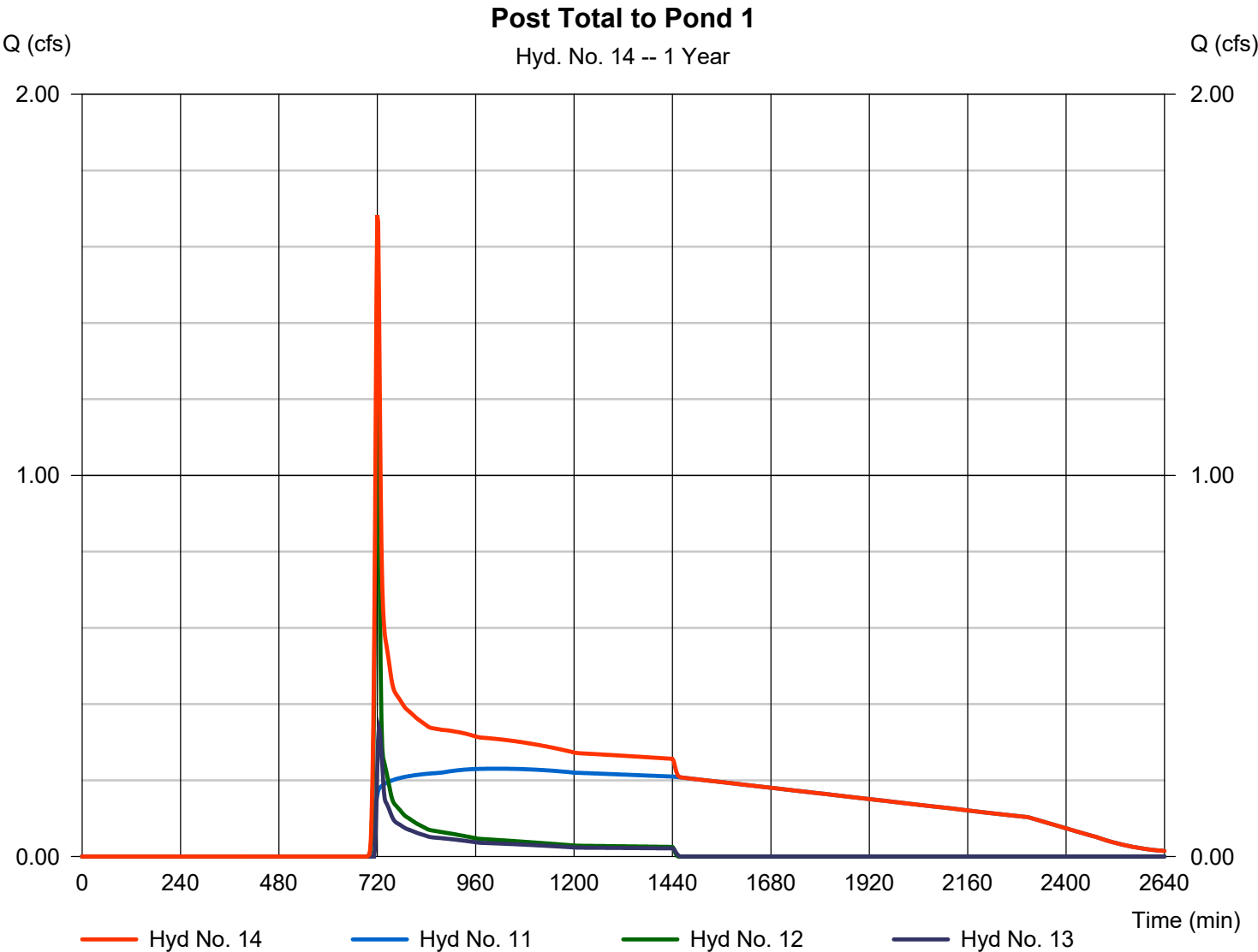


Hydrograph Report

Hyd. No. 14

Post Total to Pond 1

Hydrograph type	= Combine	Peak discharge	= 1.683 cfs
Storm frequency	= 1 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 24,135 cuft
Inflow hyds.	= 11, 12, 13	Contrib. drain. area	= 3.130 ac



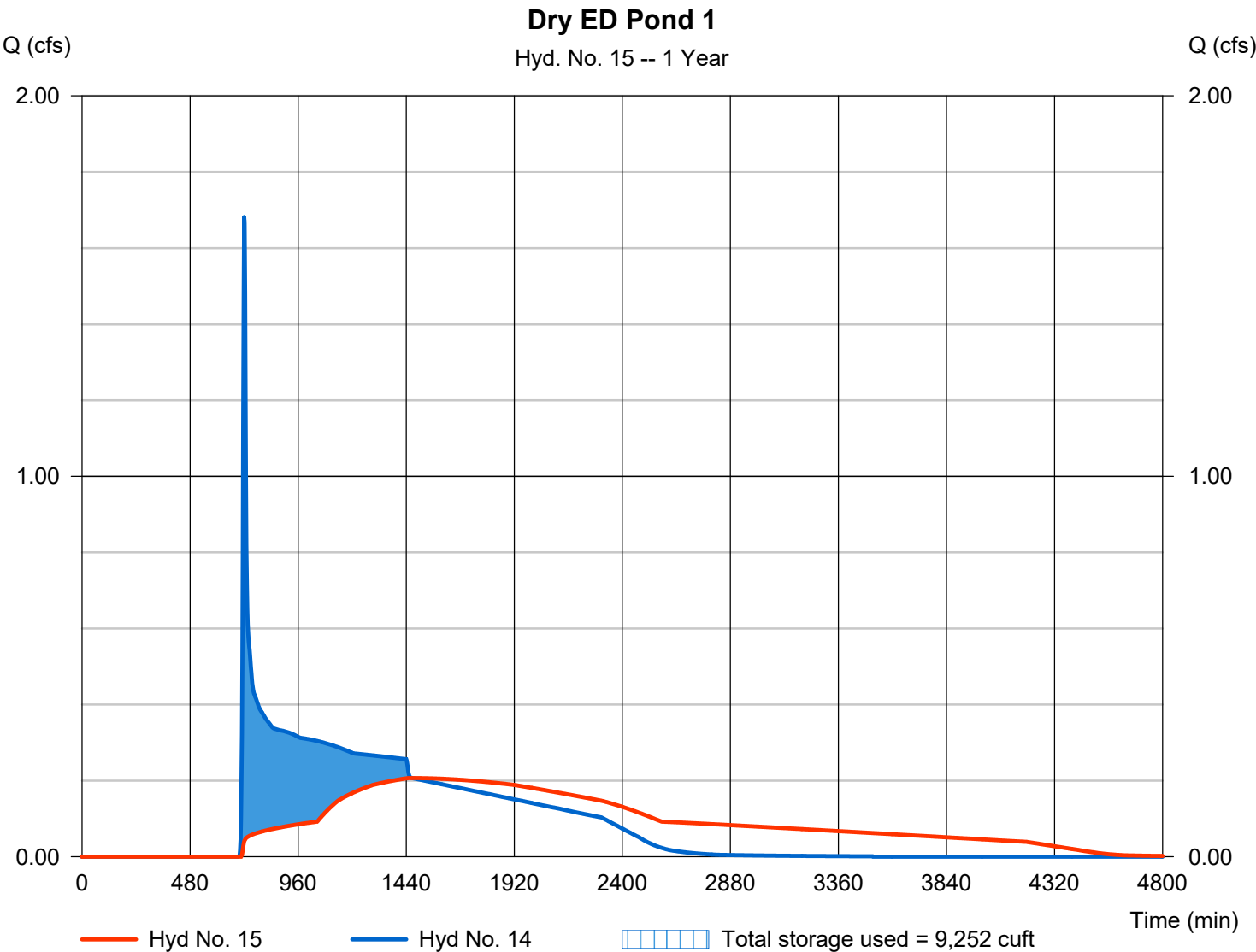
Hydrograph Report

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 0.207 cfs
Storm frequency	= 1 yrs	Time to peak	= 1468 min
Time interval	= 2 min	Hyd. volume	= 24,120 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 684.52 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 9,252 cuft

Storage Indication method used.



Pond No. 1 - Dry ED Pond 1

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Beginning Elevation = 681.50 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	681.50	00	0	0
0.50	682.00	2,257	564	564
2.50	684.00	3,909	6,166	6,730
4.50	686.00	5,787	9,696	16,426
6.50	688.00	7,891	13,678	30,104
7.50	689.00	9,029	8,460	38,564

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 30.00	1.50	2.50	0.00
Span (in)	= 30.00	1.50	2.50	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 681.00	681.50	684.00	0.00
Length (ft)	= 29.00	0.00	0.00	0.00
Slope (%)	= 1.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 2.00	6.57	0.00	0.00
Crest El. (ft)	= 685.00	687.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	Rect	---	---
Multi-Stage	= Yes	Yes	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	681.50	0.00	0.00	0.00	---	0.00	0.00	---	---	---	---	0.000
0.50	564	682.00	1.76 ic	0.04 ic	0.00	---	0.00	0.00	---	---	---	---	0.039
2.50	6,730	684.00	1.76 ic	0.09 ic	0.00	---	0.00	0.00	---	---	---	---	0.092
4.50	16,426	686.00	7.15 oc	0.12 ic	0.23 ic	---	6.66	0.00	---	---	---	---	7.002
6.50	30,104	688.00	49.95 ic	0.07 ic	0.19 ic	---	27.82 s	21.88	---	---	---	---	49.95
7.50	38,564	689.00	59.84 ic	0.03 ic	0.10 ic	---	23.85 s	35.86 s	---	---	---	---	59.84

Hydrograph Report

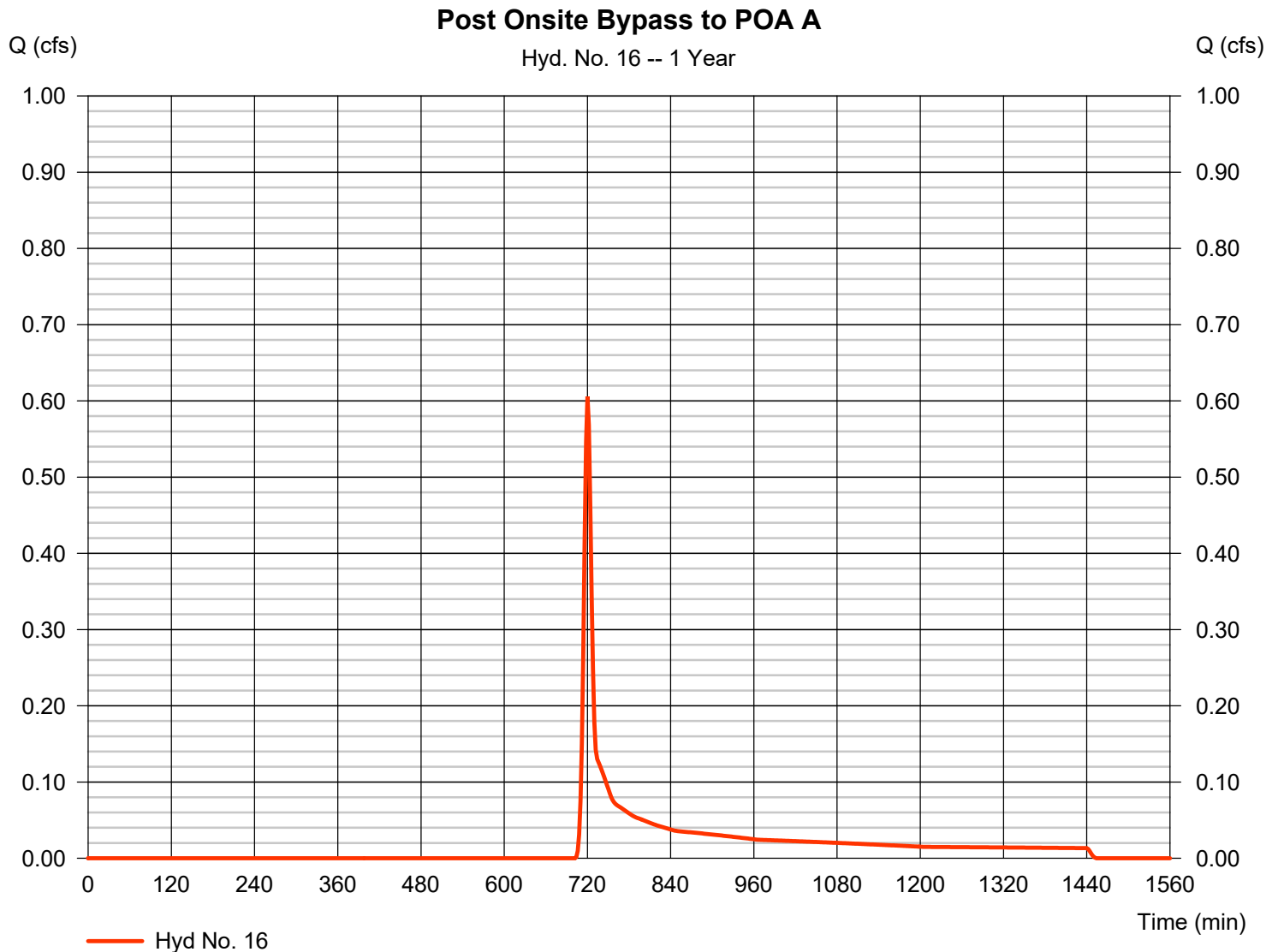
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Wednesday, 06 / 3 / 2026

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 0.606 cfs
Storm frequency	= 1 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 1,617 cuft
Drainage area	= 0.770 ac	Curve number	= 63
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

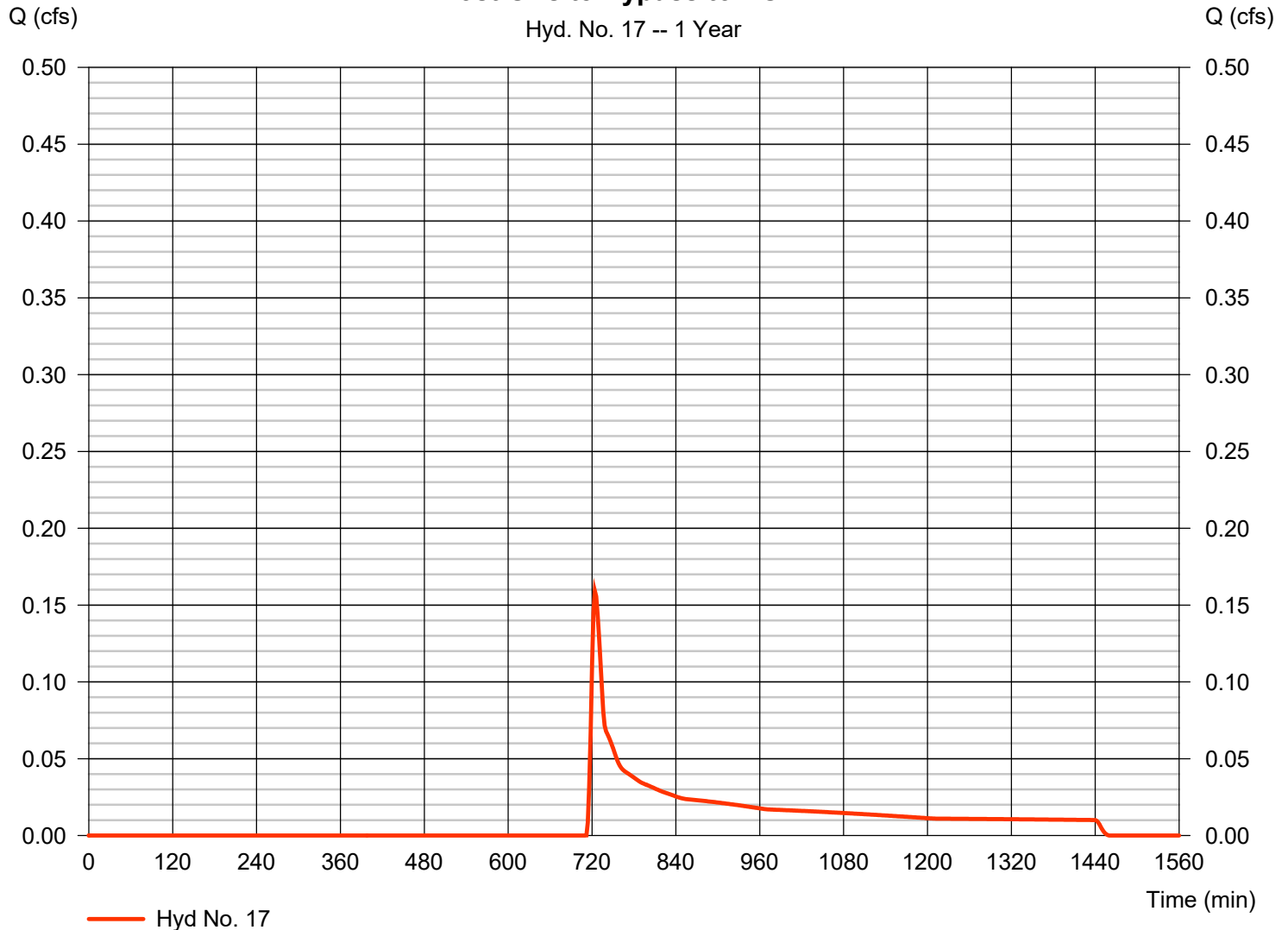
Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.159 cfs
Storm frequency	=	1 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	911 cuft
Drainage area	=	0.840 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

Post Offsite Bypass to POA A



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

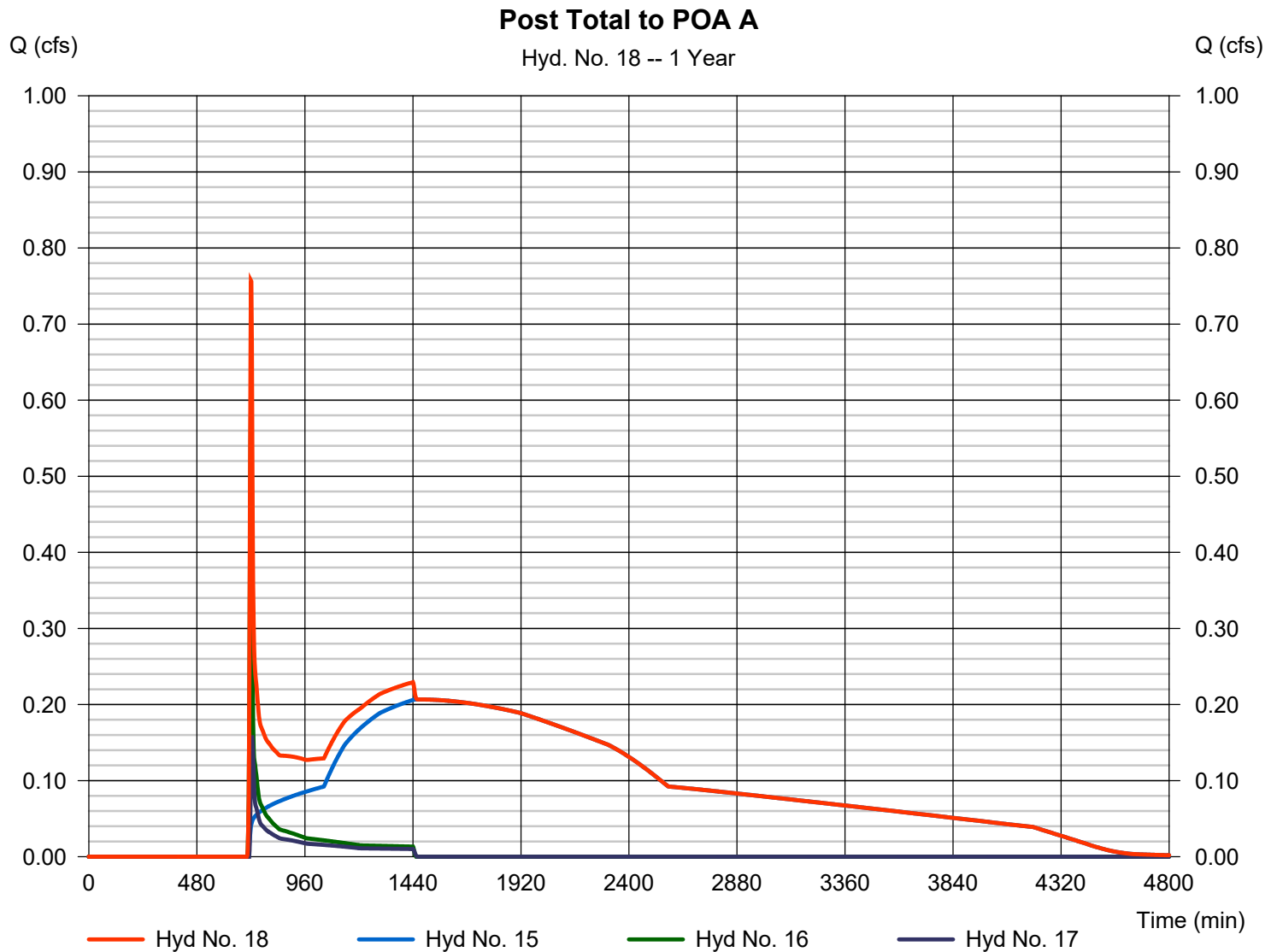
Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type = Combine
Storm frequency = 1 yrs
Time interval = 2 min
Inflow hyds. = 15, 16, 17

Peak discharge = 0.757 cfs
Time to peak = 720 min
Hyd. volume = 26,648 cuft
Contrib. drain. area = 1.610 ac



Hydrograph Report

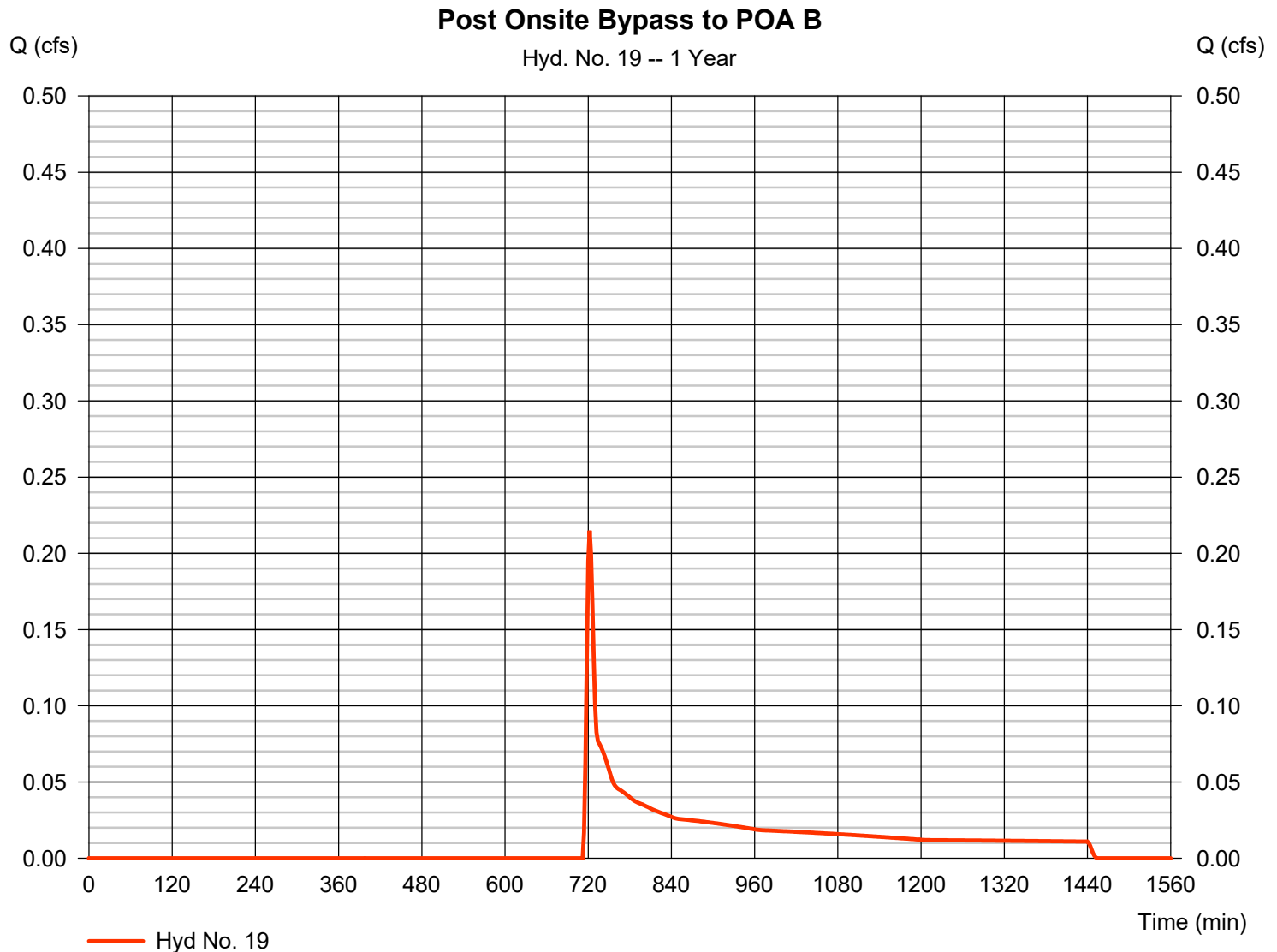
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 0.215 cfs
Storm frequency	= 1 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 989 cuft
Drainage area	= 0.940 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

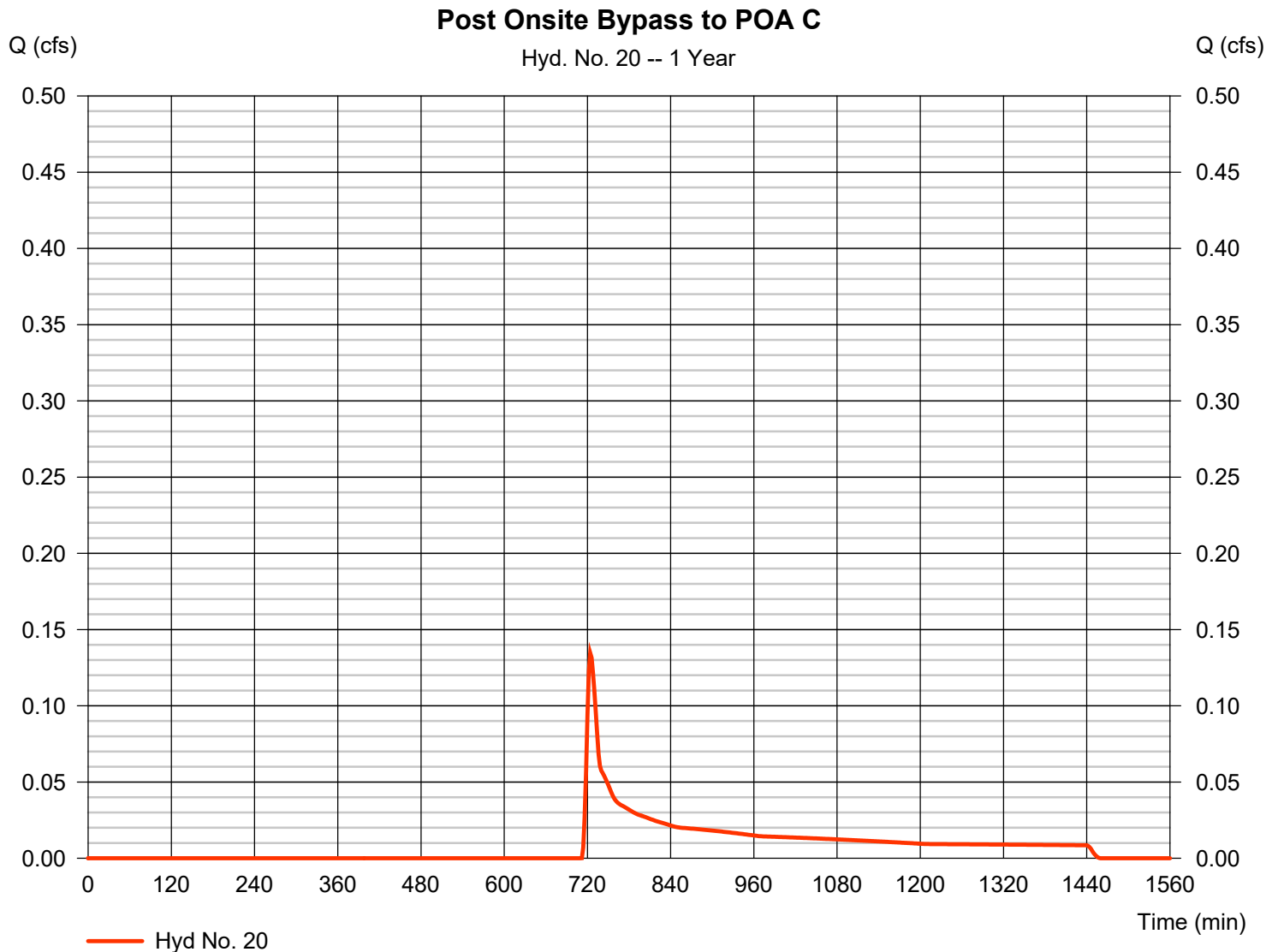
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 0.135 cfs
Storm frequency	= 1 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 770 cuft
Drainage area	= 0.710 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

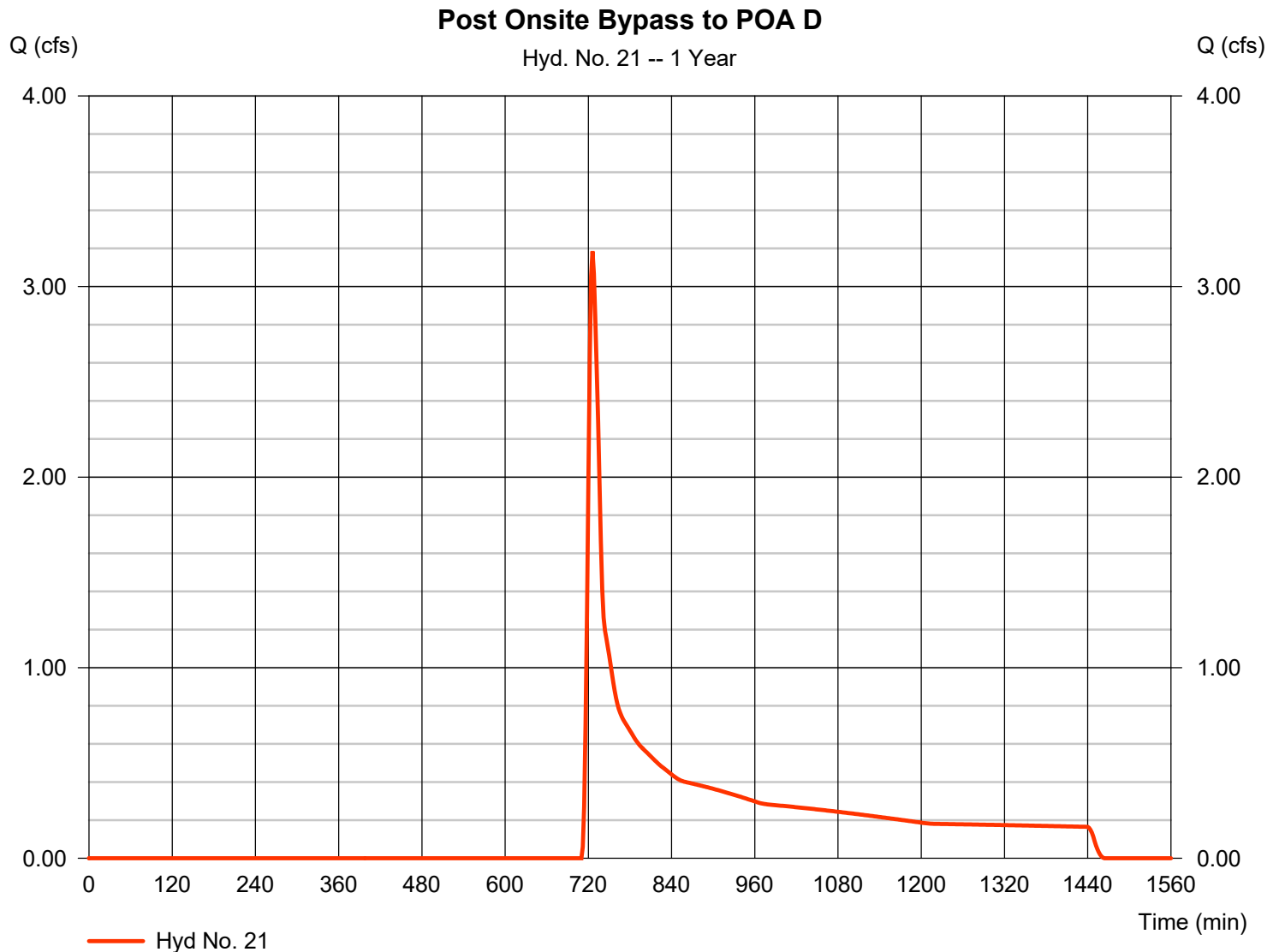
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 3.185 cfs
Storm frequency	= 1 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 16,216 cuft
Drainage area	= 12.950 ac	Curve number	= 57
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 15.00 min
Total precip.	= 3.33 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

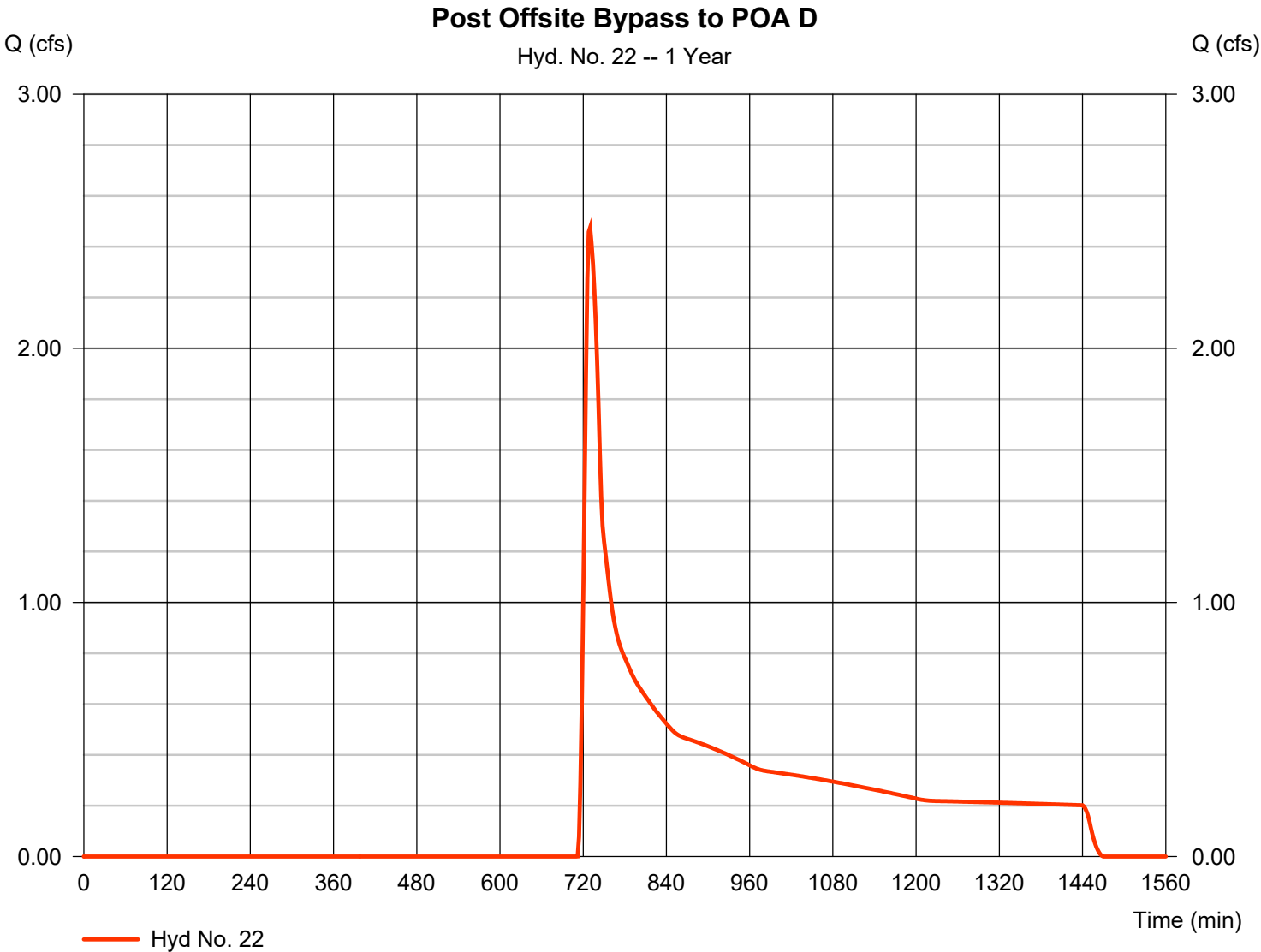
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.475 cfs
Storm frequency	=	1 yrs	Time to peak	=	730 min
Time interval	=	2 min	Hyd. volume	=	18,187 cuft
Drainage area	=	17.290 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	17.00 min
Total precip.	=	3.33 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

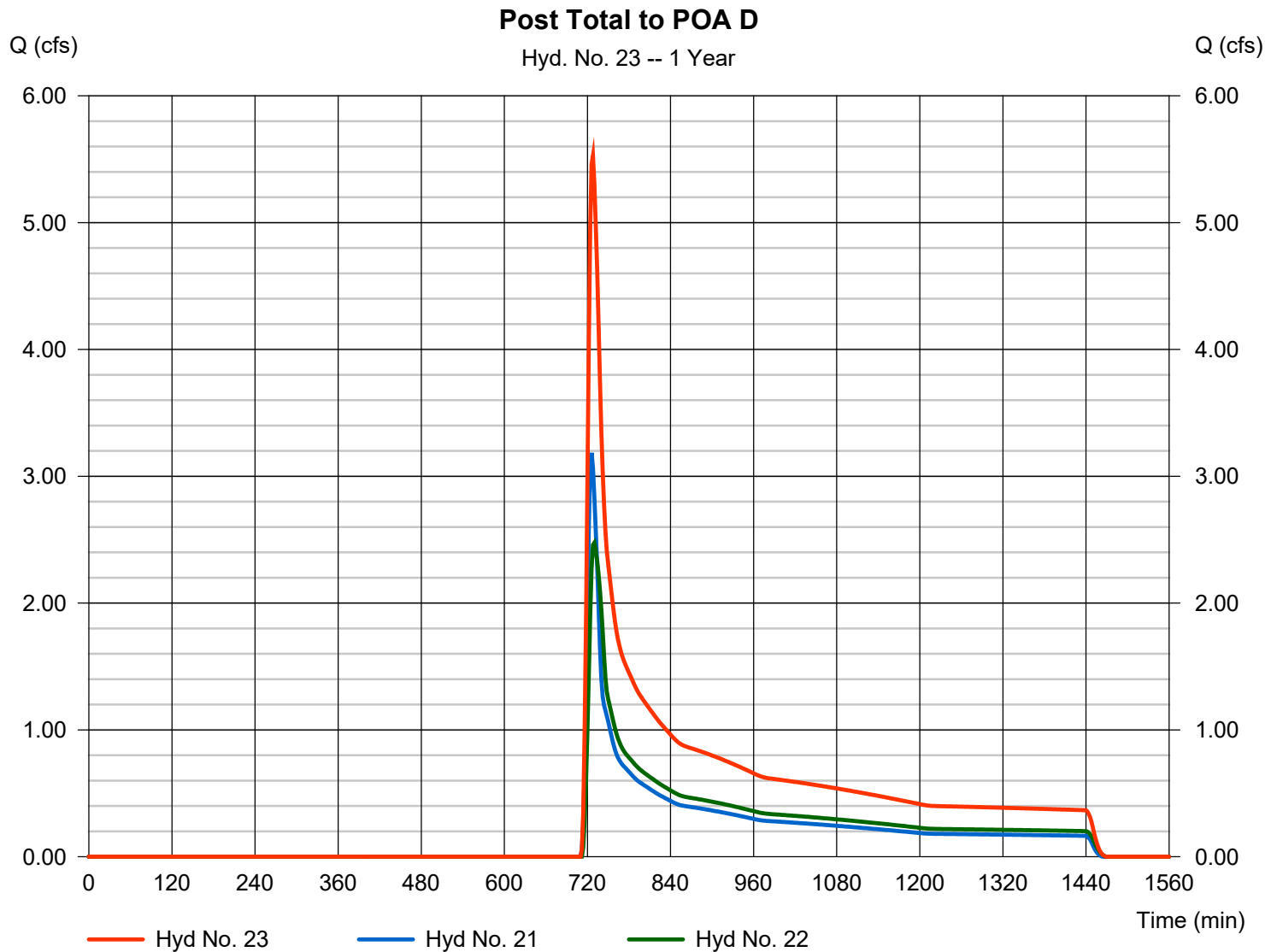
Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type = Combine
Storm frequency = 1 yrs
Time interval = 2 min
Inflow hyds. = 21, 22

Peak discharge = 5.526 cfs
Time to peak = 728 min
Hyd. volume = 34,403 cuft
Contrib. drain. area = 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.974	2	722	3,344	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	1.052	2	724	4,381	-----	-----	-----	Pre Offsite to POA A
3	Combine	1.997	2	722	7,725	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	2.640	2	724	10,995	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	0.544	2	724	2,266	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	4.487	2	726	20,822	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	5.208	2	728	27,931	-----	-----	-----	Pre Offsite to POA D
8	Combine	9.532	2	726	48,753	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	12.43	2	718	25,021	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	0.405	2	898	24,995	10	695.89	13,695	Dry ED Pond 2
12	SCS Runoff	1.830	2	720	4,427	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	0.712	2	724	2,965	-----	-----	-----	Post Offsite to Pond 1
14	Combine	2.609	2	720	32,387	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	0.341	2	1226	32,371	14	685.03	11,699	Dry ED Pond 1
16	SCS Runoff	0.898	2	720	2,238	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	0.336	2	724	1,399	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	1.222	2	720	36,009	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	0.442	2	722	1,519	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	0.284	2	724	1,183	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	5.871	2	726	24,088	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	5.208	2	728	27,931	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	10.92	2	726	52,019	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 2 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

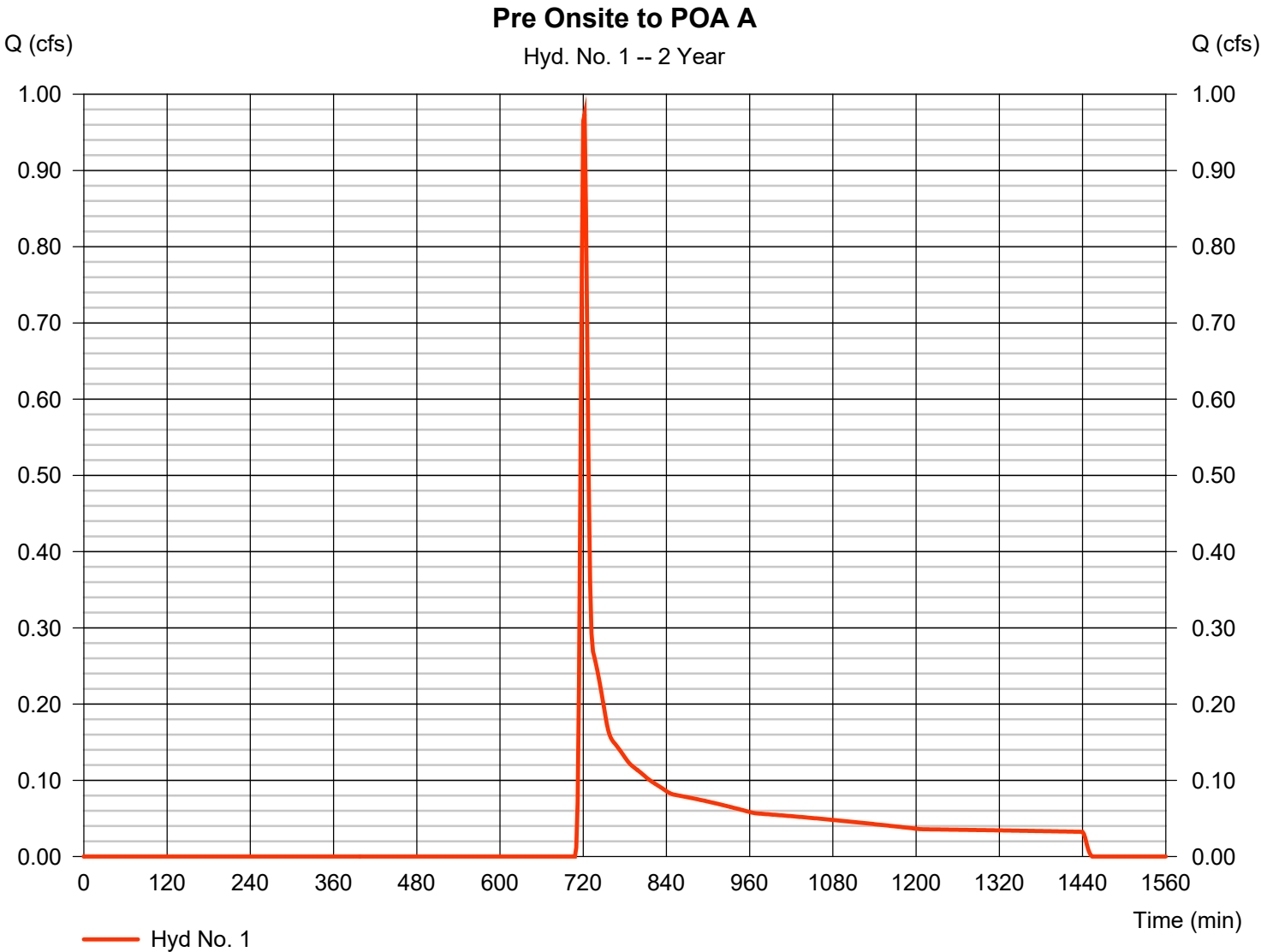
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.974 cfs
Storm frequency	=	2 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	3,344 cuft
Drainage area	=	2.070 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	9.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

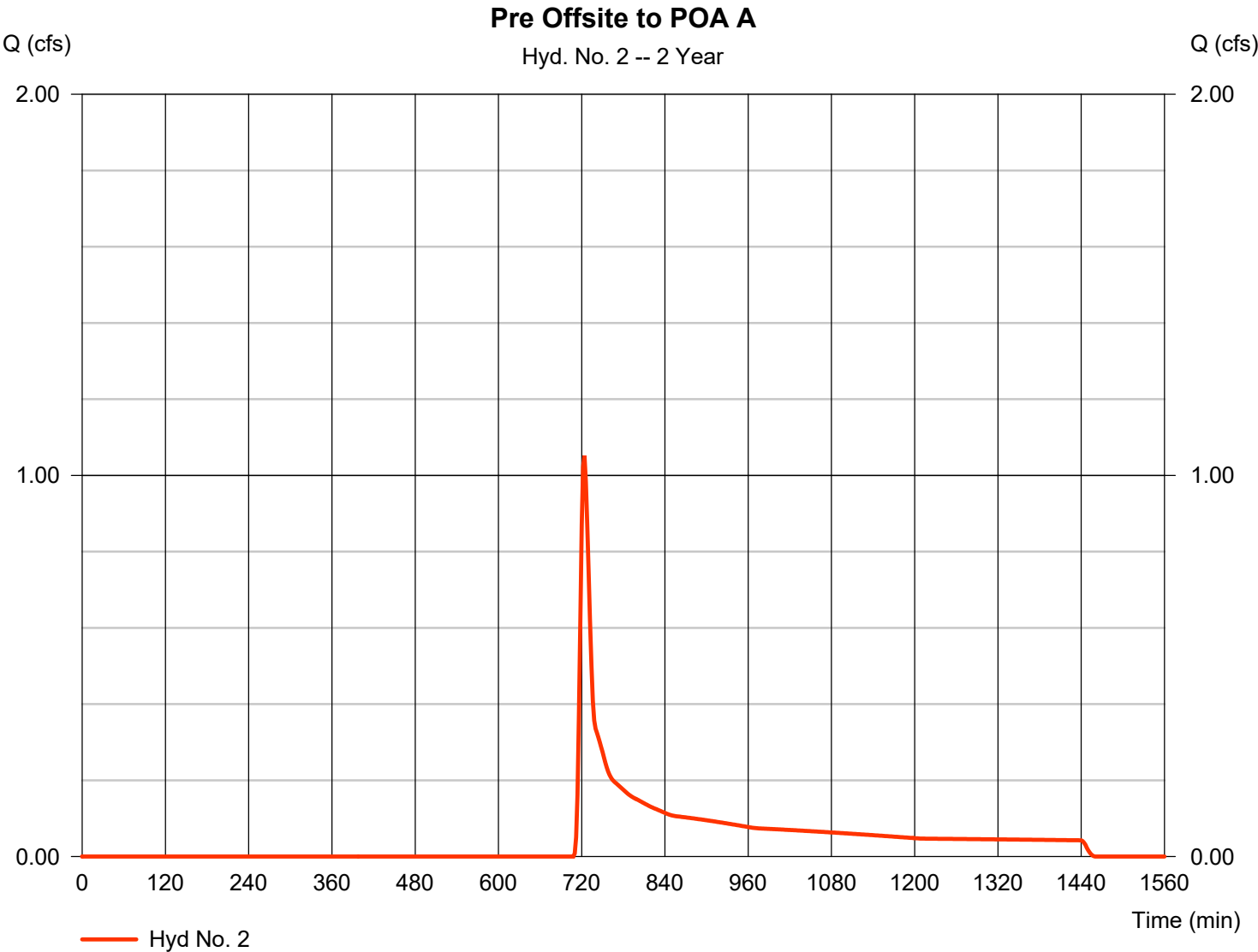
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 1.052 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 4,381 cuft
Drainage area	= 2.630 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

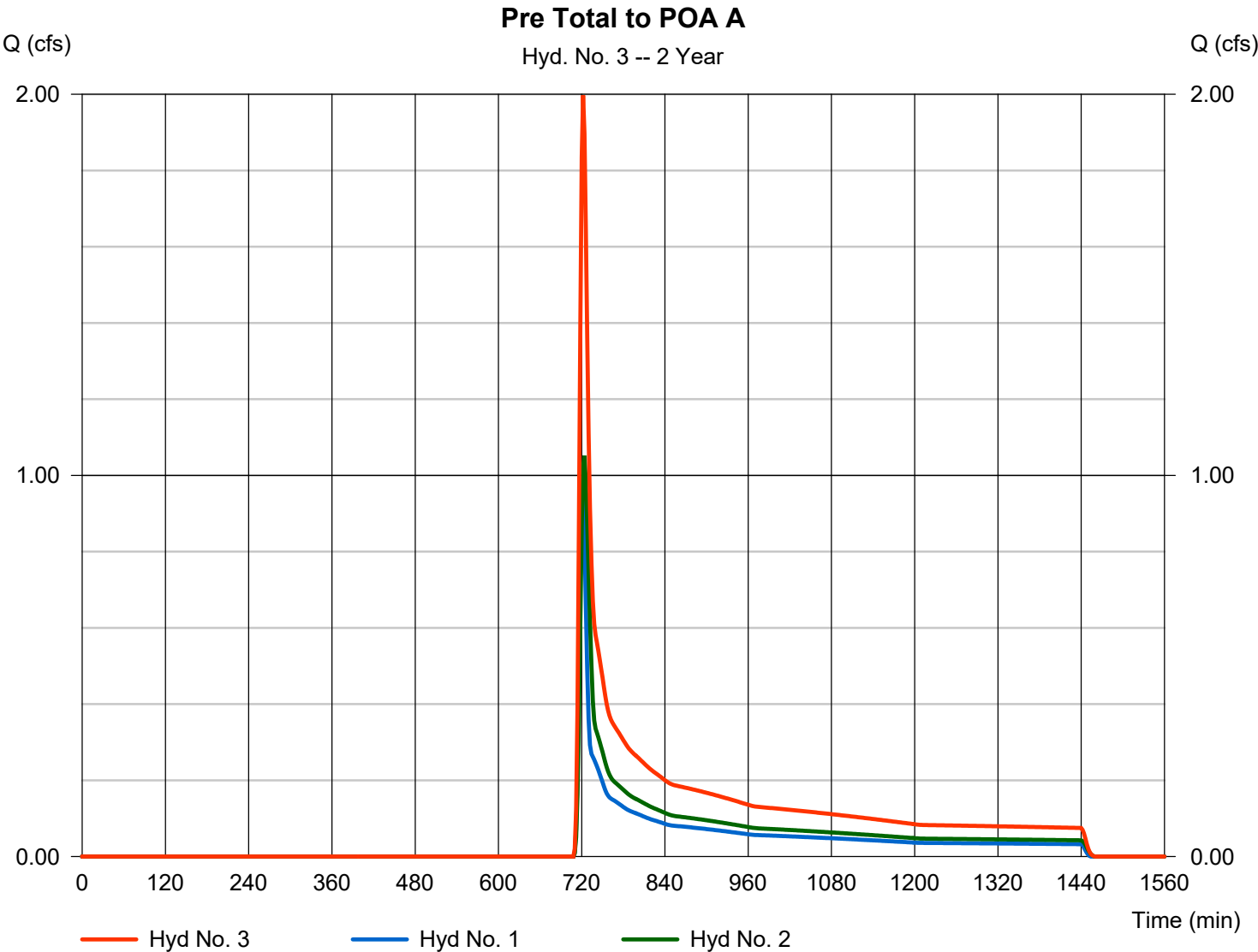
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 3

Pre Total to POA A

Hydrograph type	= Combine	Peak discharge	= 1.997 cfs
Storm frequency	= 2 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 7,725 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 4.700 ac

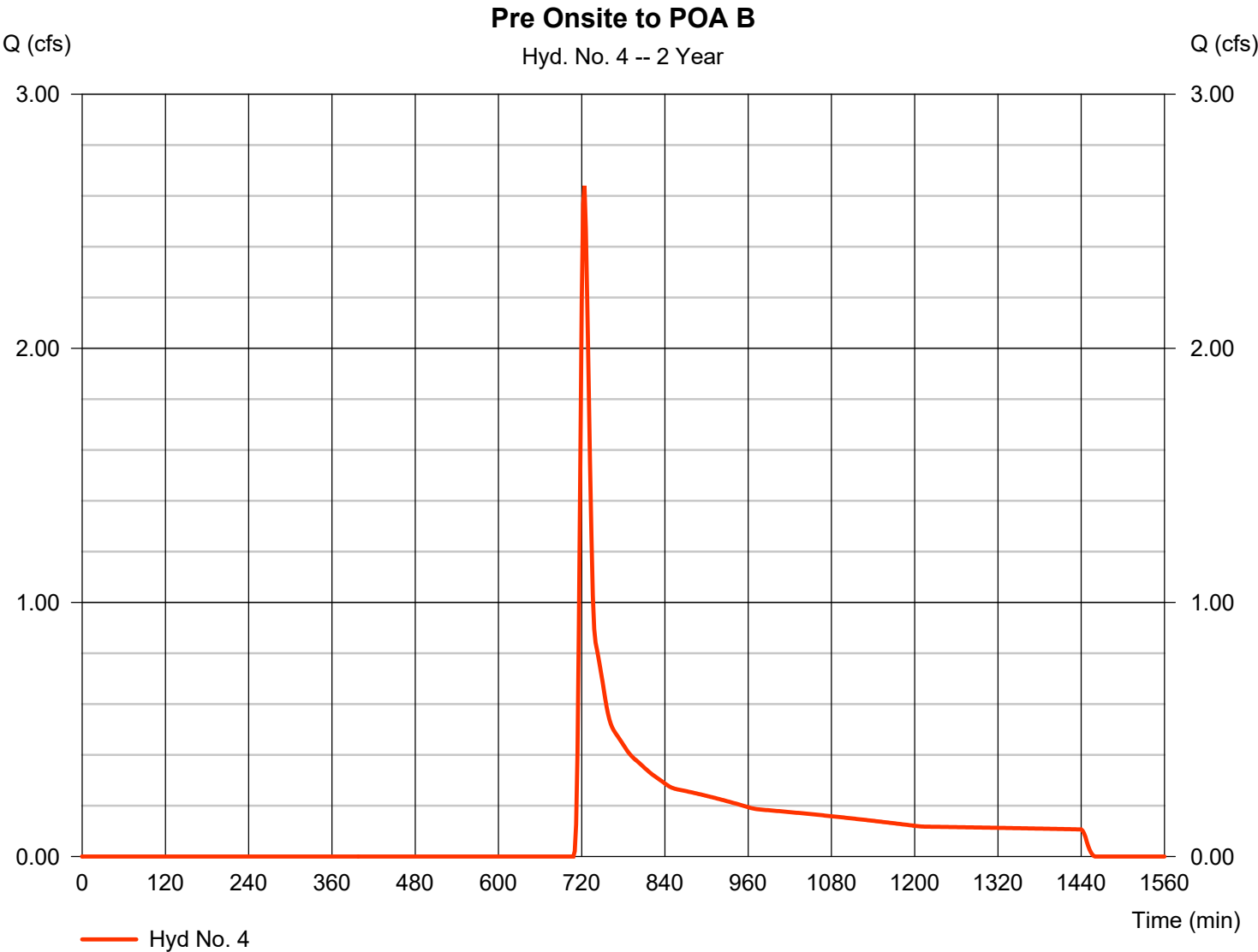


Hydrograph Report

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 2.640 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 10,995 cuft
Drainage area	= 6.600 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

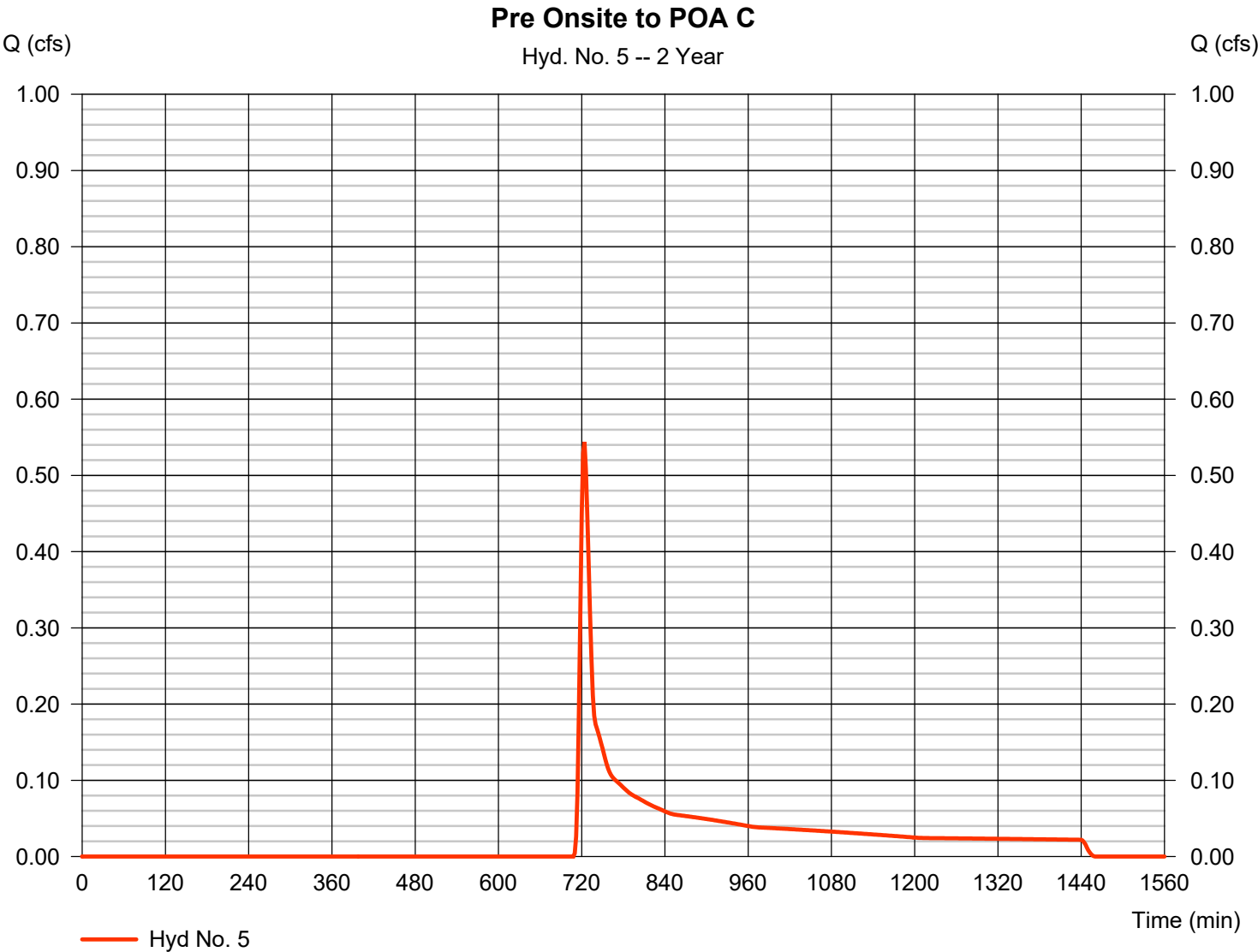


Hydrograph Report

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.544 cfs
Storm frequency	=	2 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	2,266 cuft
Drainage area	=	1.360 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

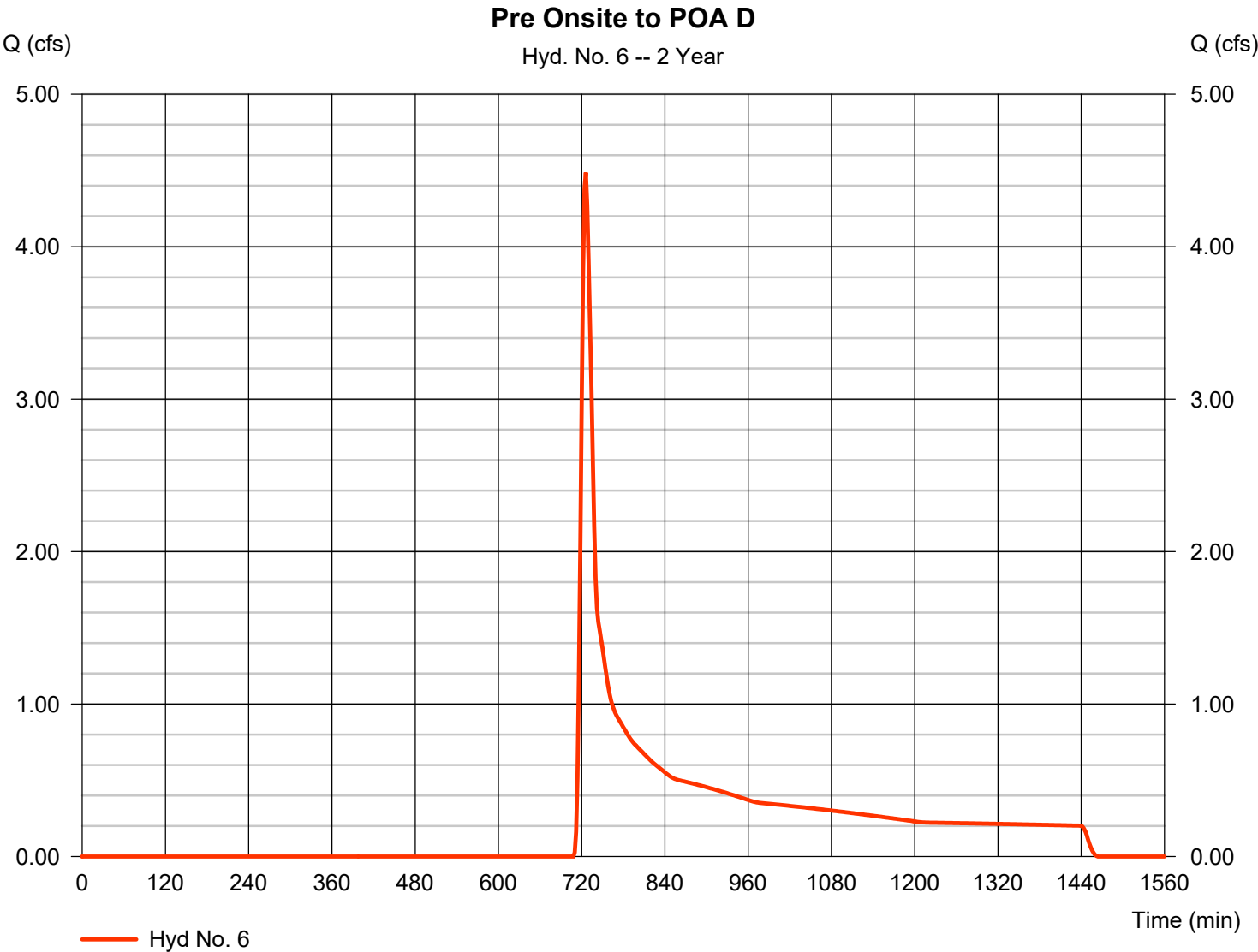


Hydrograph Report

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 4.487 cfs
Storm frequency	= 2 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 20,822 cuft
Drainage area	= 13.220 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 15.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

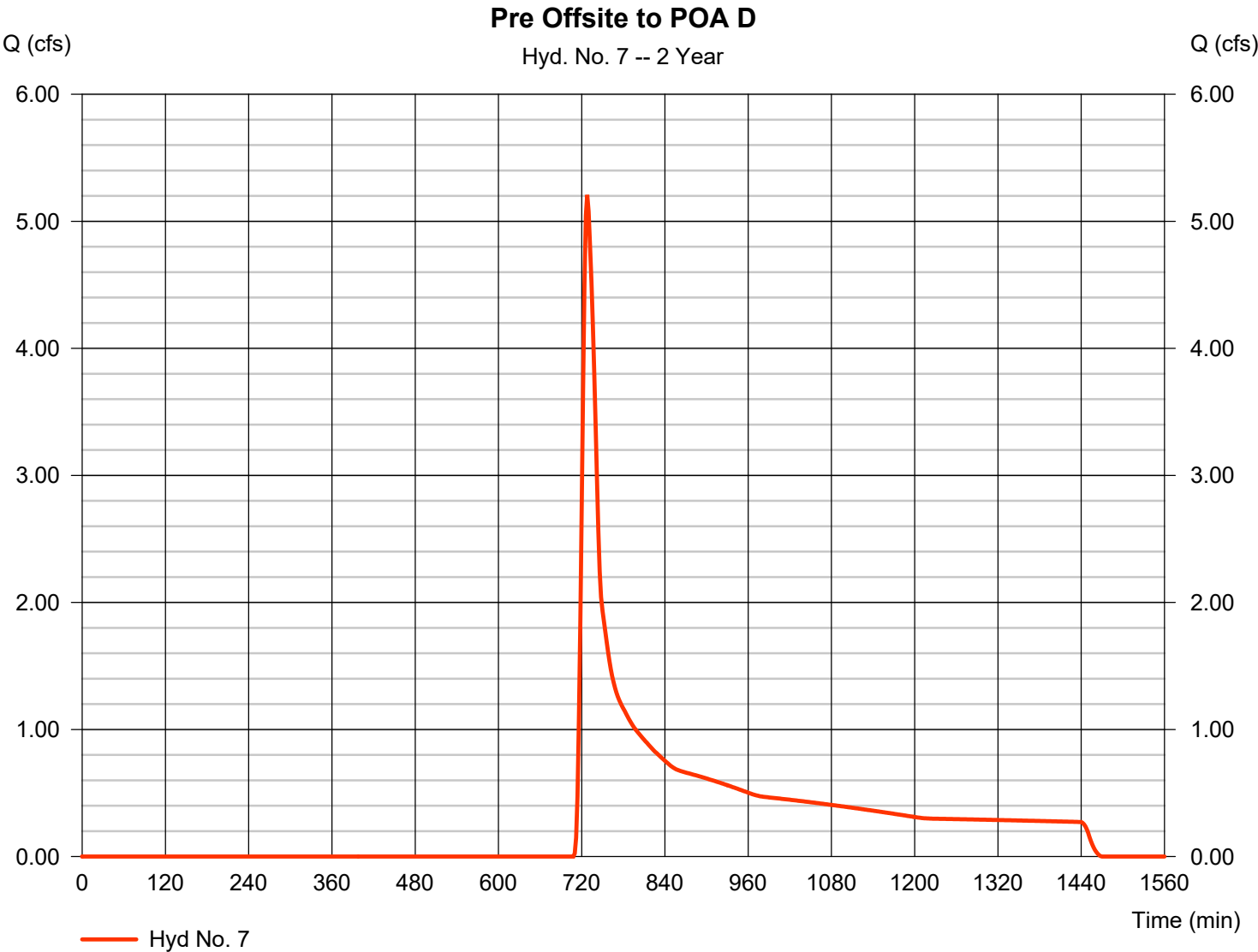


Hydrograph Report

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.208 cfs
Storm frequency	=	2 yrs	Time to peak	=	728 min
Time interval	=	2 min	Hyd. volume	=	27,931 cuft
Drainage area	=	17.290 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	17.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

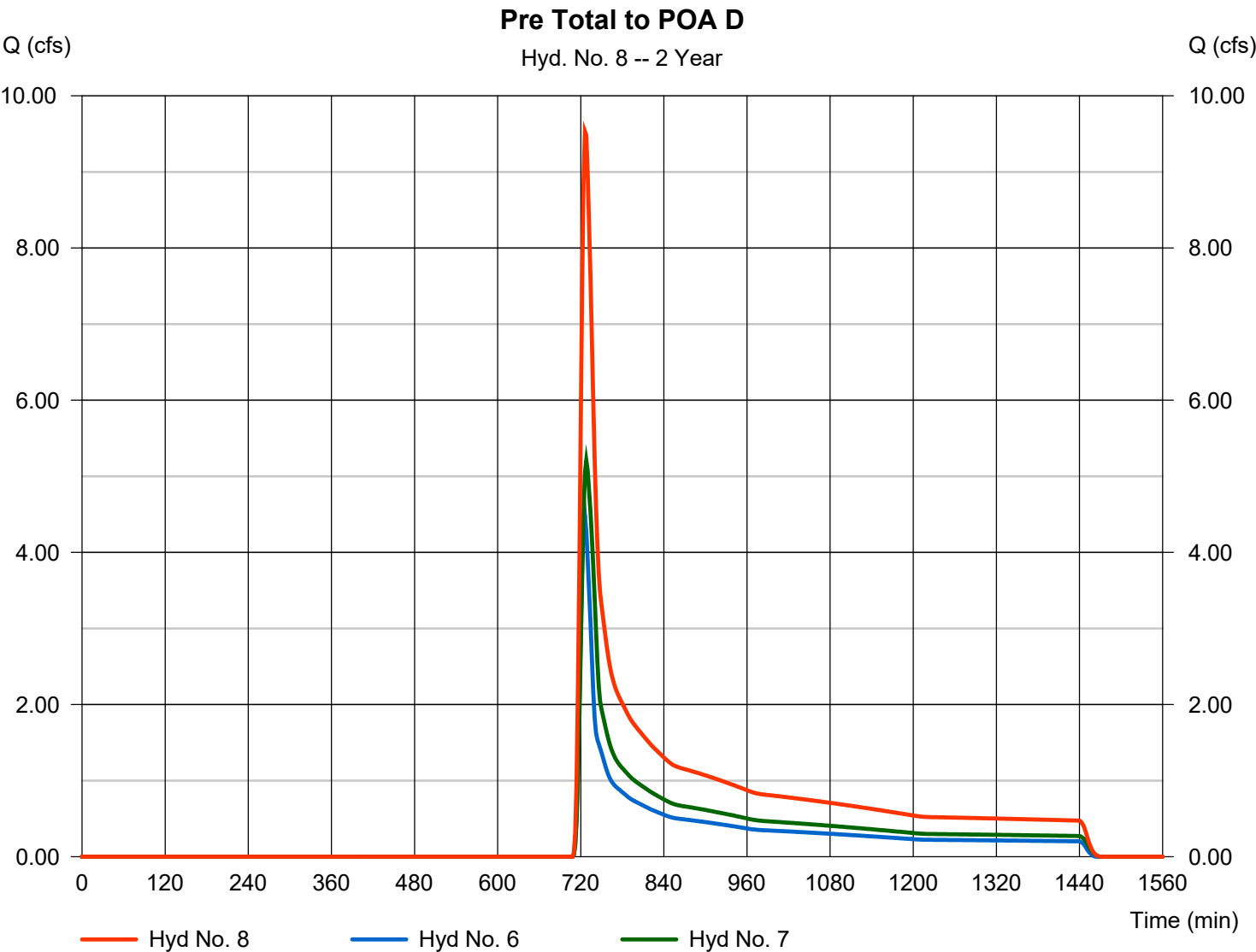
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Wednesday, 06 / 3 / 2026

Hyd. No. 8

Pre Total to POA D

Hydrograph type	= Combine	Peak discharge	= 9.532 cfs
Storm frequency	= 2 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 48,753 cuft
Inflow hyds.	= 6, 7	Contrib. drain. area	= 30.510 ac

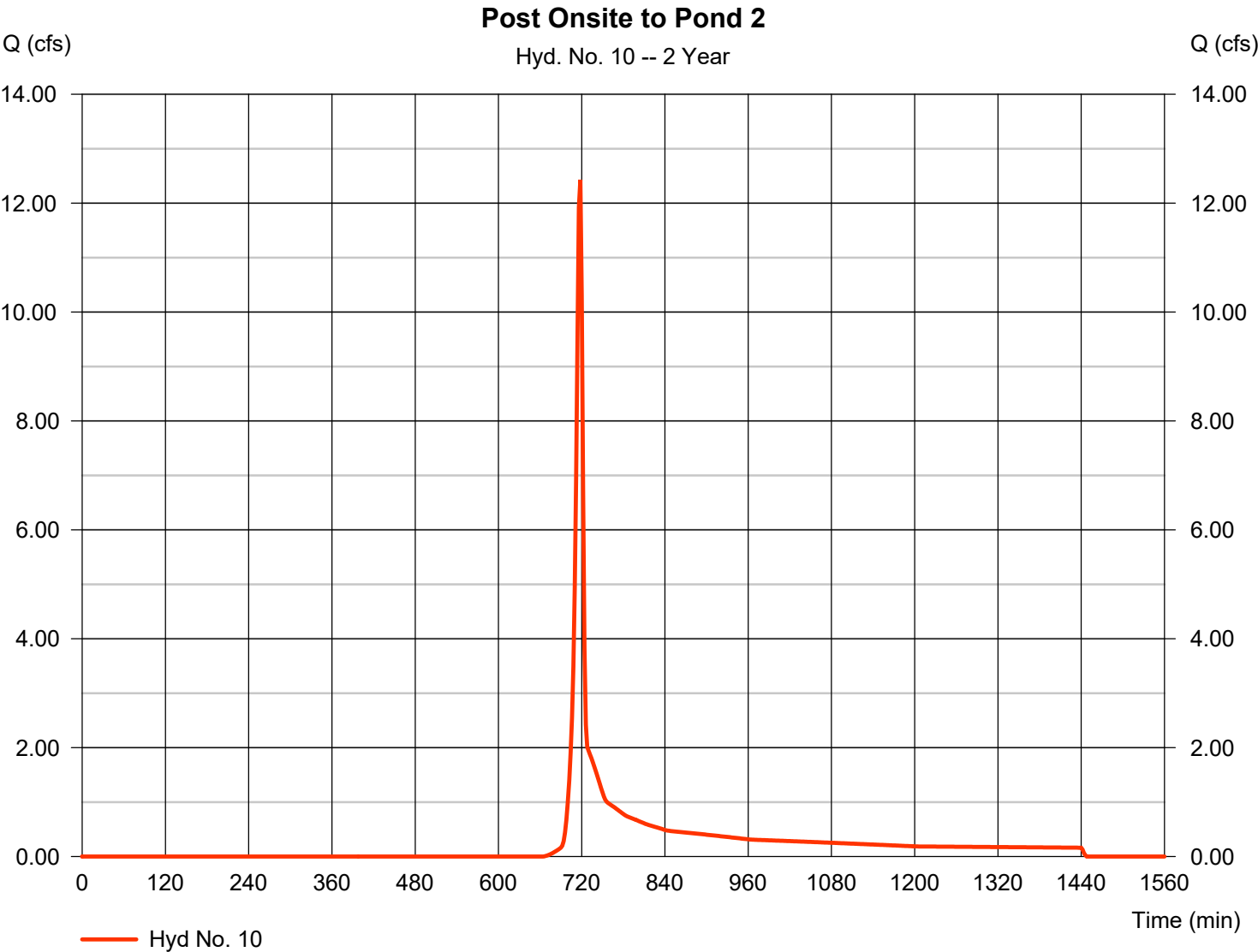


Hydrograph Report

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	=	SCS Runoff	Peak discharge	=	12.43 cfs
Storm frequency	=	2 yrs	Time to peak	=	718 min
Time interval	=	2 min	Hyd. volume	=	25,021 cuft
Drainage area	=	6.530 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	5.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

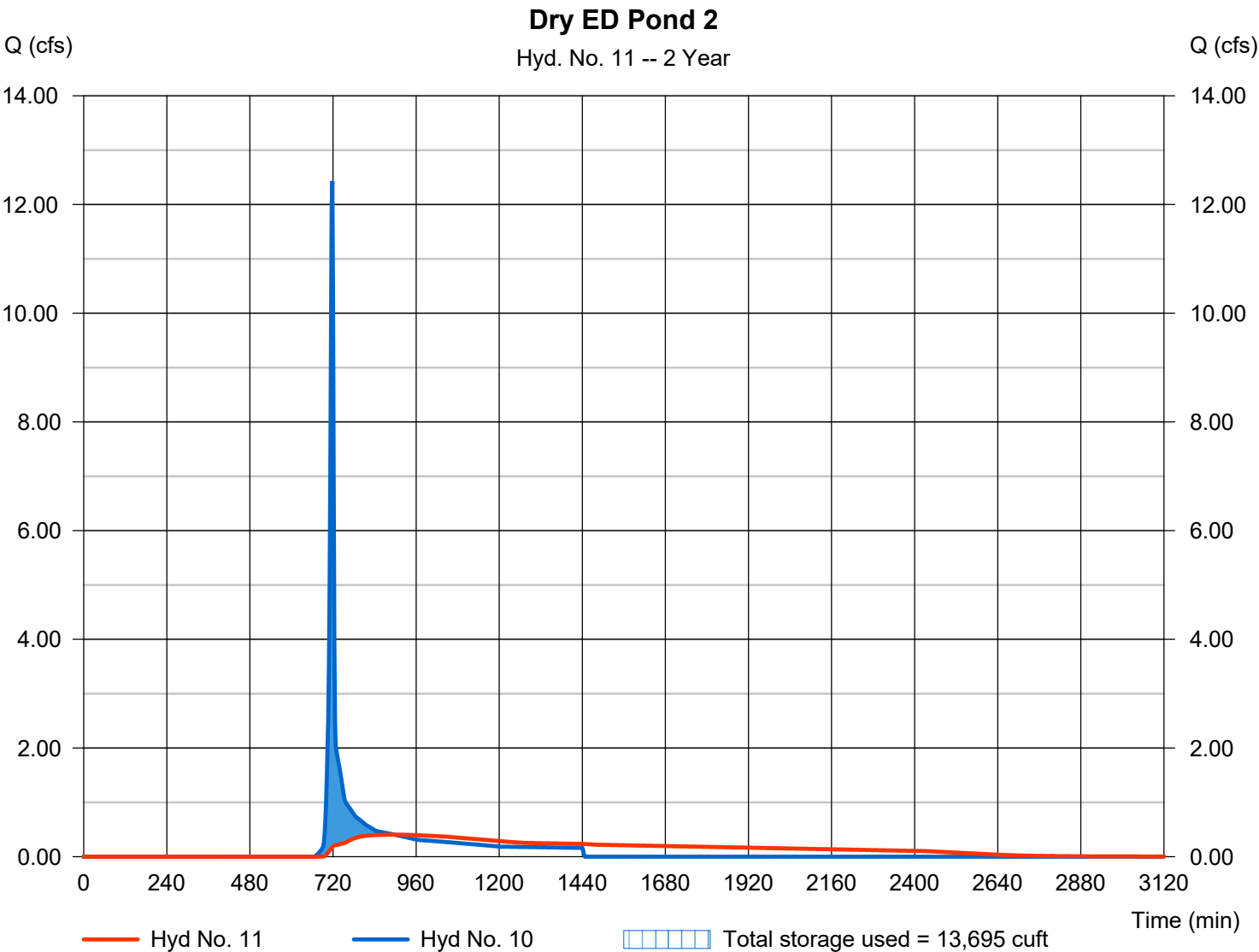
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 0.405 cfs
Storm frequency	= 2 yrs	Time to peak	= 898 min
Time interval	= 2 min	Hyd. volume	= 24,995 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 695.89 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 13,695 cuft

Storage Indication method used.

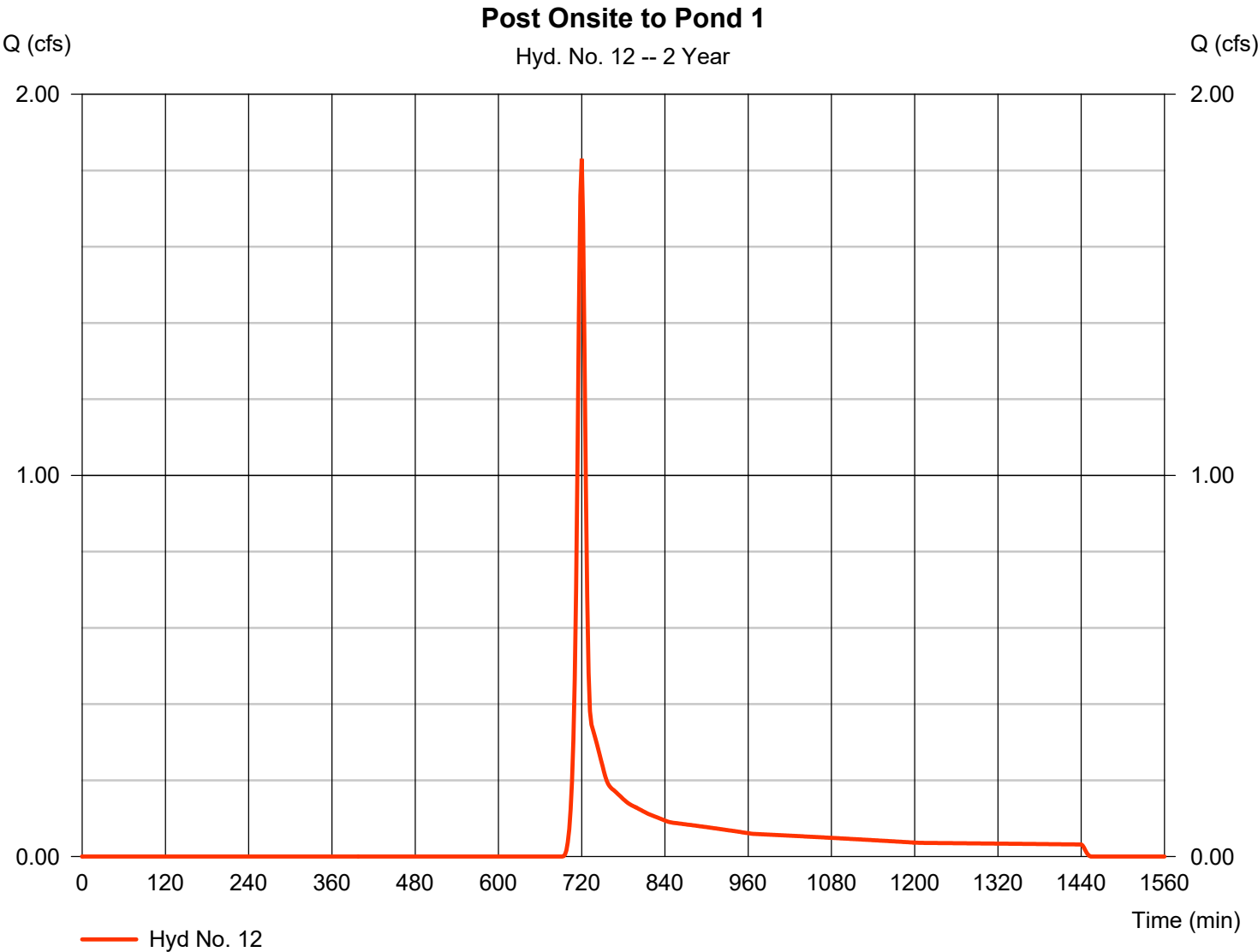


Hydrograph Report

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.830 cfs
Storm frequency	= 2 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 4,427 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

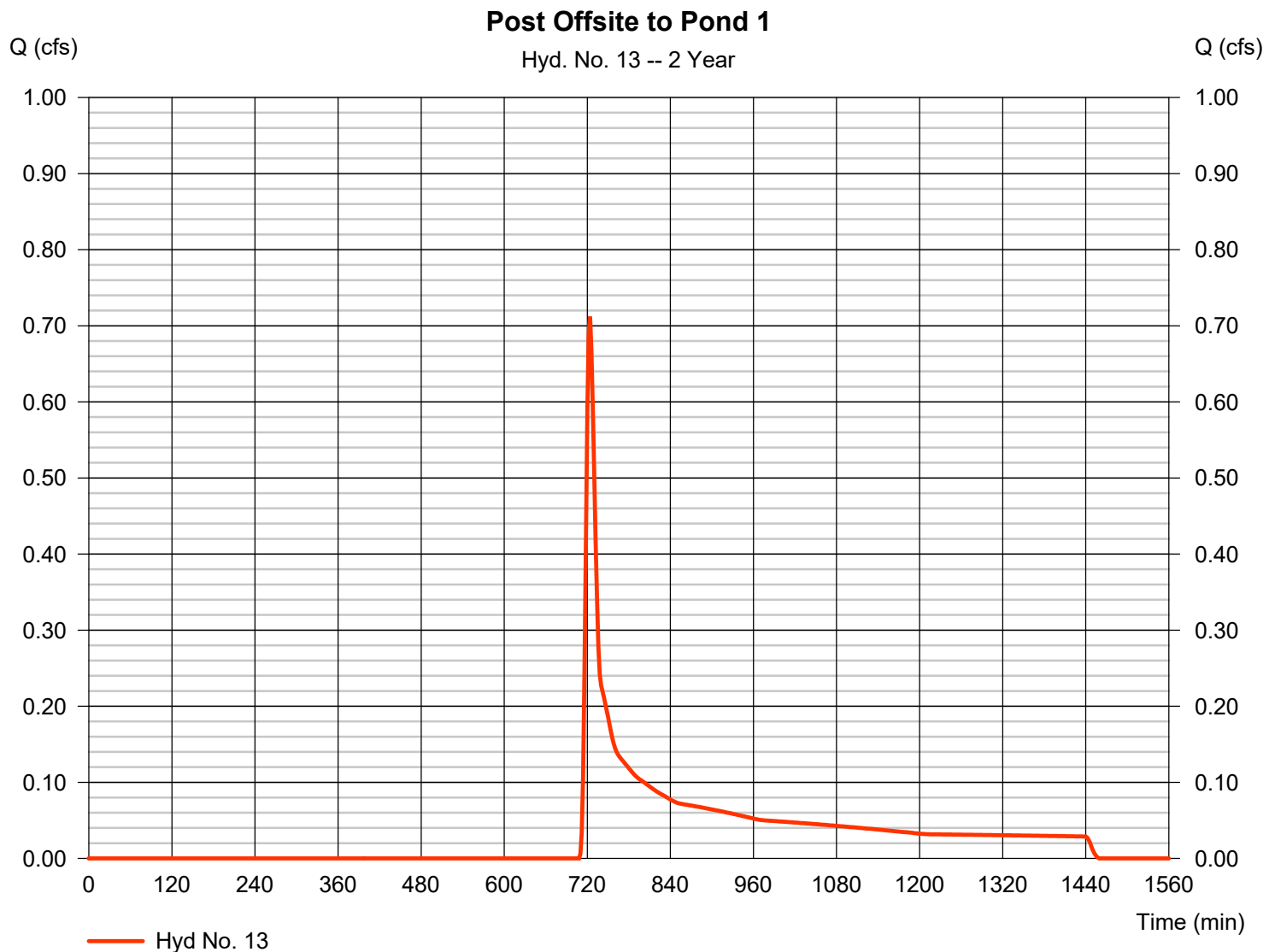
Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type = SCS Runoff
 Storm frequency = 2 yrs
 Time interval = 2 min
 Drainage area = 1.780 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 3.78 in
 Storm duration = 24 hrs

Peak discharge = 0.712 cfs
 Time to peak = 724 min
 Hyd. volume = 2,965 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 12.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

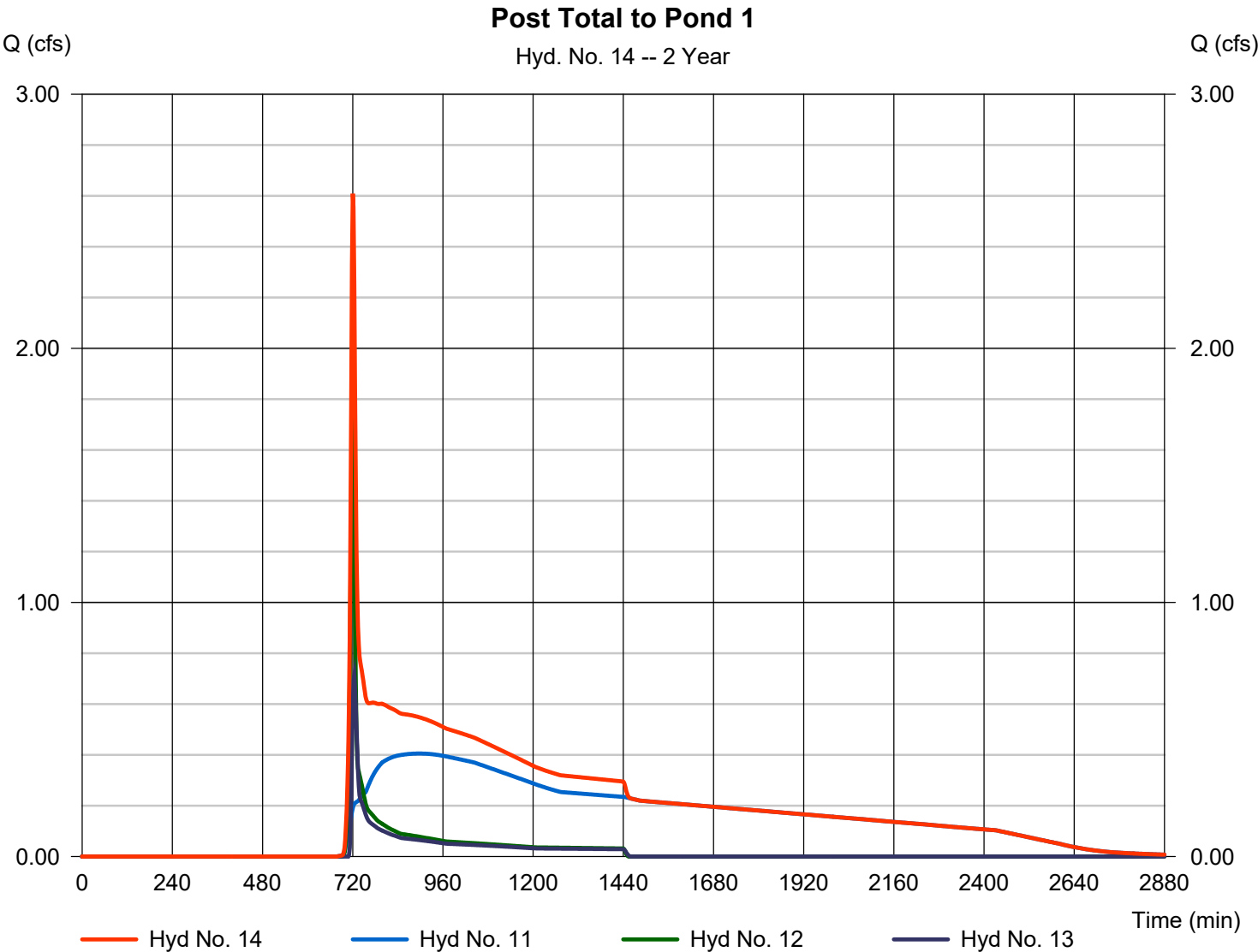
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 14

Post Total to Pond 1

Hydrograph type	= Combine	Peak discharge	= 2.609 cfs
Storm frequency	= 2 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 32,387 cuft
Inflow hyds.	= 11, 12, 13	Contrib. drain. area	= 3.130 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

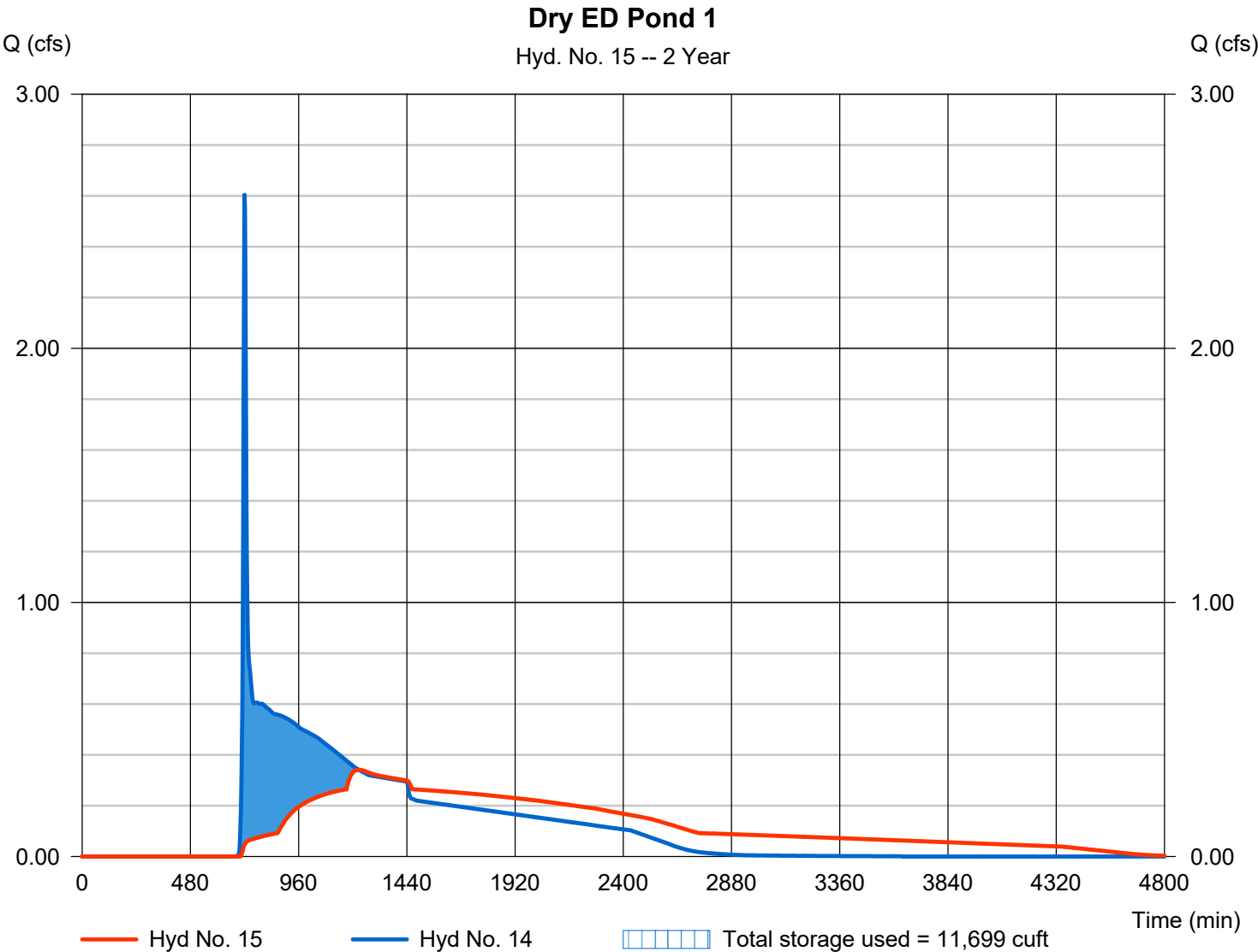
Wednesday, 06 / 3 / 2026

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 0.341 cfs
Storm frequency	= 2 yrs	Time to peak	= 1226 min
Time interval	= 2 min	Hyd. volume	= 32,371 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 685.03 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 11,699 cuft

Storage Indication method used.



Hydrograph Report

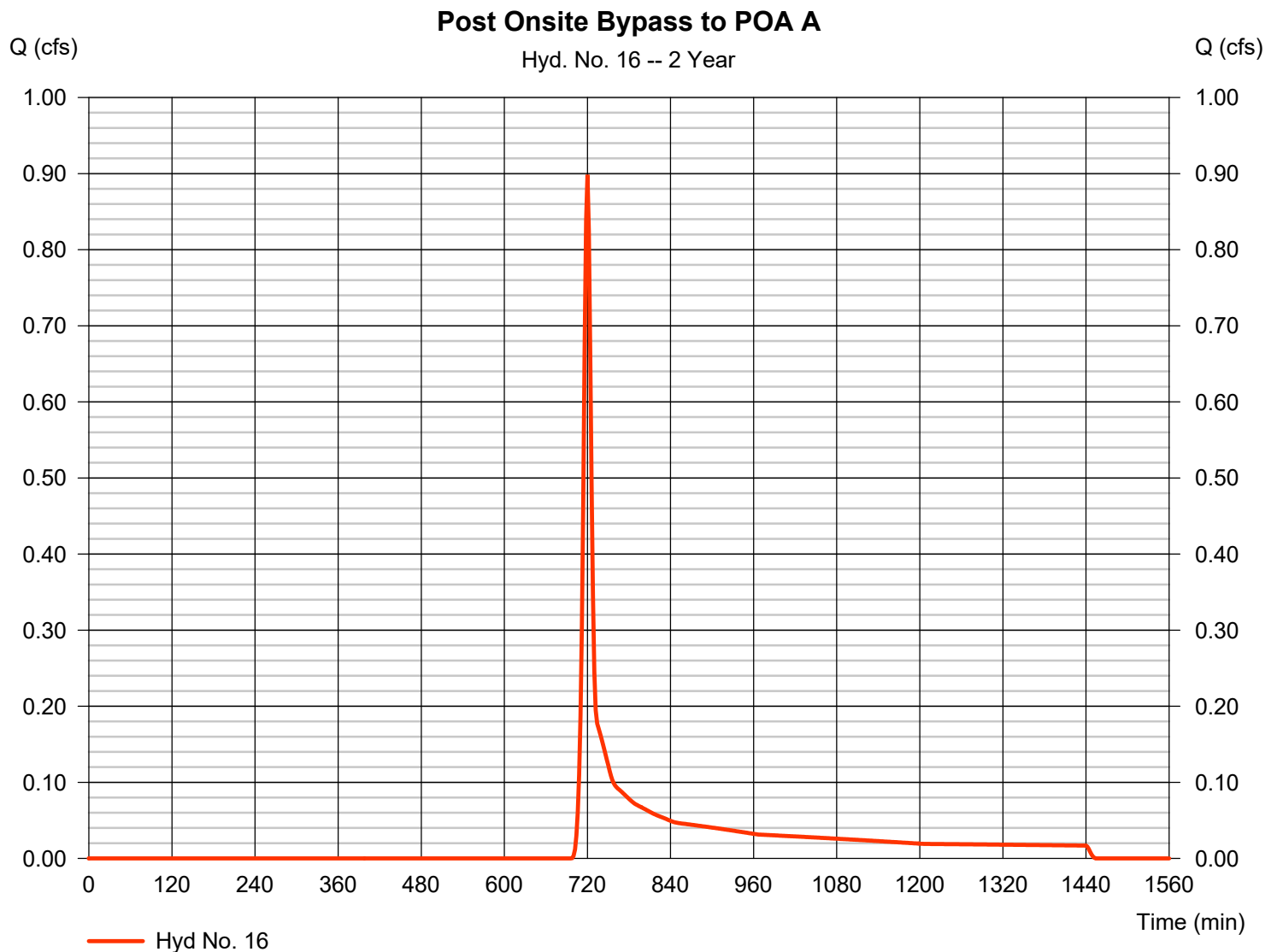
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 0.898 cfs
Storm frequency	= 2 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 2,238 cuft
Drainage area	= 0.770 ac	Curve number	= 63
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

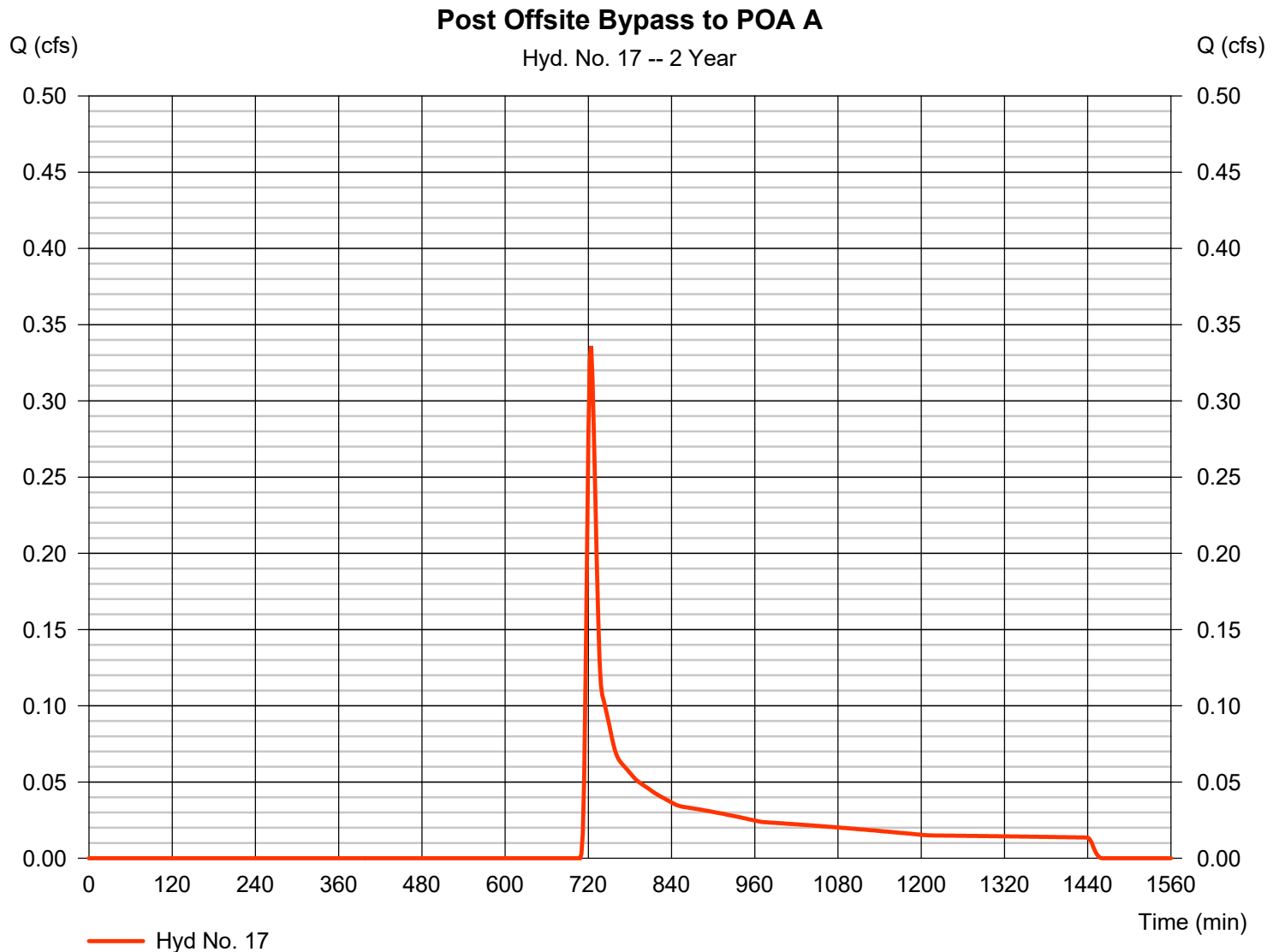
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 0.336 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 1,399 cuft
Drainage area	= 0.840 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 3.78 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

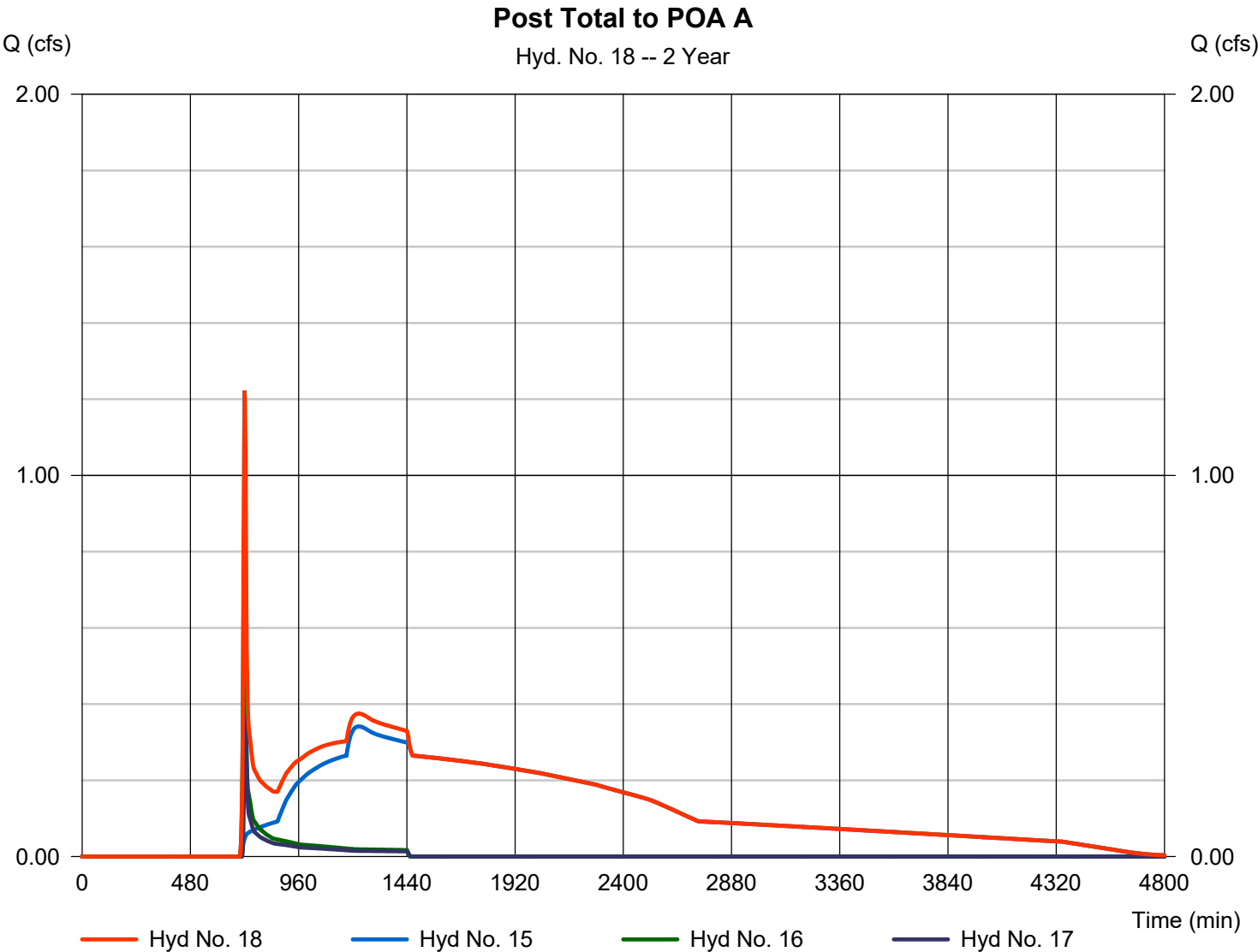
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type	= Combine	Peak discharge	= 1.222 cfs
Storm frequency	= 2 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 36,009 cuft
Inflow hyds.	= 15, 16, 17	Contrib. drain. area	= 1.610 ac



Hydrograph Report

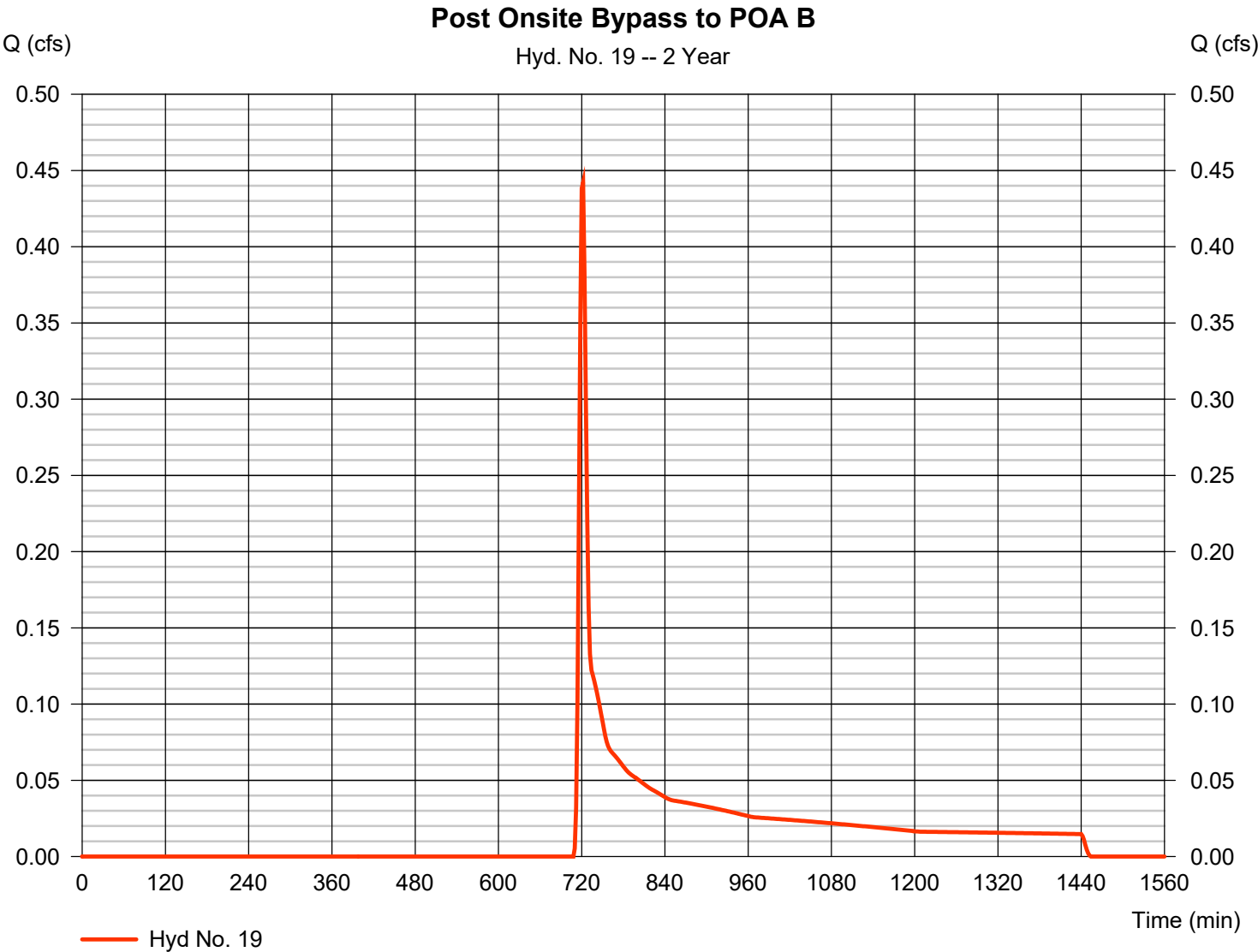
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.442 cfs
Storm frequency	=	2 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	1,519 cuft
Drainage area	=	0.940 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

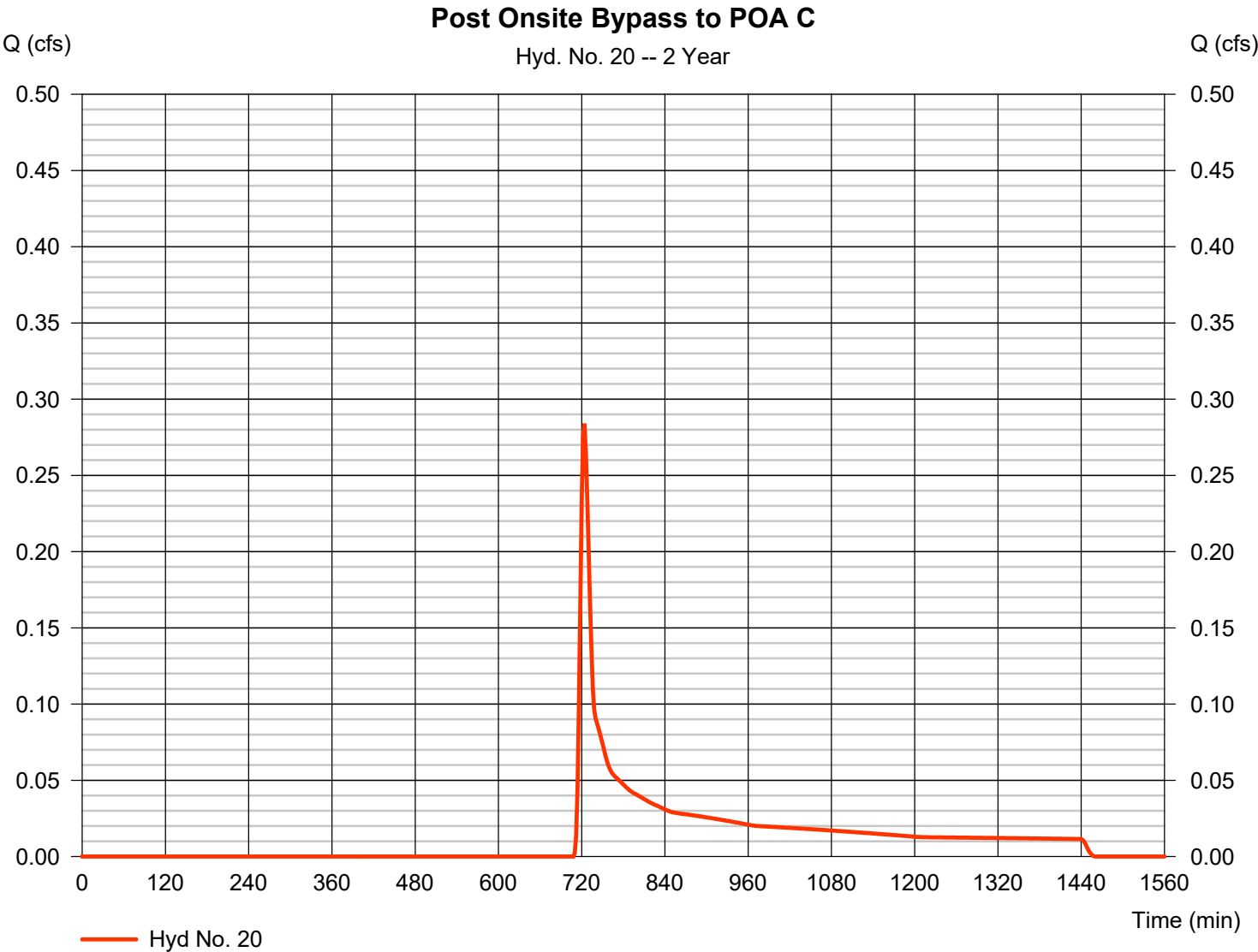
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.284 cfs
Storm frequency	=	2 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	1,183 cuft
Drainage area	=	0.710 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

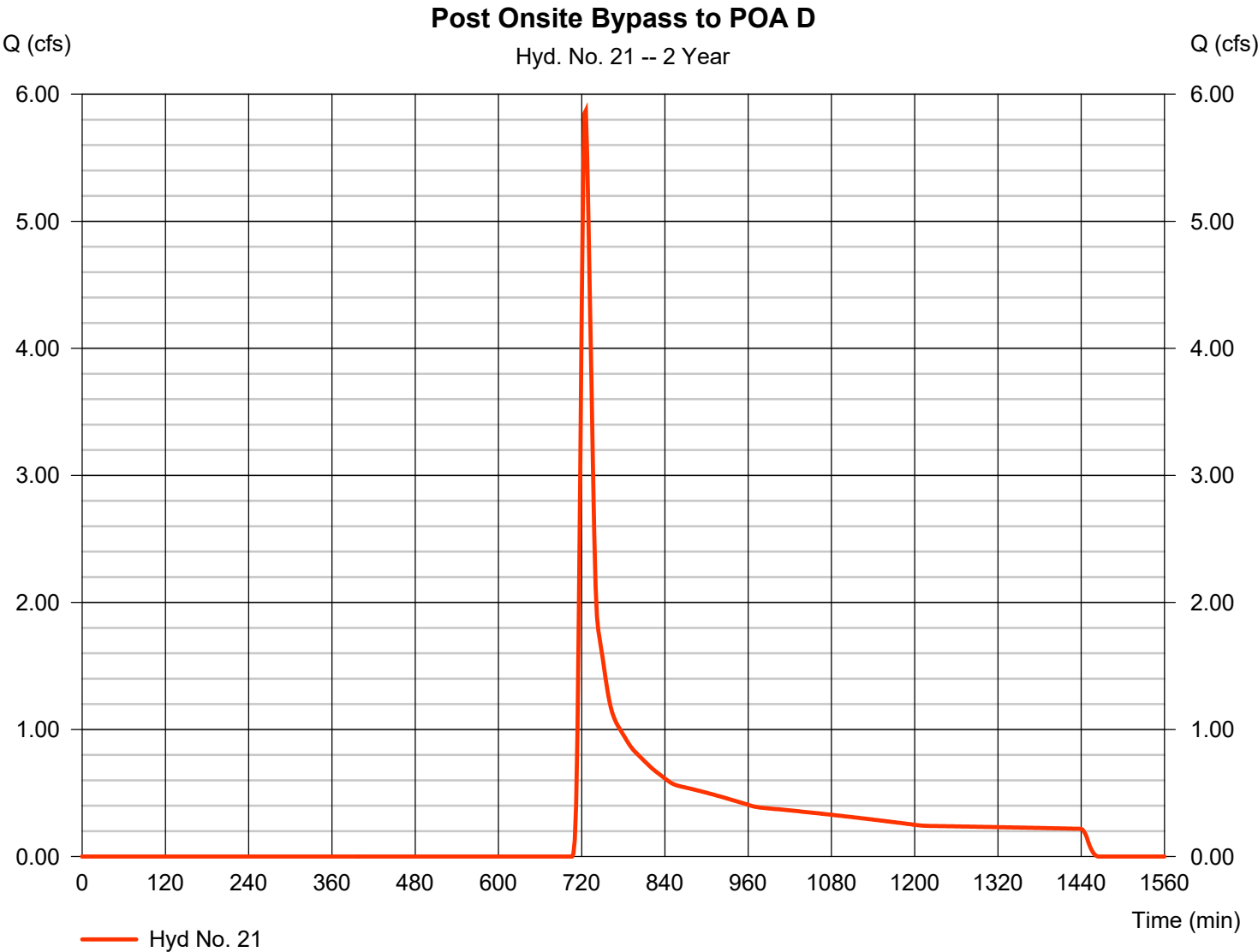
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.871 cfs
Storm frequency	=	2 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	24,088 cuft
Drainage area	=	12.950 ac	Curve number	=	57
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	15.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

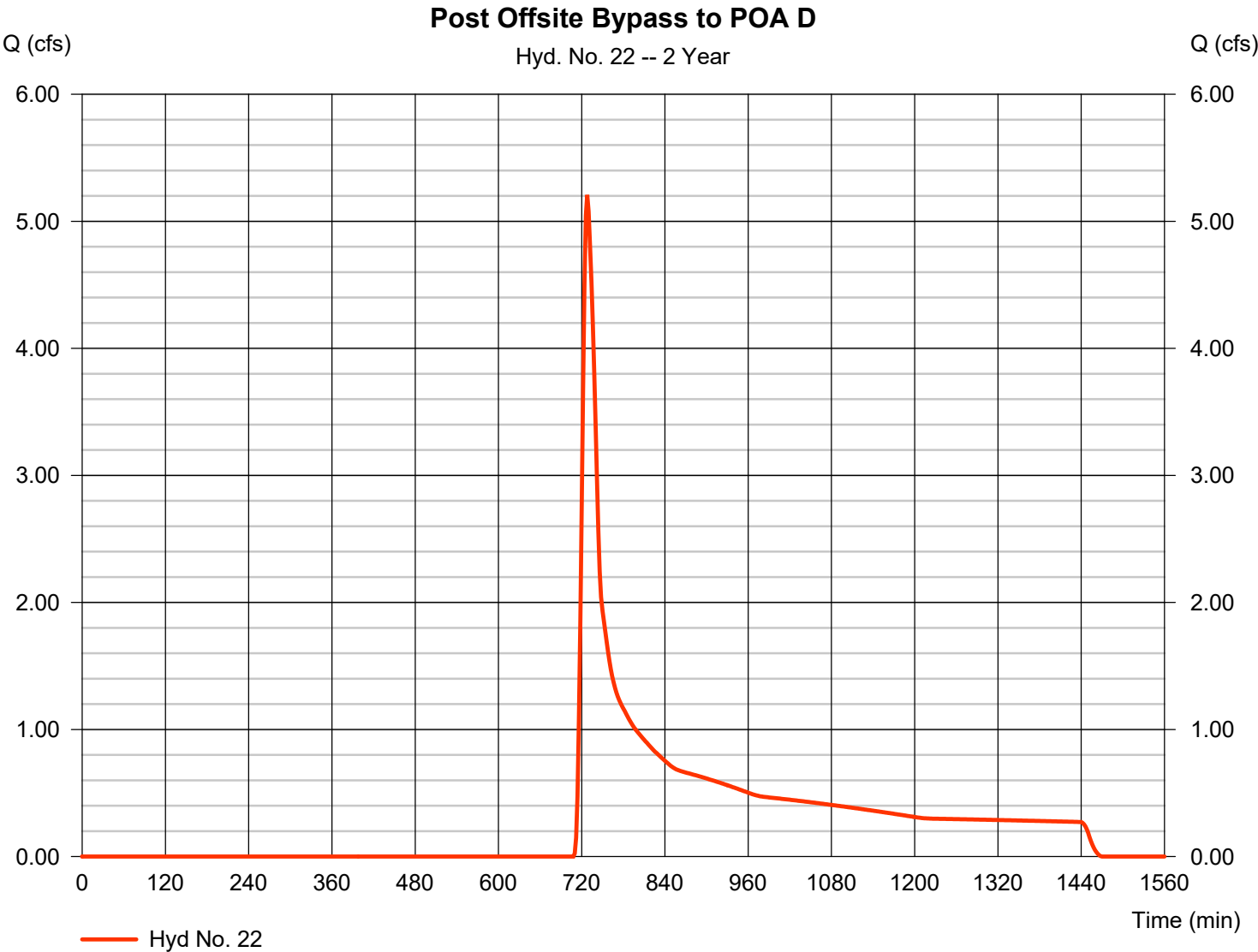


Hydrograph Report

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.208 cfs
Storm frequency	=	2 yrs	Time to peak	=	728 min
Time interval	=	2 min	Hyd. volume	=	27,931 cuft
Drainage area	=	17.290 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	17.00 min
Total precip.	=	3.78 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

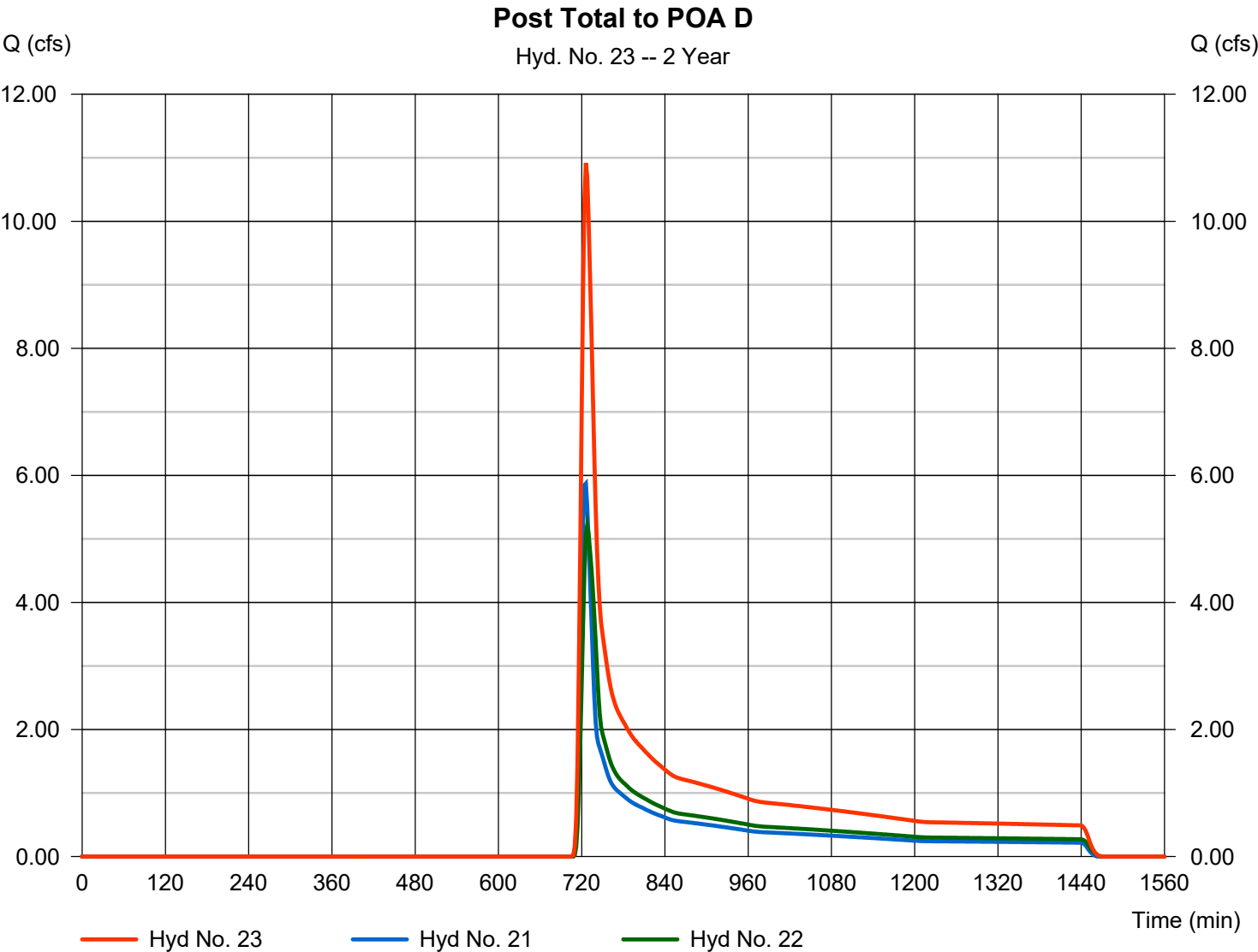


Hydrograph Report

Hyd. No. 23

Post Total to POA D

Hydrograph type	= Combine	Peak discharge	= 10.92 cfs
Storm frequency	= 2 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 52,019 cuft
Inflow hyds.	= 21, 22	Contrib. drain. area	= 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

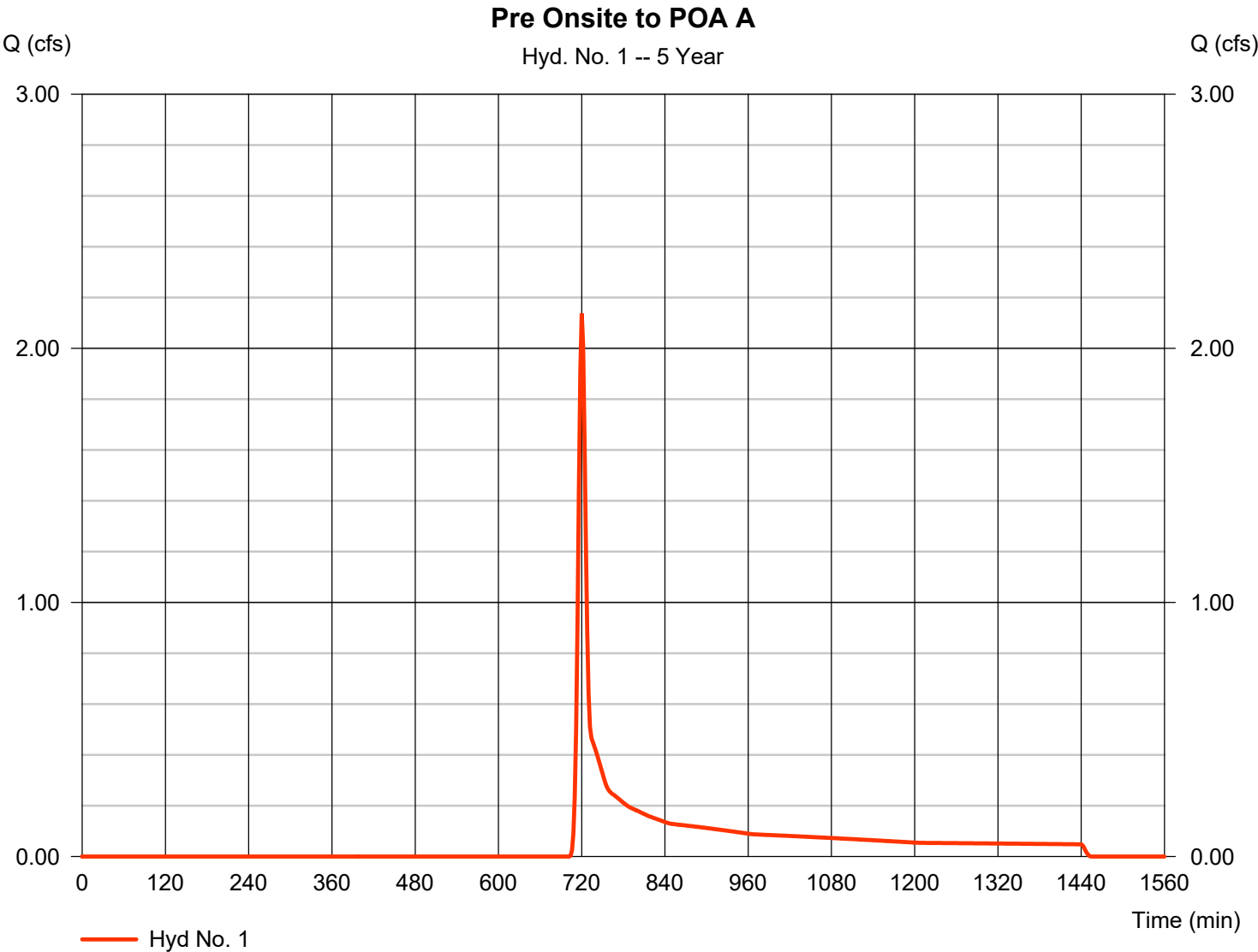
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.138	2	720	5,783	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	2.328	2	722	7,577	-----	-----	-----	Pre Offsite to POA A
3	Combine	4.341	2	722	13,360	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	5.842	2	722	19,015	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	1.204	2	722	3,918	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	9.963	2	724	36,010	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	11.55	2	726	48,304	-----	-----	-----	Pre Offsite to POA D
8	Combine	21.34	2	726	84,314	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	18.27	2	718	36,536	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	0.598	2	874	36,510	10	696.60	20,083	Dry ED Pond 2
12	SCS Runoff	2.875	2	720	6,704	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	1.576	2	722	5,128	-----	-----	-----	Post Offsite to Pond 1
14	Combine	4.622	2	720	48,342	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	0.752	2	962	48,327	14	685.16	12,346	Dry ED Pond 1
16	SCS Runoff	1.464	2	720	3,460	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	0.744	2	722	2,420	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	2.203	2	720	54,207	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	0.971	2	720	2,626	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	0.628	2	722	2,046	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	11.93	2	724	40,271	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	11.55	2	726	48,304	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	23.12	2	726	88,575	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 5 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.138 cfs
Storm frequency	=	5 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	5,783 cuft
Drainage area	=	2.070 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	9.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

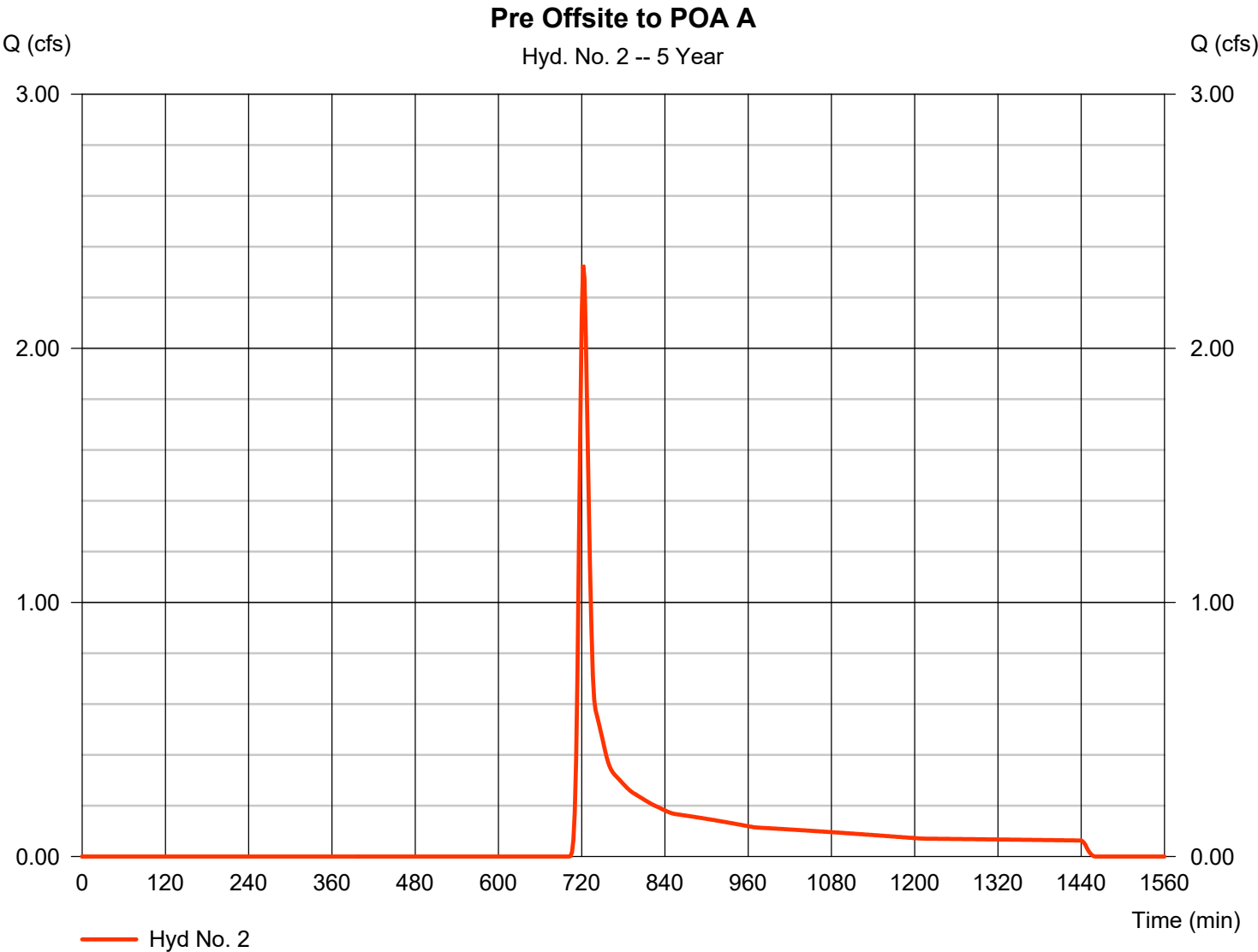


Hydrograph Report

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.328 cfs
Storm frequency	=	5 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	7,577 cuft
Drainage area	=	2.630 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

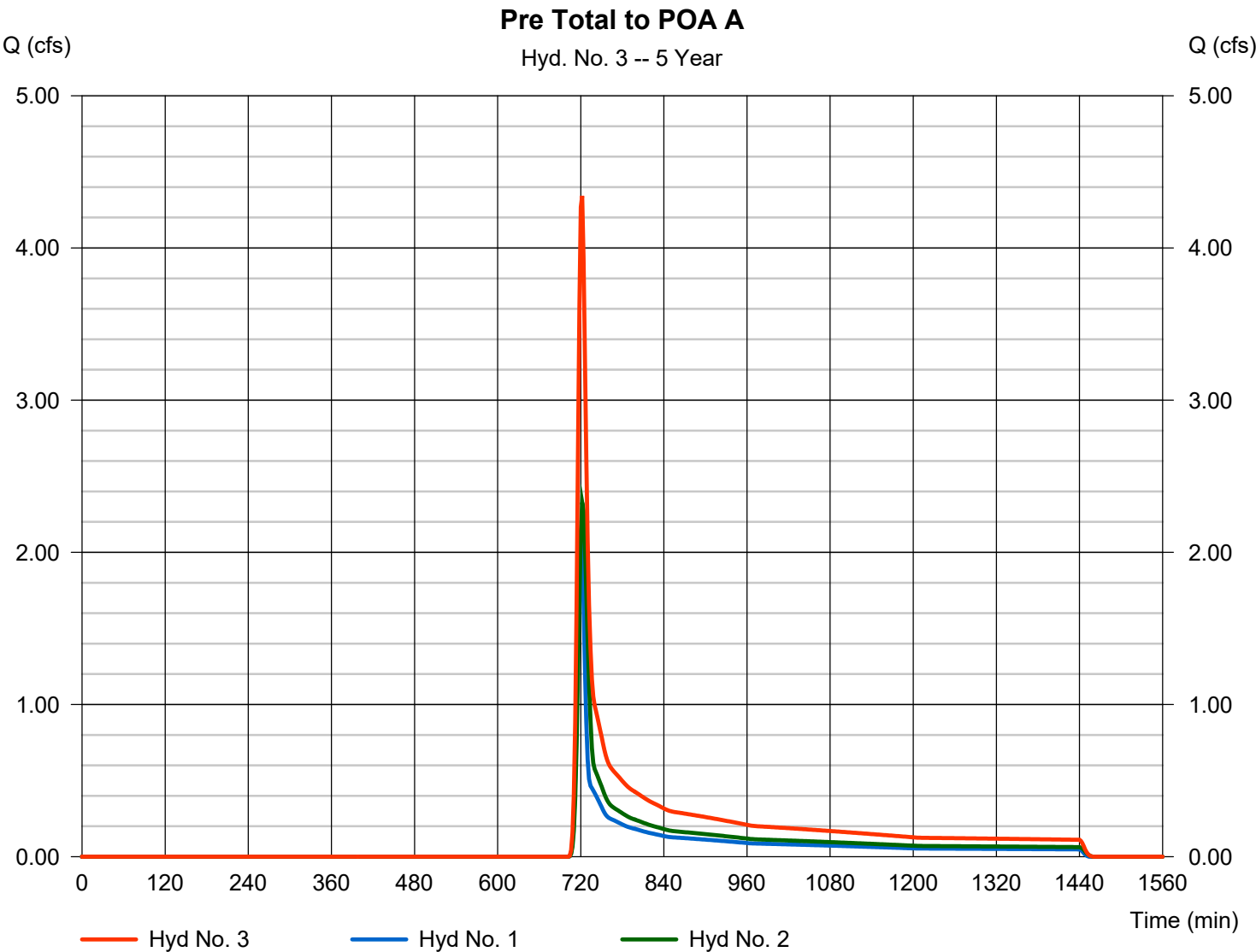
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 3

Pre Total to POA A

Hydrograph type	= Combine	Peak discharge	= 4.341 cfs
Storm frequency	= 5 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 13,360 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 4.700 ac

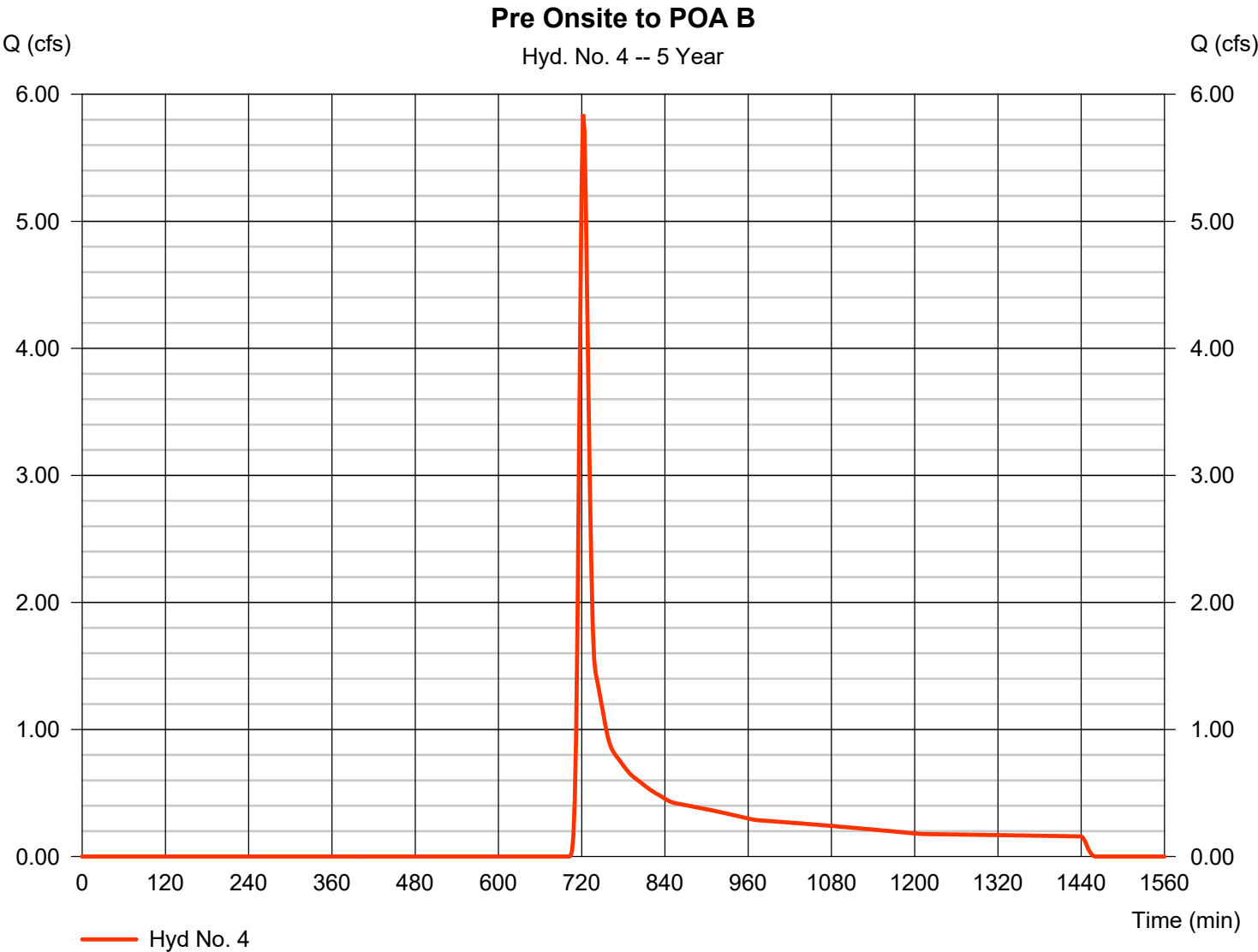


Hydrograph Report

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.842 cfs
Storm frequency	=	5 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	19,015 cuft
Drainage area	=	6.600 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

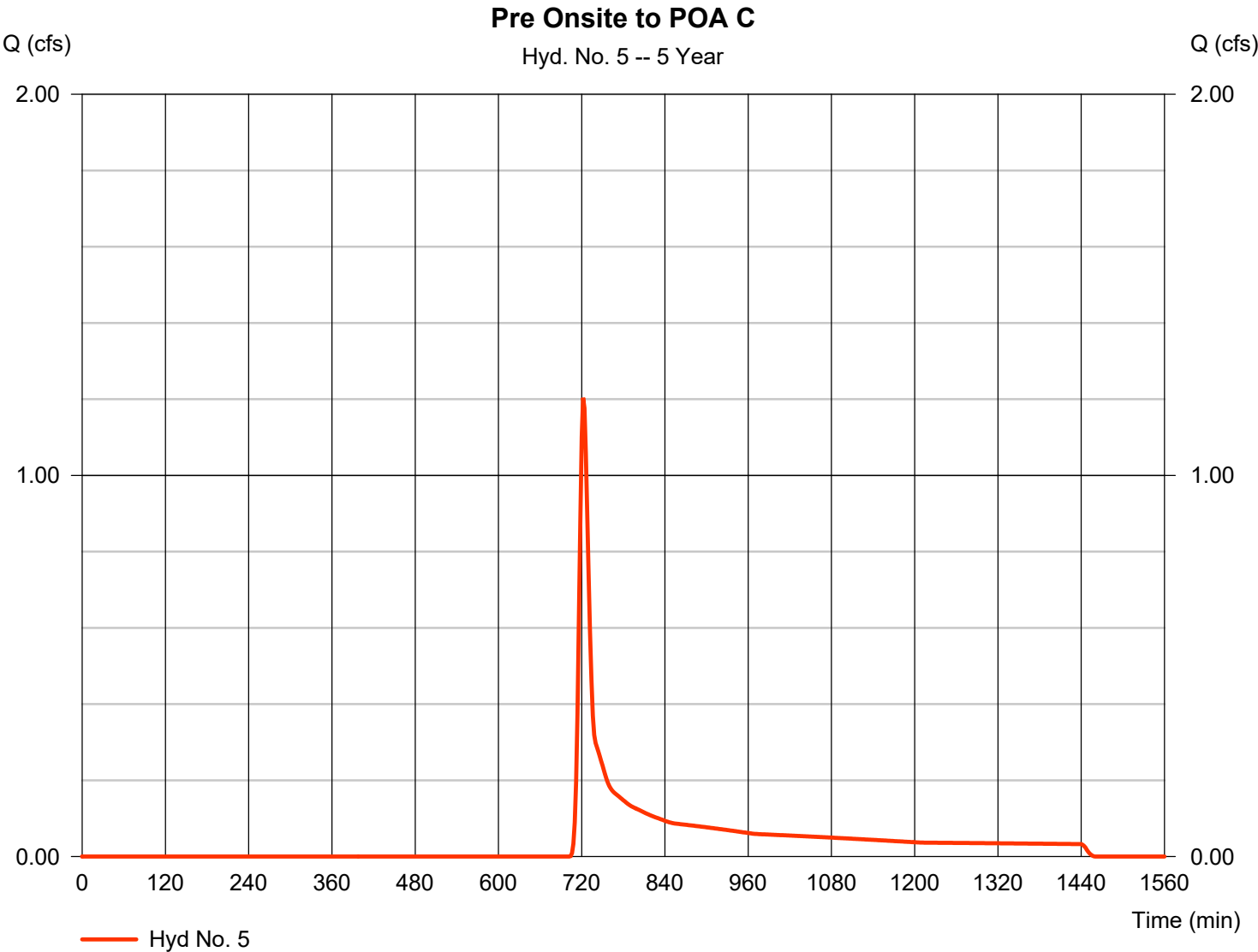
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 1.204 cfs
Storm frequency	= 5 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 3,918 cuft
Drainage area	= 1.360 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

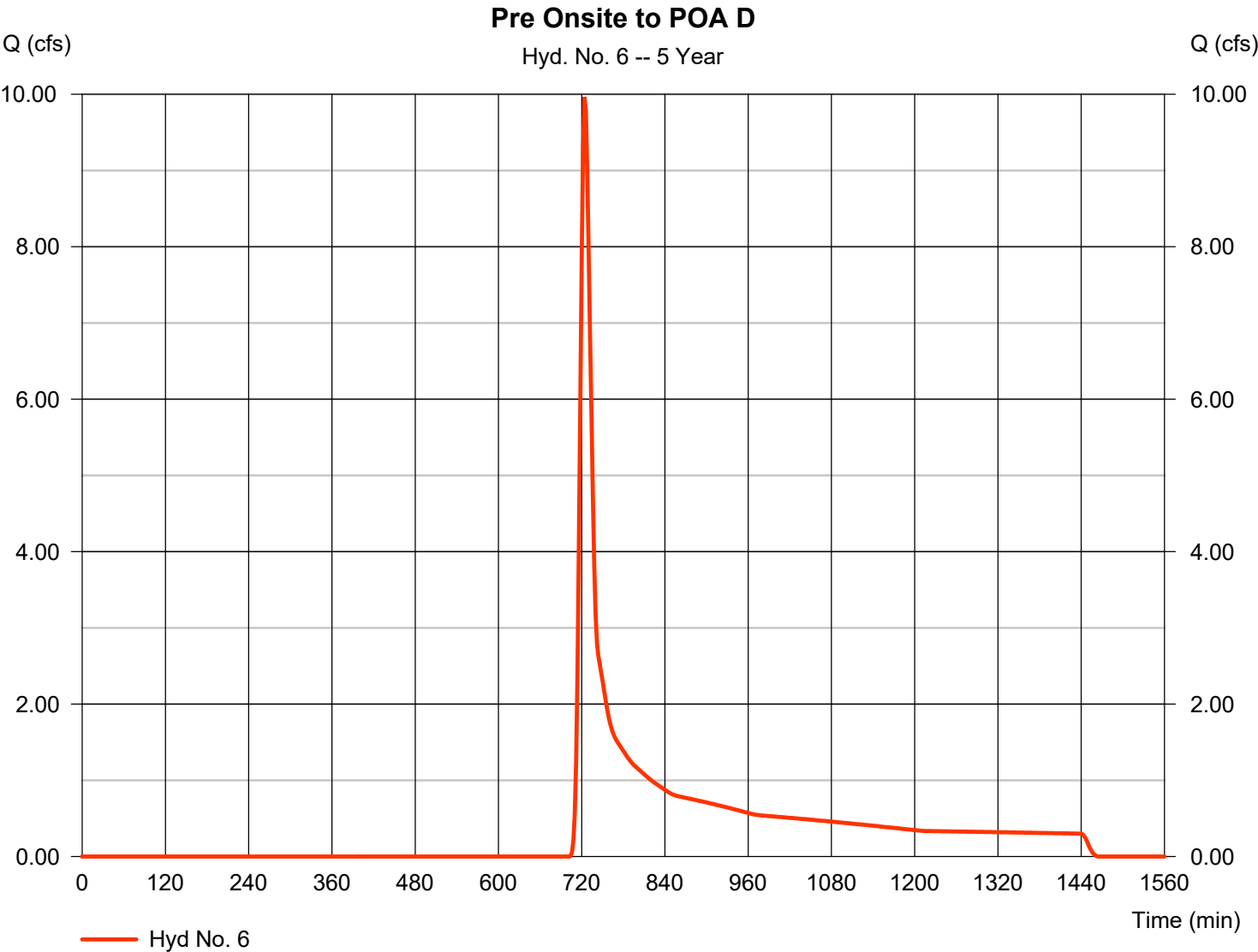


Hydrograph Report

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 9.963 cfs
Storm frequency	= 5 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 36,010 cuft
Drainage area	= 13.220 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 15.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

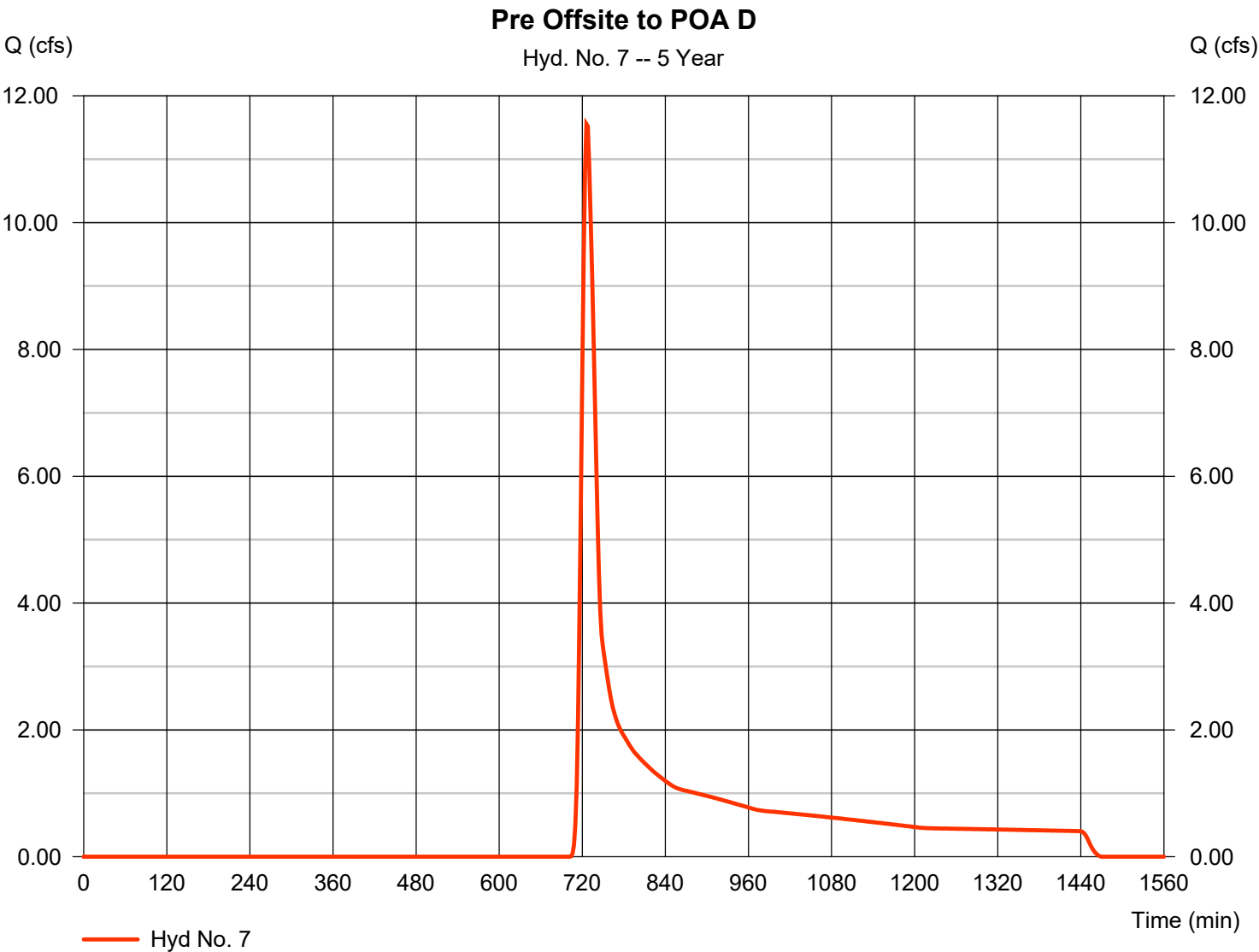
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 11.55 cfs
Storm frequency	= 5 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 48,304 cuft
Drainage area	= 17.290 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 17.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

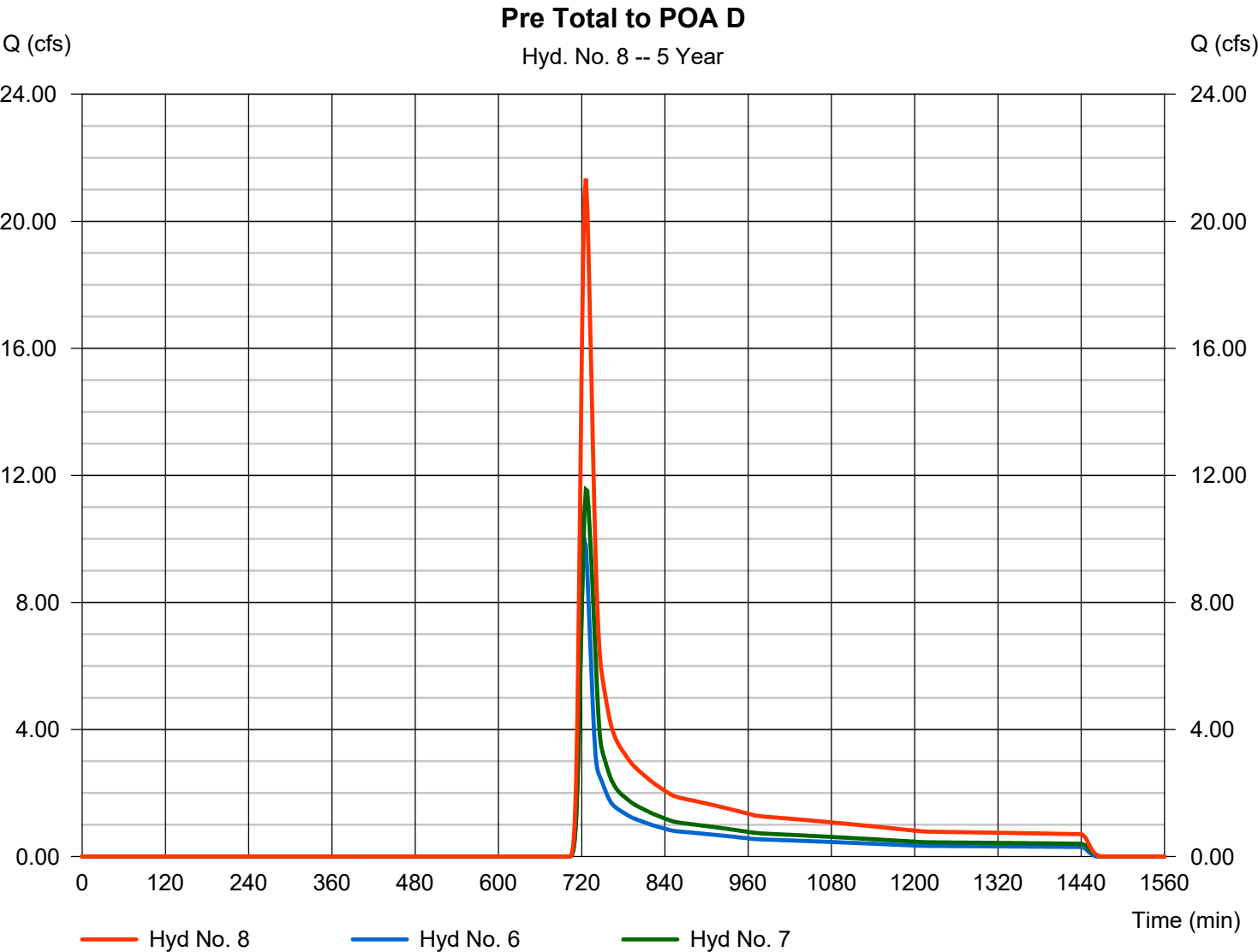


Hydrograph Report

Hyd. No. 8

Pre Total to POA D

Hydrograph type	= Combine	Peak discharge	= 21.34 cfs
Storm frequency	= 5 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 84,314 cuft
Inflow hyds.	= 6, 7	Contrib. drain. area	= 30.510 ac



Hydrograph Report

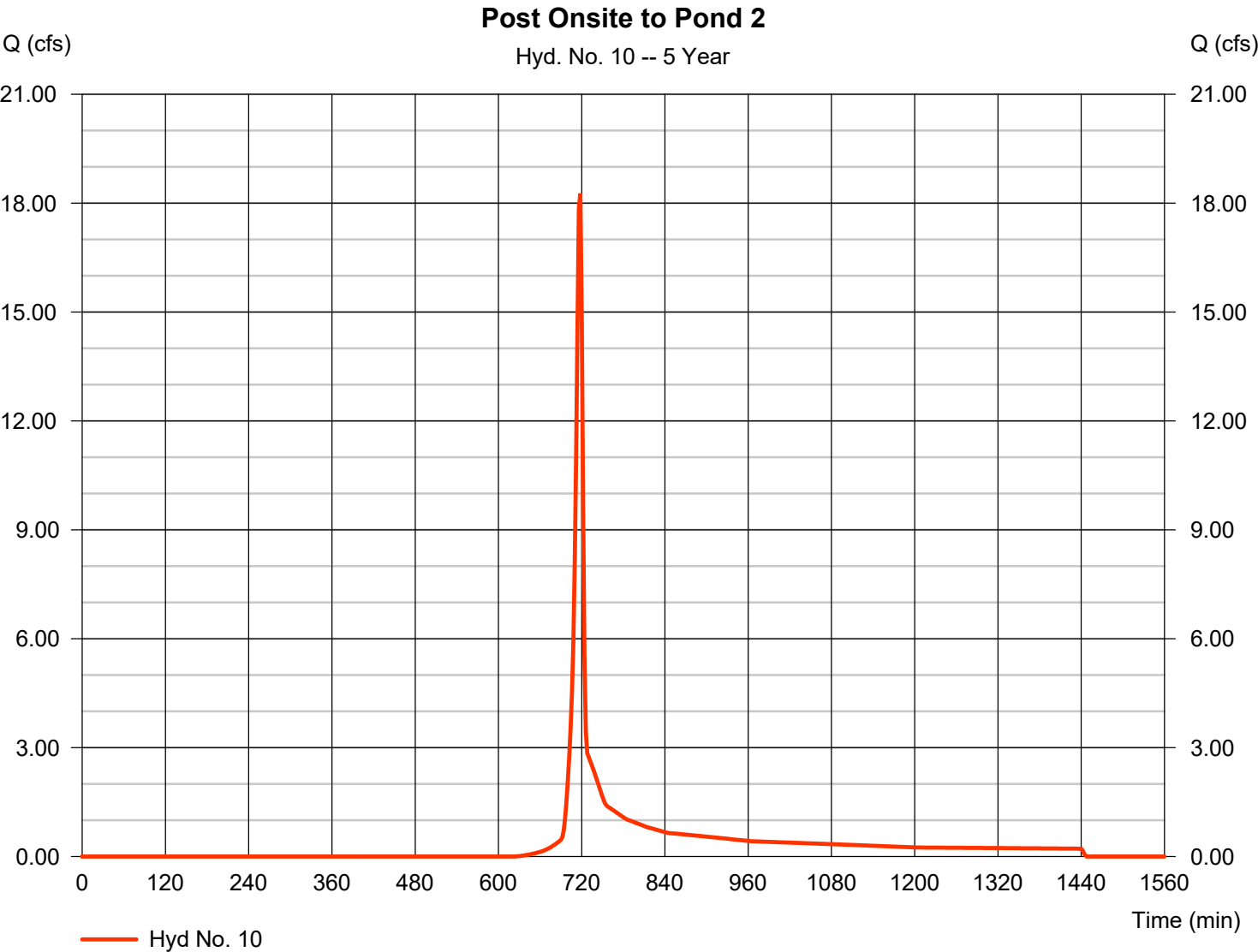
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	=	SCS Runoff	Peak discharge	=	18.27 cfs
Storm frequency	=	5 yrs	Time to peak	=	718 min
Time interval	=	2 min	Hyd. volume	=	36,536 cuft
Drainage area	=	6.530 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	5.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

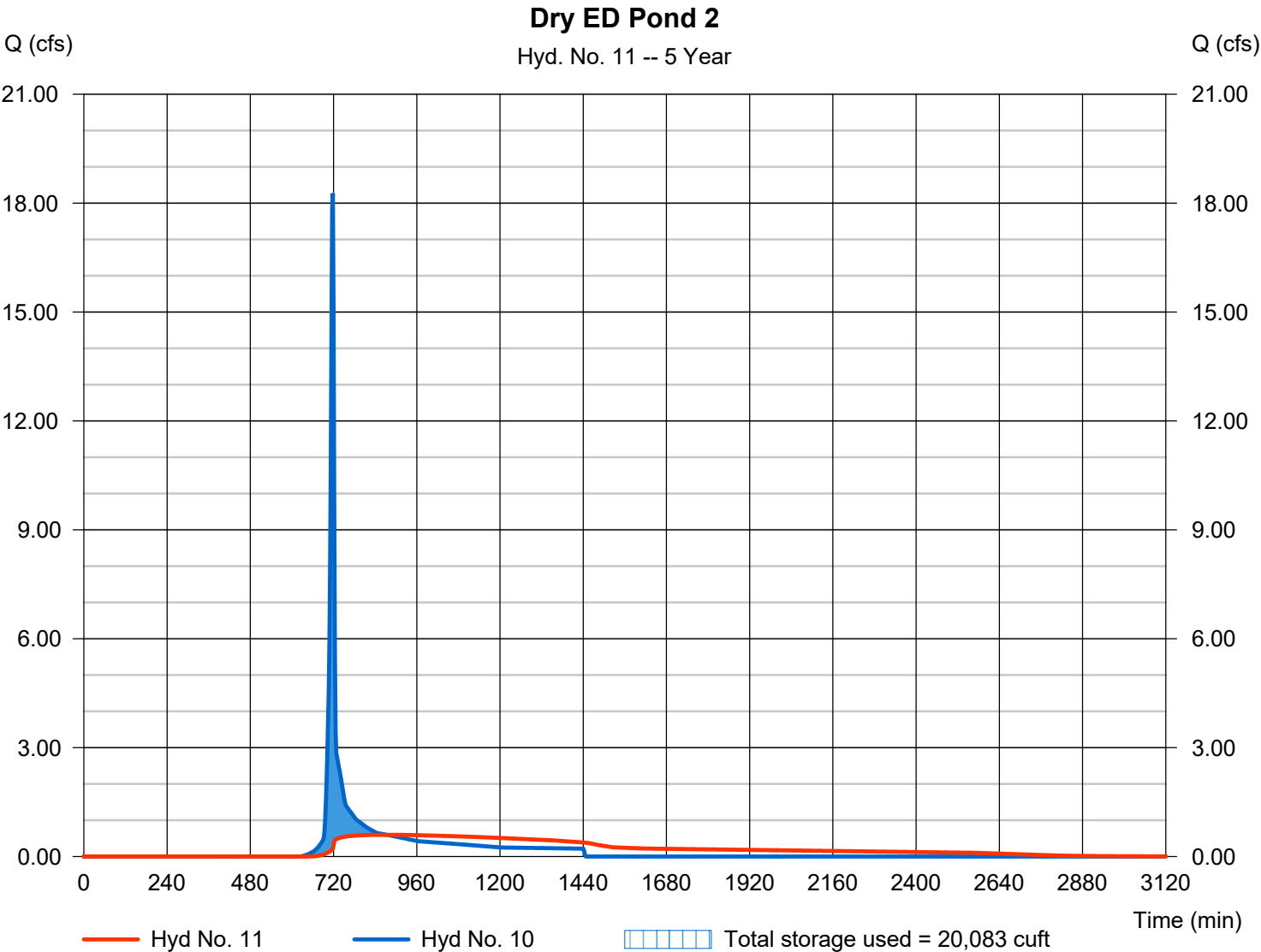
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 0.598 cfs
Storm frequency	= 5 yrs	Time to peak	= 874 min
Time interval	= 2 min	Hyd. volume	= 36,510 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 696.60 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 20,083 cuft

Storage Indication method used.



Hydrograph Report

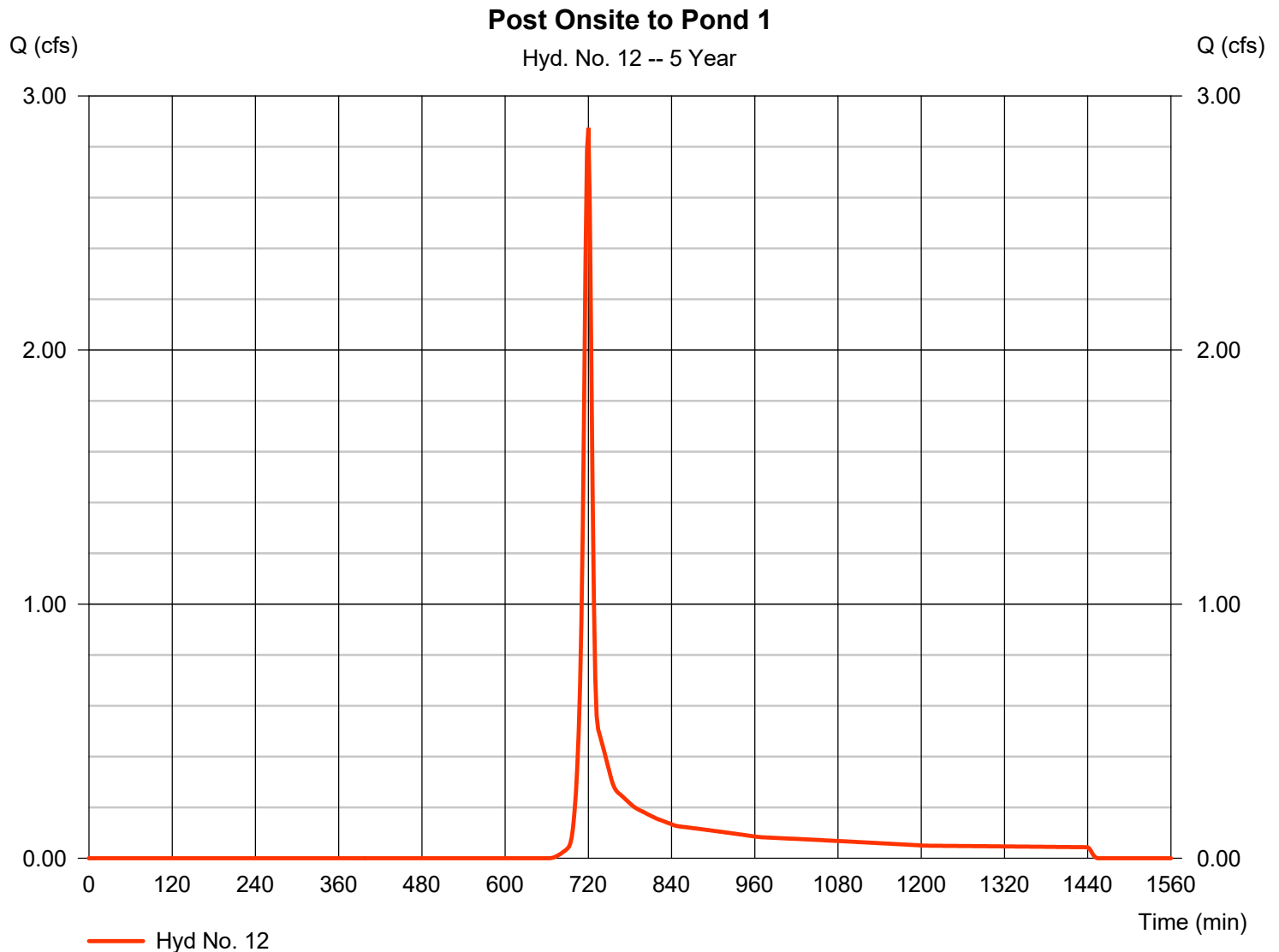
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 2.875 cfs
Storm frequency	= 5 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 6,704 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



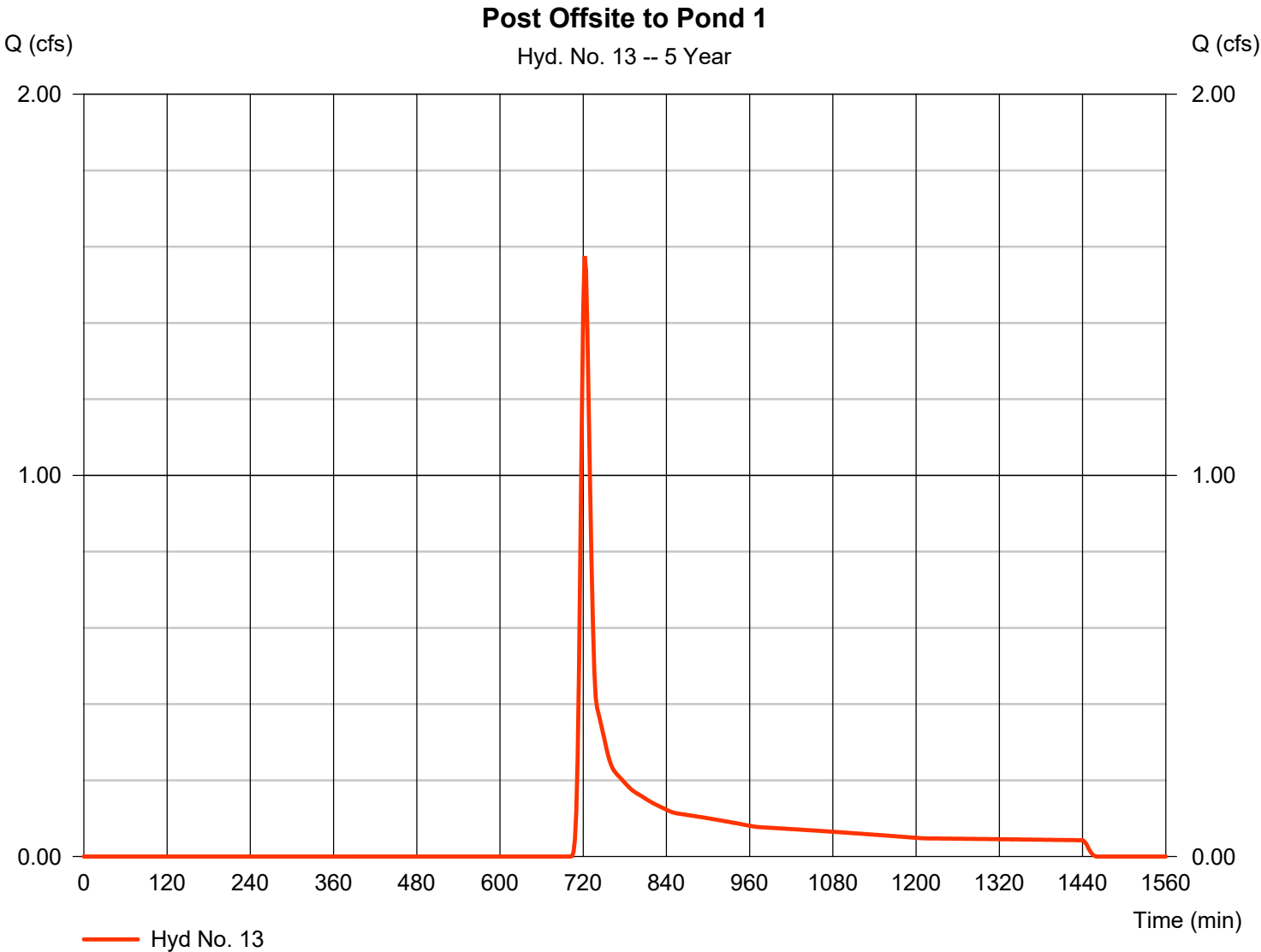
Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022
Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.576 cfs
Storm frequency	=	5 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	5,128 cuft
Drainage area	=	1.780 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

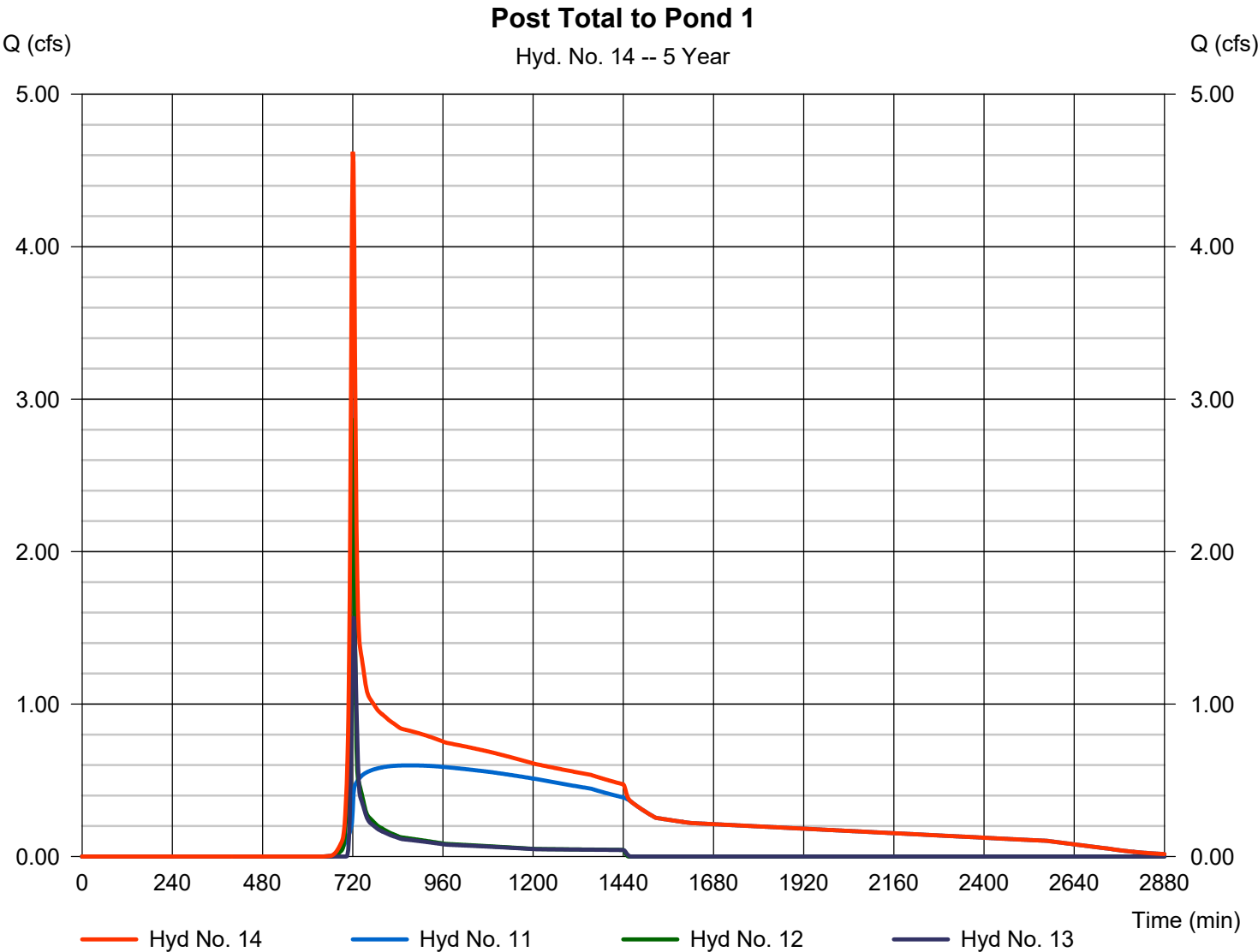


Hydrograph Report

Hyd. No. 14

Post Total to Pond 1

Hydrograph type	= Combine	Peak discharge	= 4.622 cfs
Storm frequency	= 5 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 48,342 cuft
Inflow hyds.	= 11, 12, 13	Contrib. drain. area	= 3.130 ac



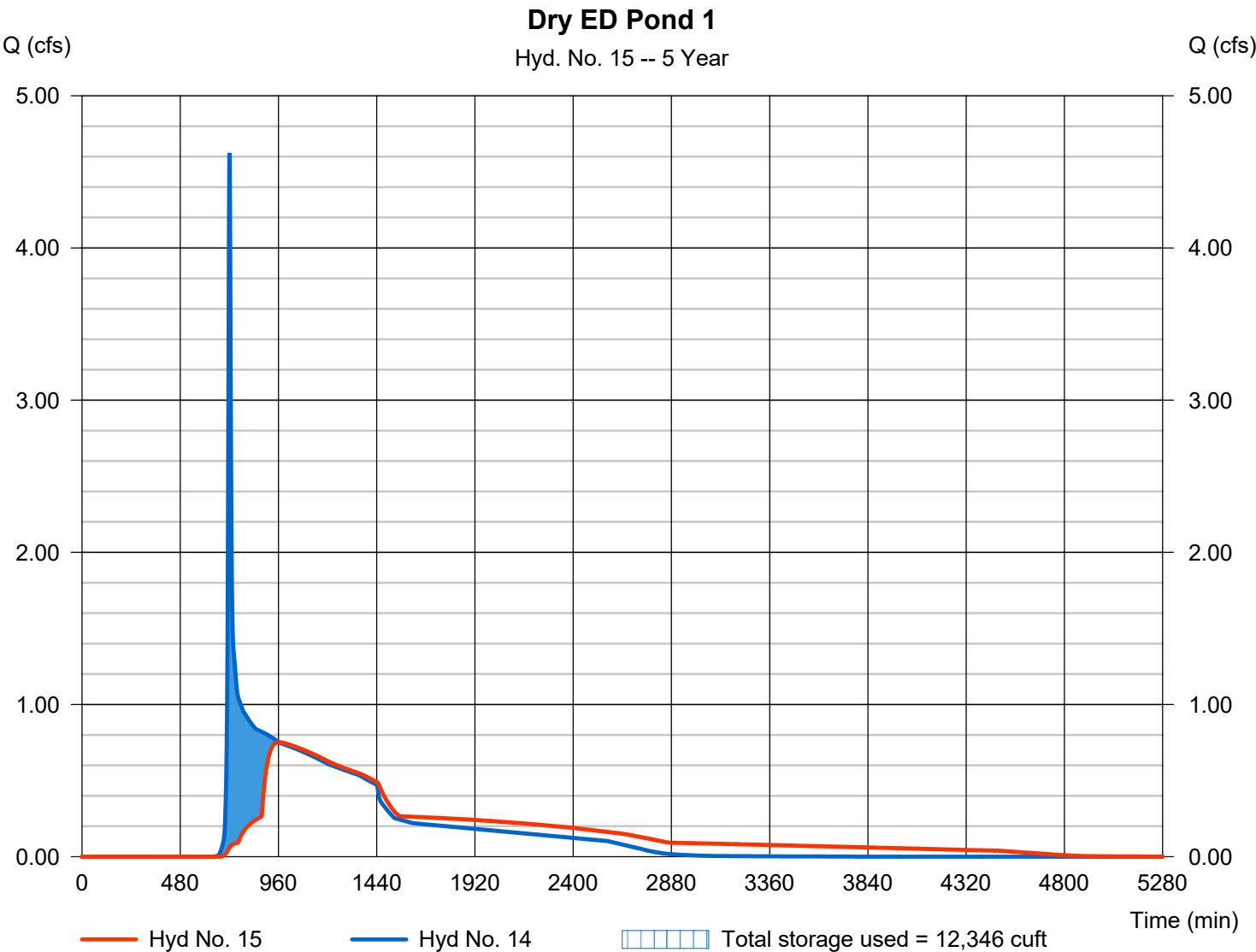
Hydrograph Report

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 0.752 cfs
Storm frequency	= 5 yrs	Time to peak	= 962 min
Time interval	= 2 min	Hyd. volume	= 48,327 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 685.16 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 12,346 cuft

Storage Indication method used.

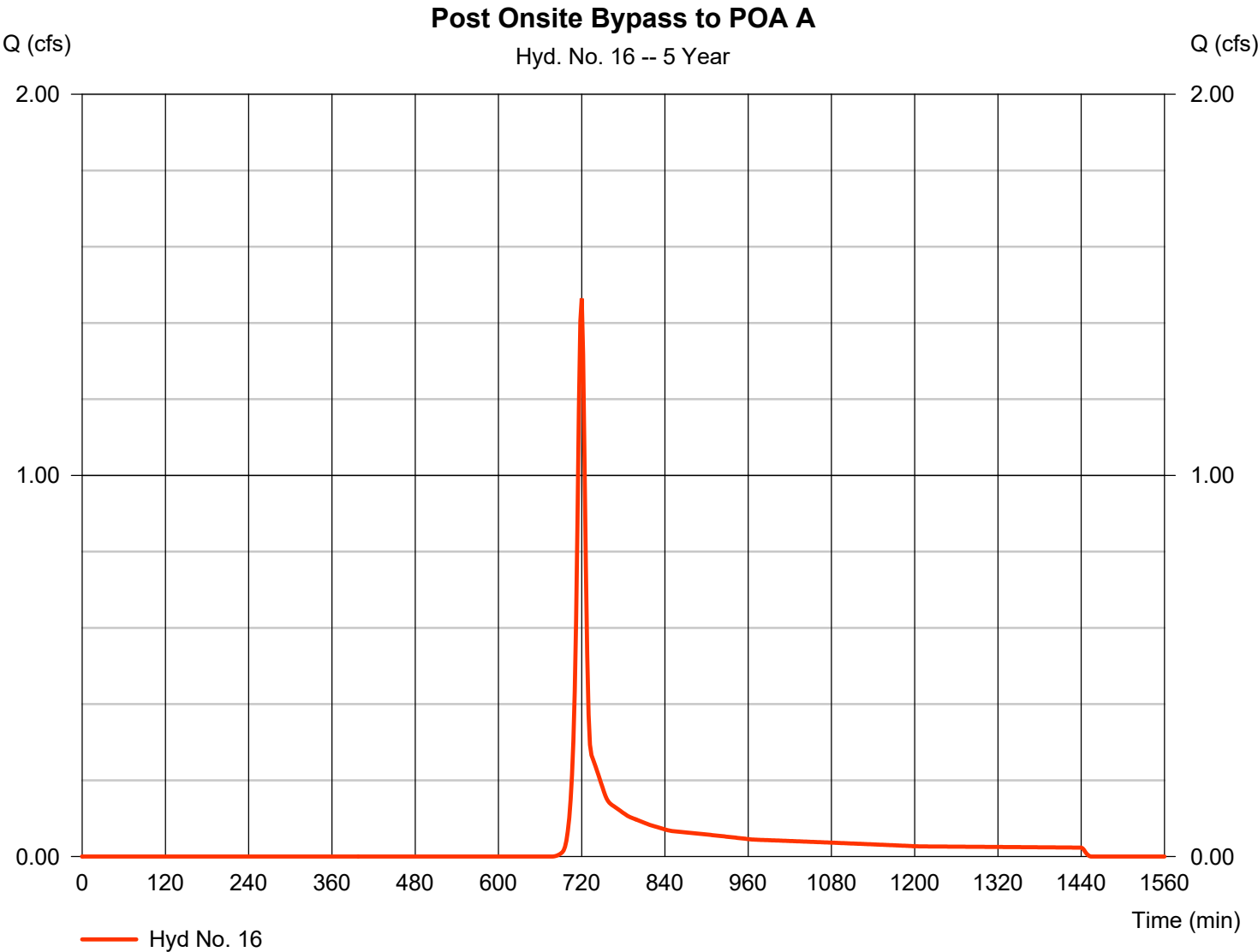


Hydrograph Report

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.464 cfs
Storm frequency	=	5 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	3,460 cuft
Drainage area	=	0.770 ac	Curve number	=	63
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

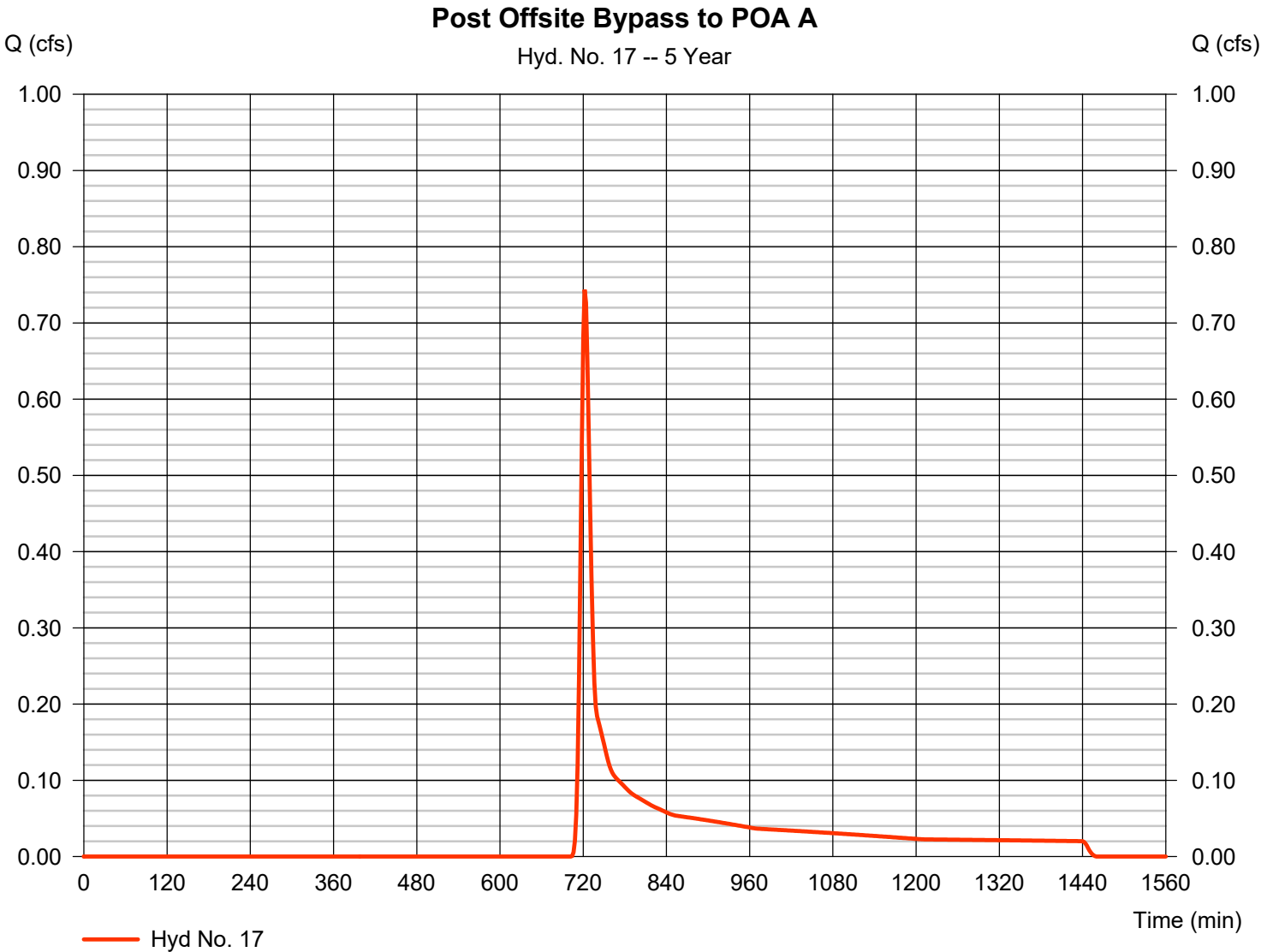
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.744 cfs
Storm frequency	=	5 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	2,420 cuft
Drainage area	=	0.840 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

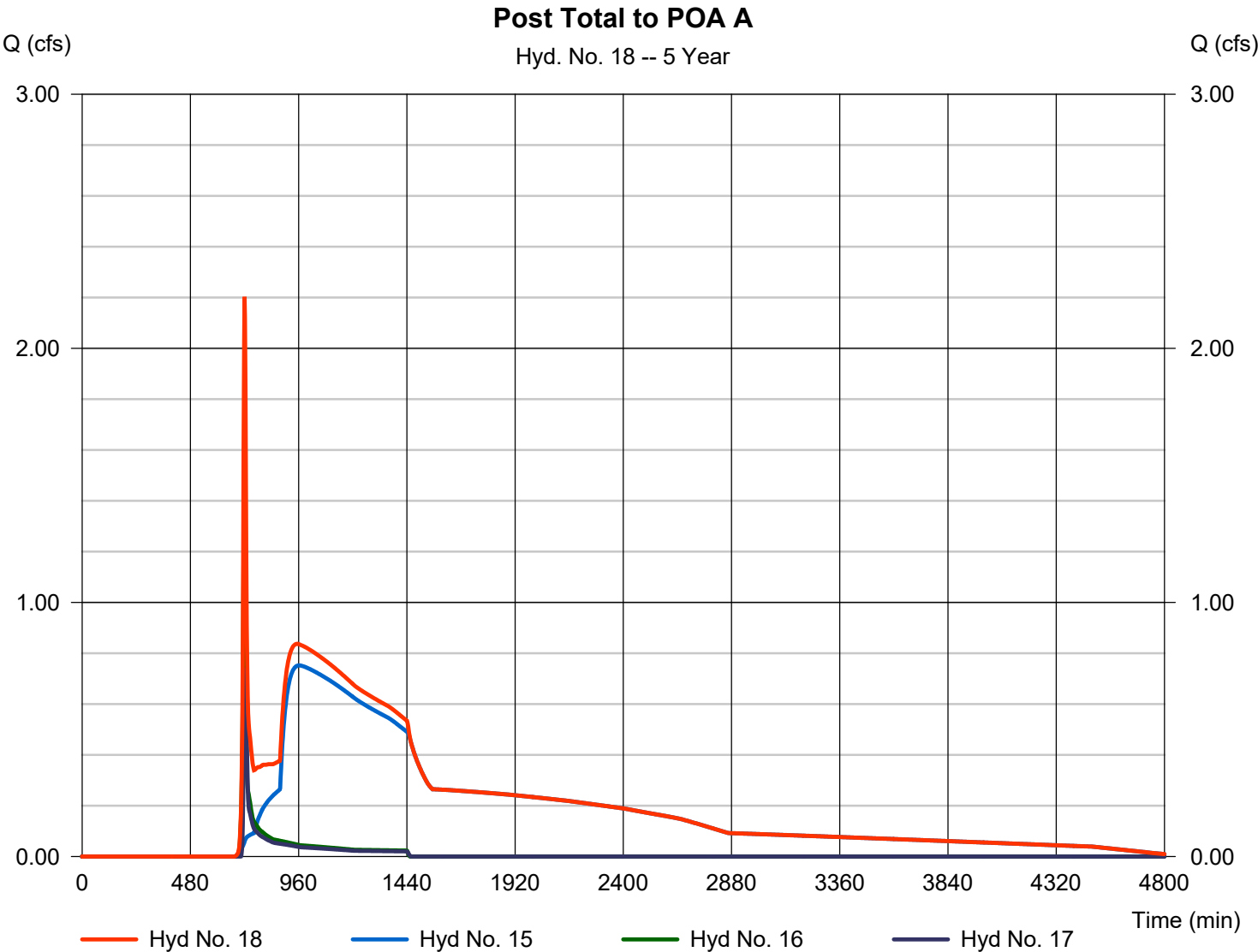
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type	= Combine	Peak discharge	= 2.203 cfs
Storm frequency	= 5 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 54,207 cuft
Inflow hyds.	= 15, 16, 17	Contrib. drain. area	= 1.610 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

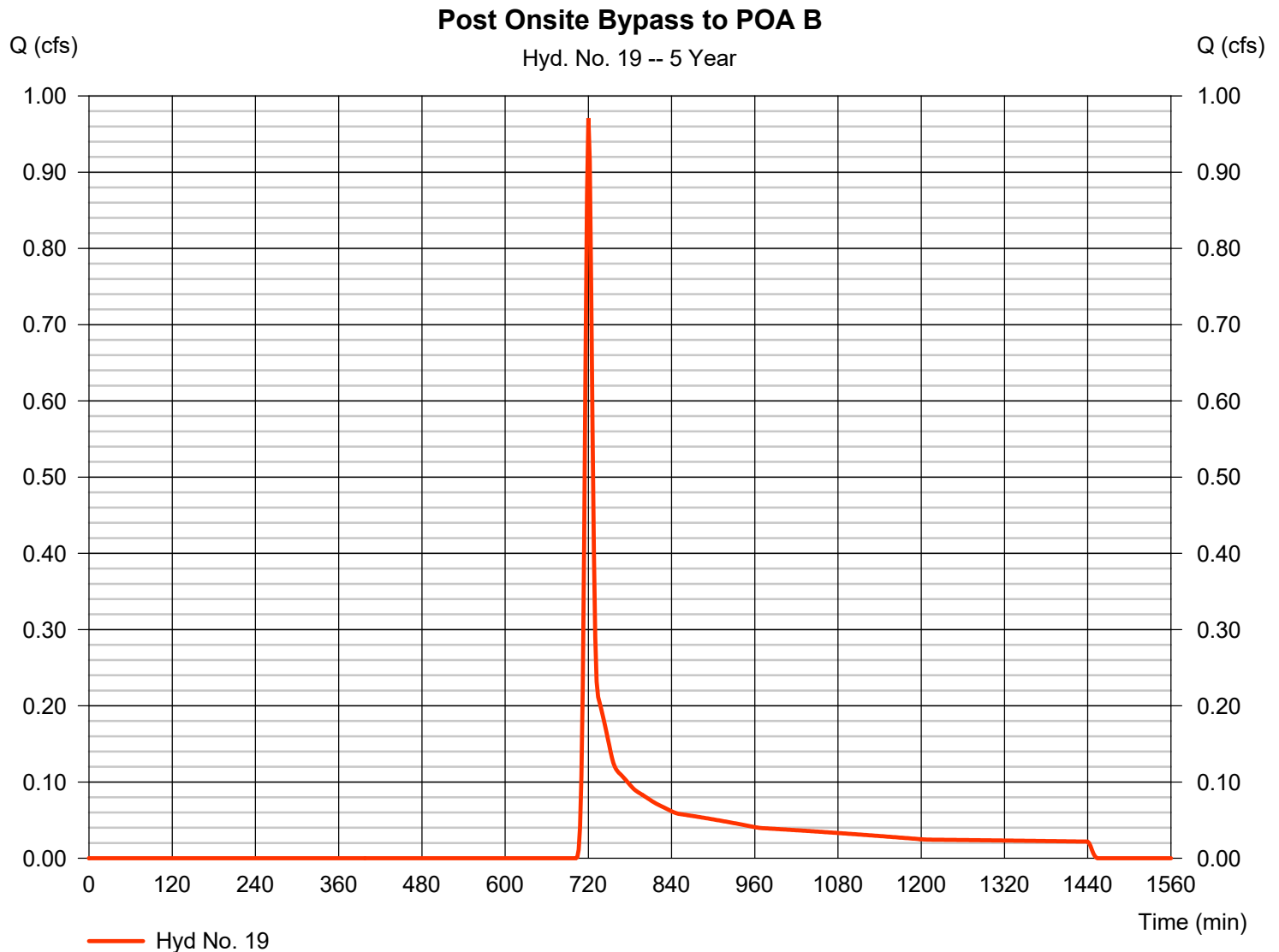
Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type = SCS Runoff
 Storm frequency = 5 yrs
 Time interval = 2 min
 Drainage area = 0.940 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 4.56 in
 Storm duration = 24 hrs

Peak discharge = 0.971 cfs
 Time to peak = 720 min
 Hyd. volume = 2,626 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 8.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

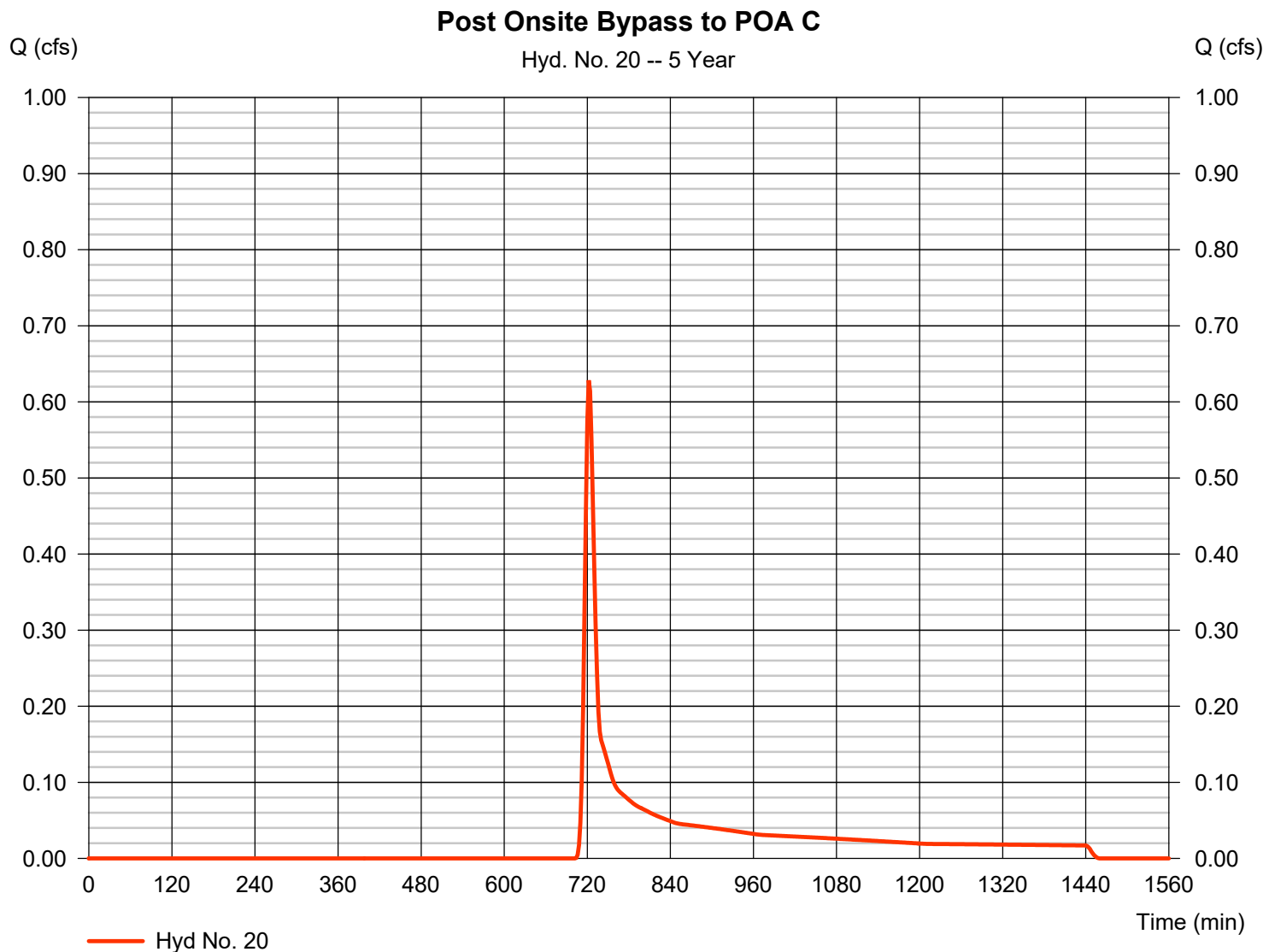
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 0.628 cfs
Storm frequency	= 5 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 2,046 cuft
Drainage area	= 0.710 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

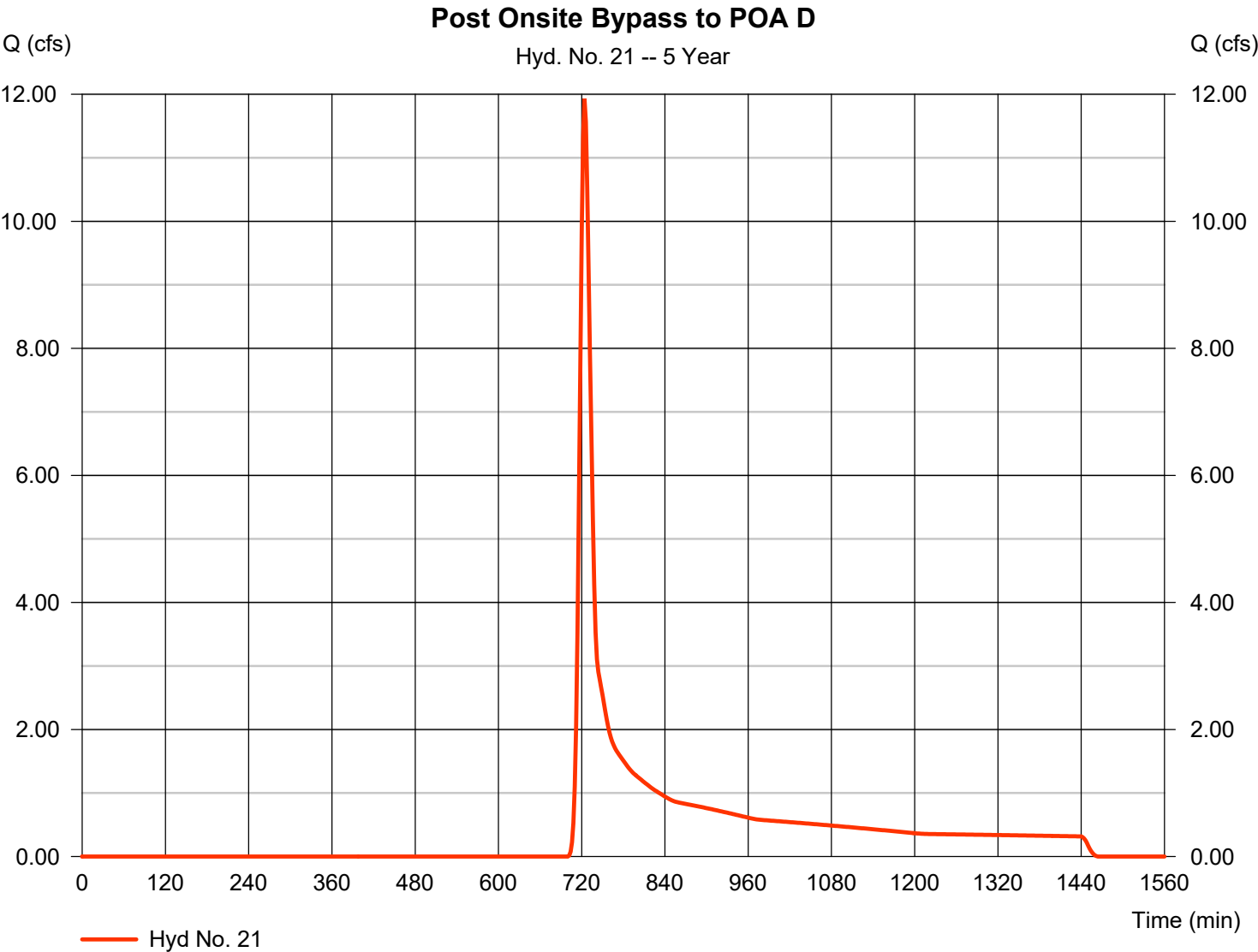
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	11.93 cfs
Storm frequency	=	5 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	40,271 cuft
Drainage area	=	12.950 ac	Curve number	=	57
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	15.00 min
Total precip.	=	4.56 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

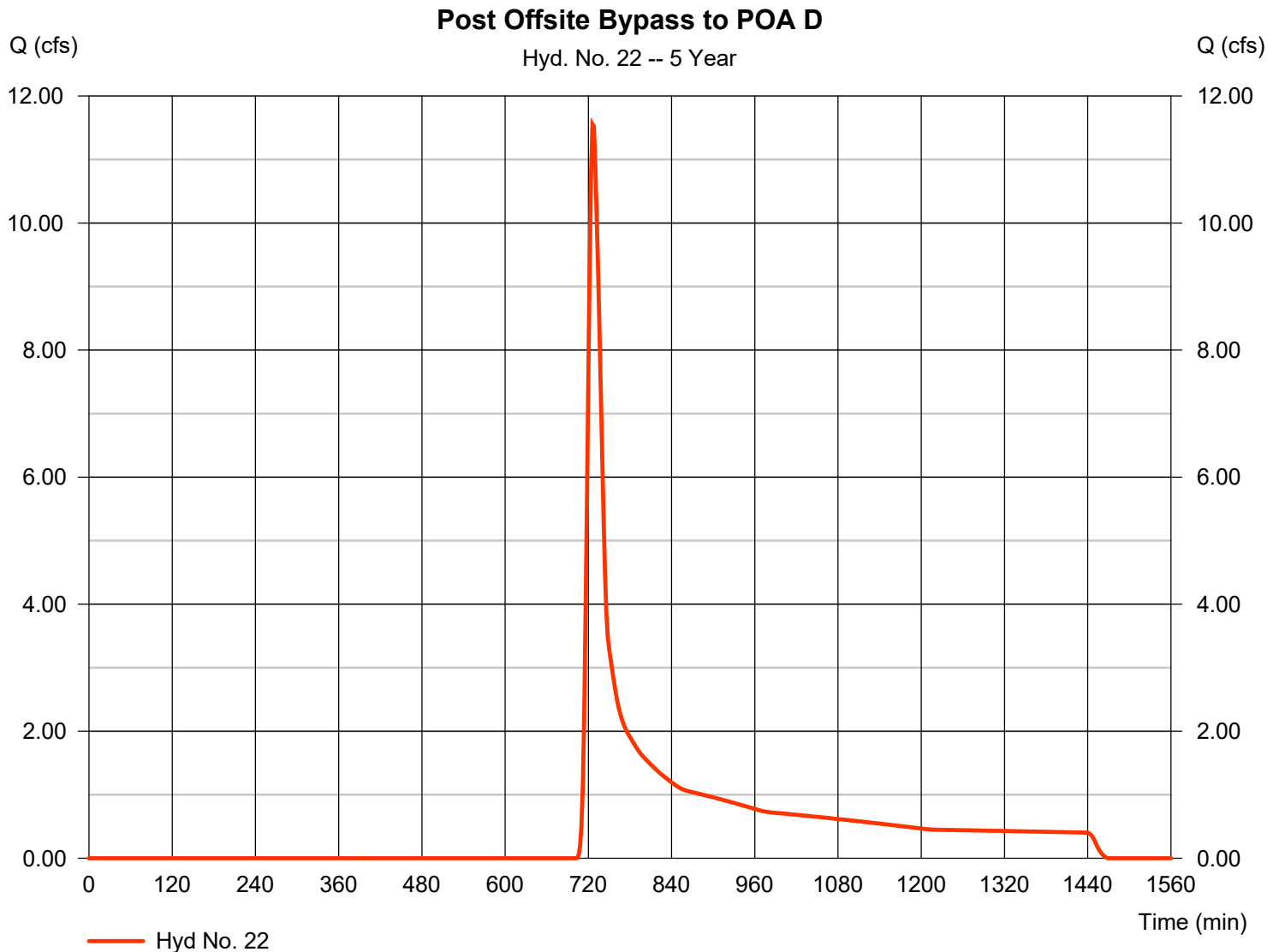
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 11.55 cfs
Storm frequency	= 5 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 48,304 cuft
Drainage area	= 17.290 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 17.00 min
Total precip.	= 4.56 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

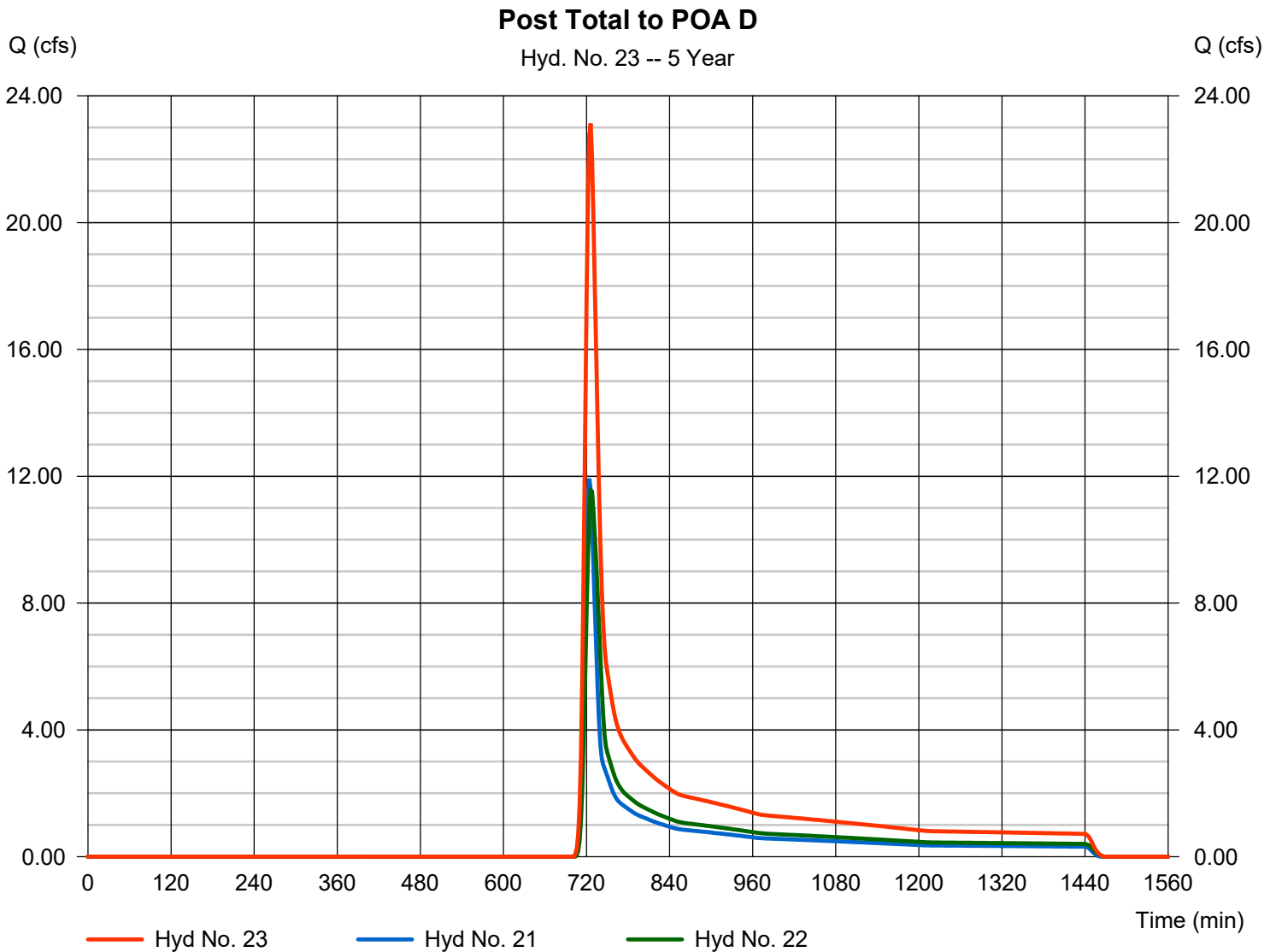
Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type = Combine
Storm frequency = 5 yrs
Time interval = 2 min
Inflow hyds. = 21, 22

Peak discharge = 23.12 cfs
Time to peak = 726 min
Hyd. volume = 88,575 cuft
Contrib. drain. area = 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.317	2	720	8,279	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	3.679	2	722	10,847	-----	-----	-----	Pre Offsite to POA A
3	Combine	6.795	2	720	19,126	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	9.233	2	722	27,221	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	1.902	2	722	5,609	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	15.84	2	724	51,551	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	18.55	2	726	69,151	-----	-----	-----	Pre Offsite to POA D
8	Combine	33.79	2	726	120,702	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	23.67	2	718	47,412	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	1.142	2	800	47,387	10	697.15	25,317	Dry ED Pond 2
12	SCS Runoff	3.862	2	720	8,896	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	2.490	2	722	7,342	-----	-----	-----	Post Offsite to Pond 1
14	Combine	6.743	2	720	63,624	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	1.416	2	834	63,608	14	685.30	13,016	Dry ED Pond 1
16	SCS Runoff	2.005	2	720	4,648	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	1.175	2	722	3,465	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	3.186	2	720	71,720	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	1.506	2	720	3,760	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	0.993	2	722	2,928	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	18.06	2	724	56,593	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	18.55	2	726	69,151	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	35.86	2	724	125,744	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 10 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

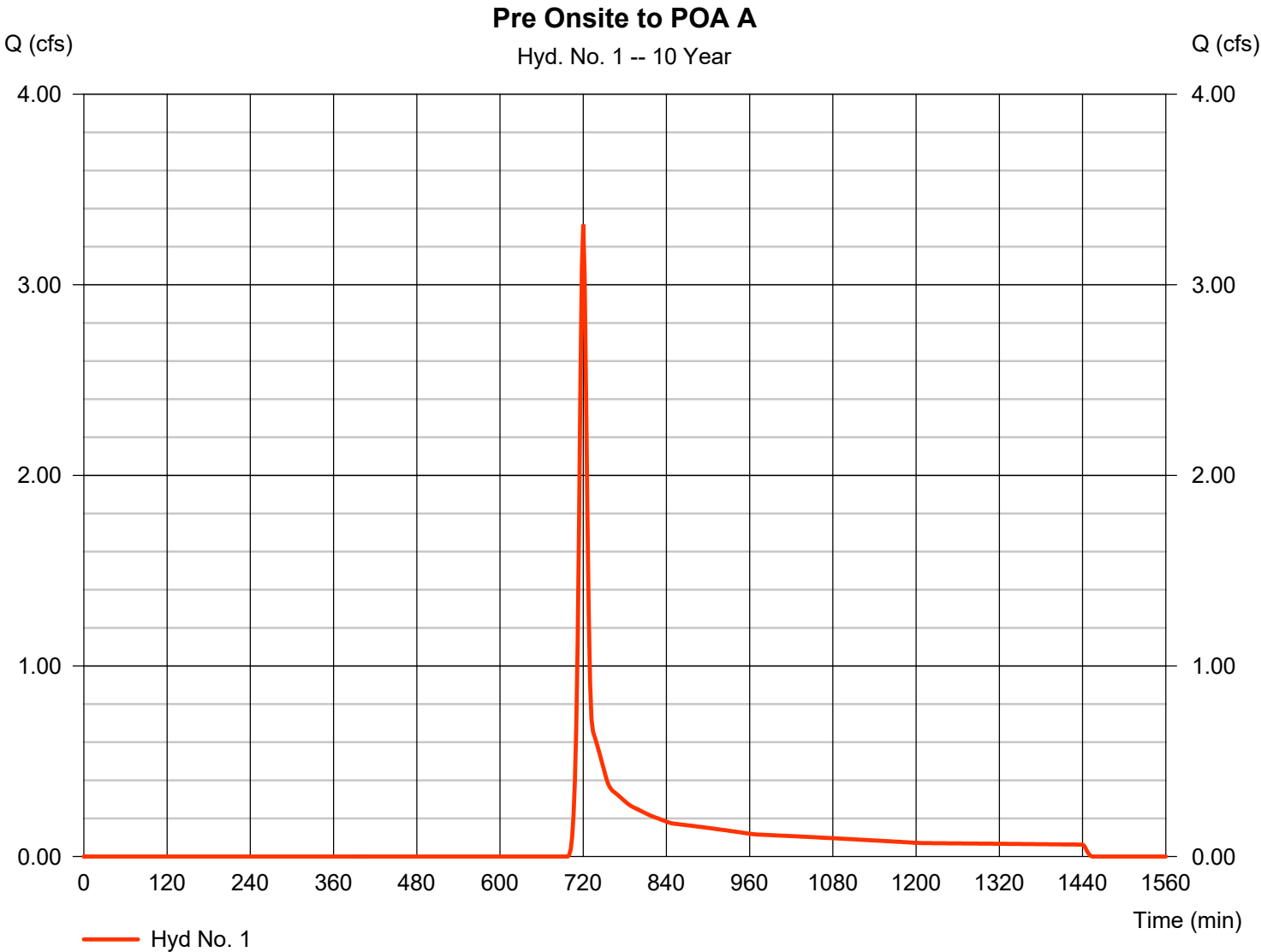
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 3.317 cfs
Storm frequency	= 10 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 8,279 cuft
Drainage area	= 2.070 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 9.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

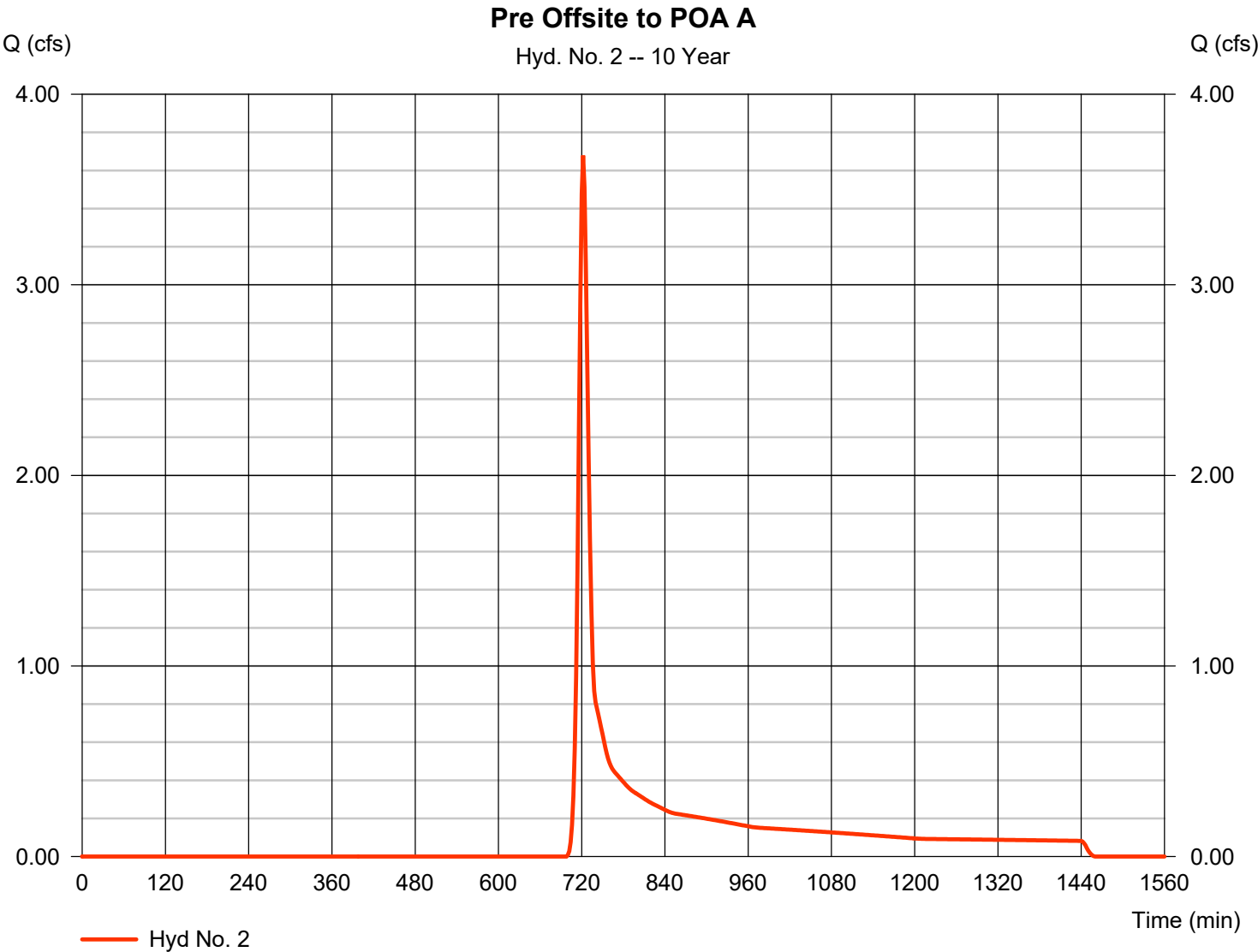


Hydrograph Report

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	3.679 cfs
Storm frequency	=	10 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	10,847 cuft
Drainage area	=	2.630 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

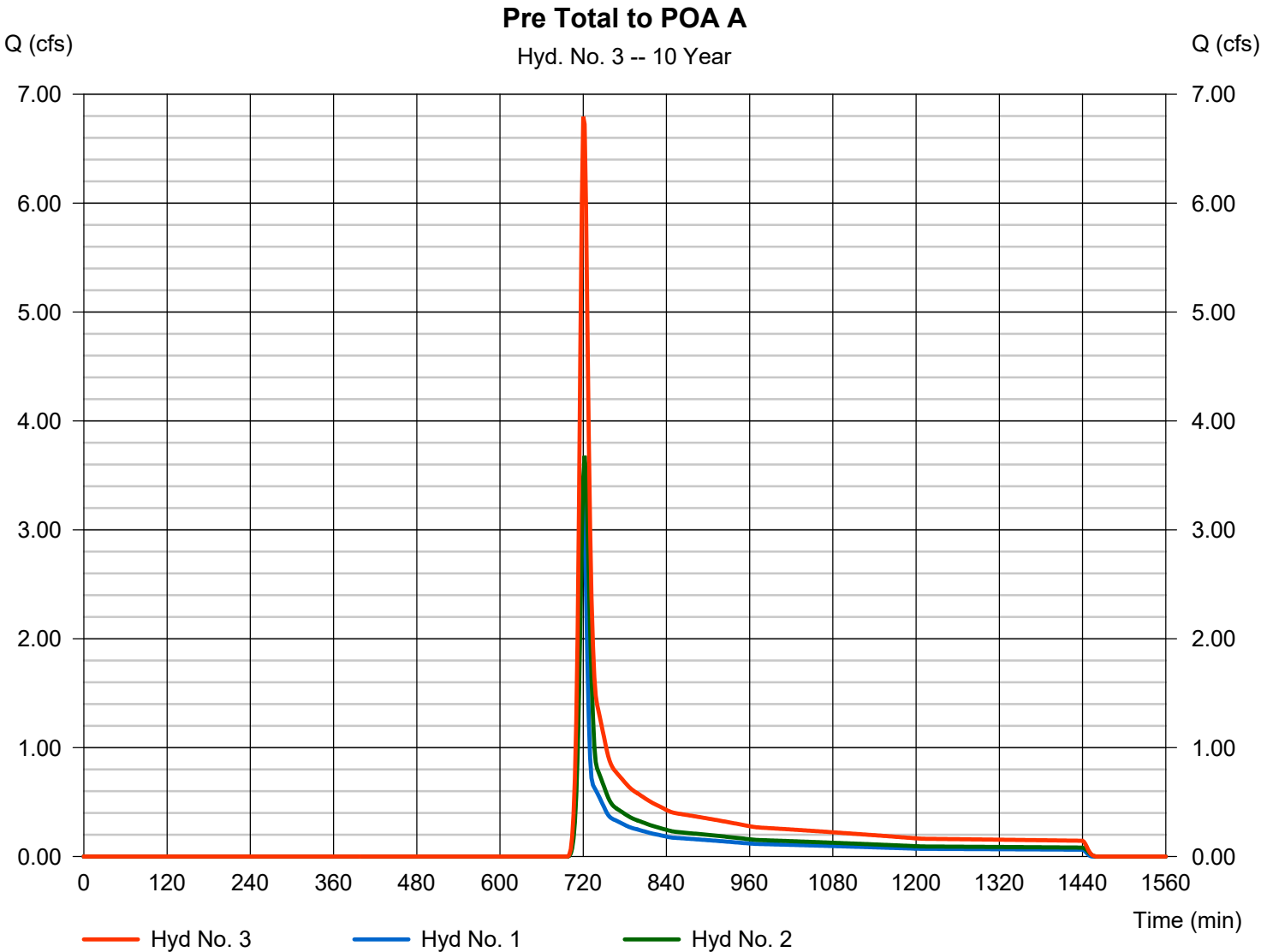
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 3

Pre Total to POA A

Hydrograph type	= Combine	Peak discharge	= 6.795 cfs
Storm frequency	= 10 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 19,126 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 4.700 ac



Hydrograph Report

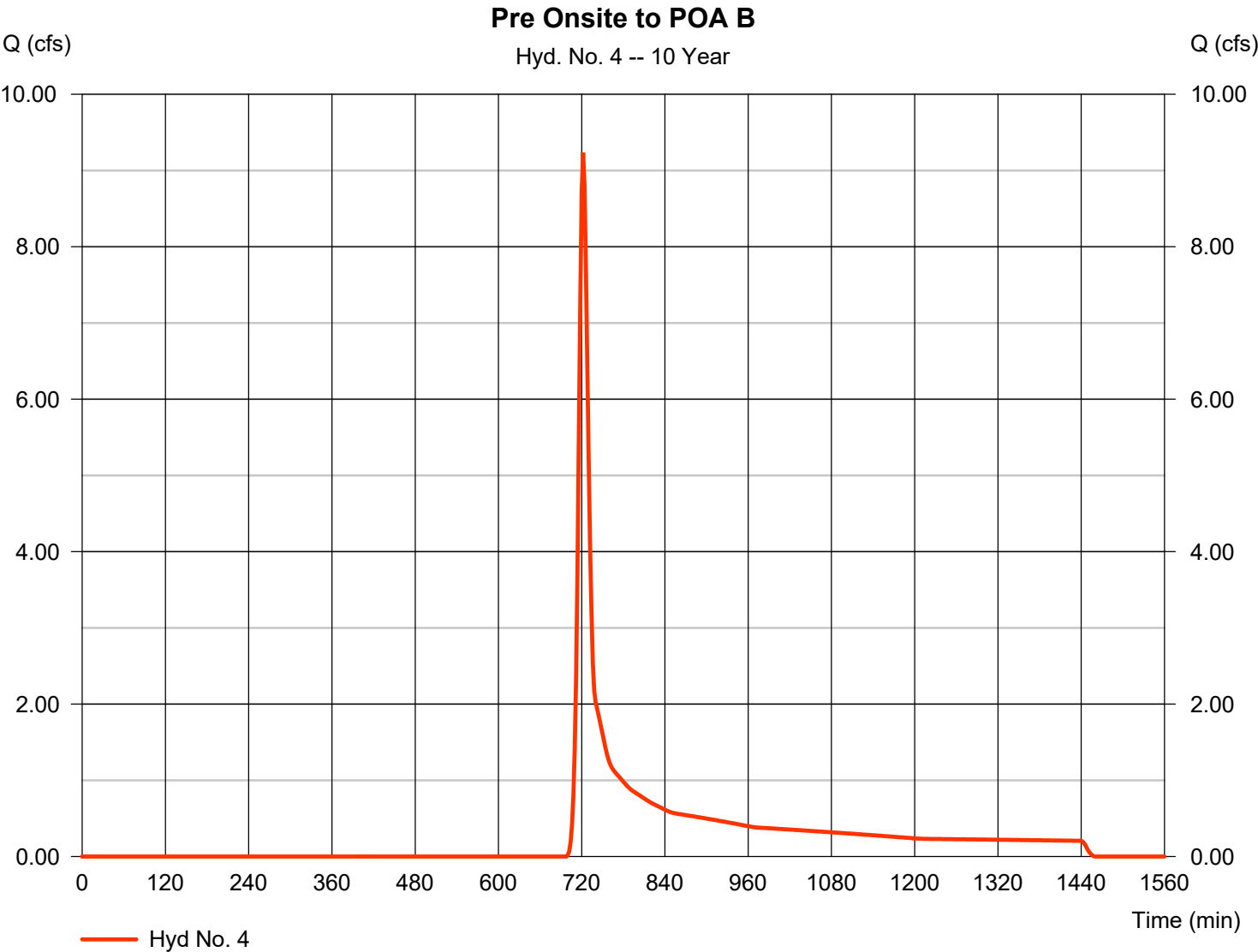
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type	=	SCS Runoff	Peak discharge	=	9.233 cfs
Storm frequency	=	10 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	27,221 cuft
Drainage area	=	6.600 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

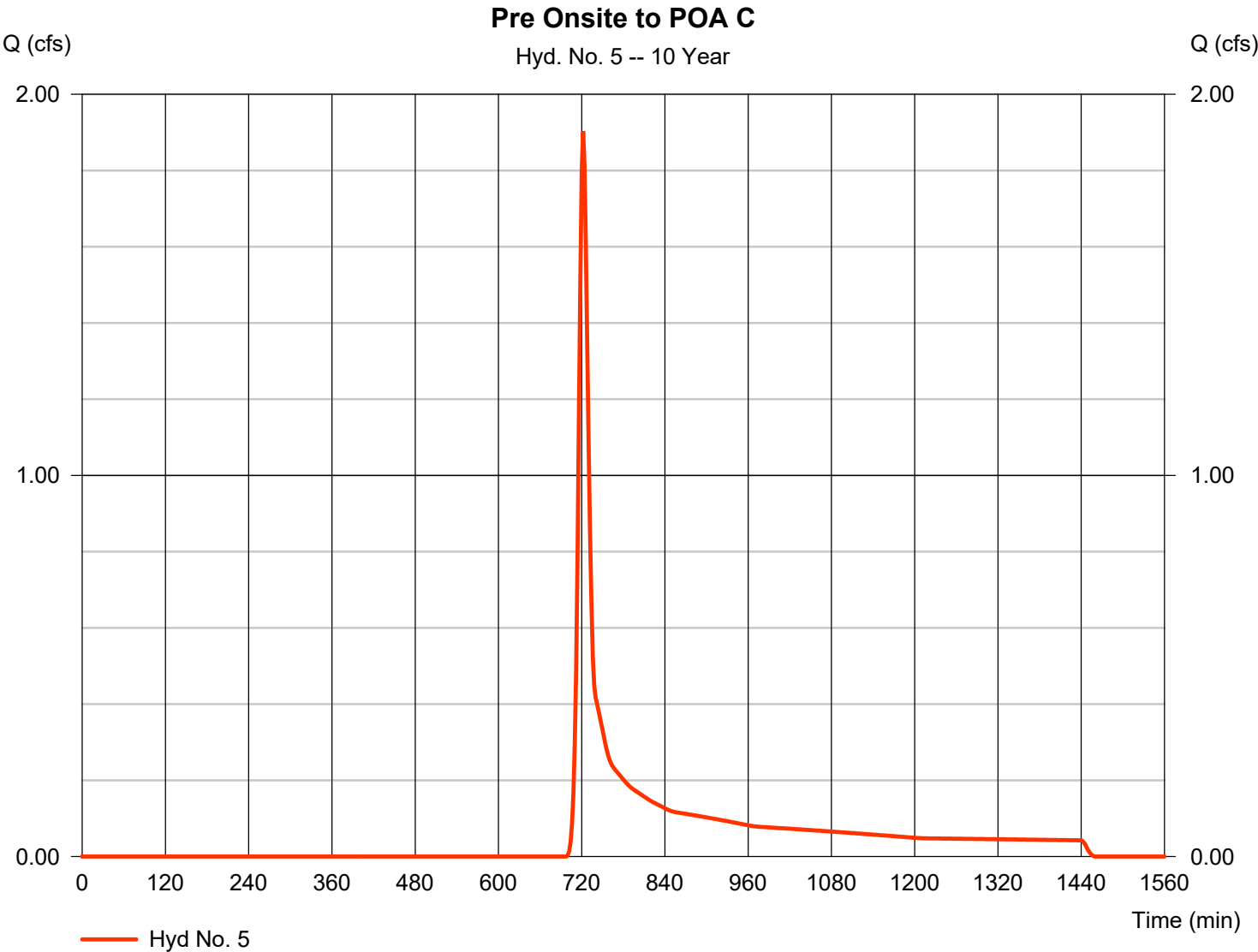


Hydrograph Report

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.902 cfs
Storm frequency	=	10 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	5,609 cuft
Drainage area	=	1.360 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

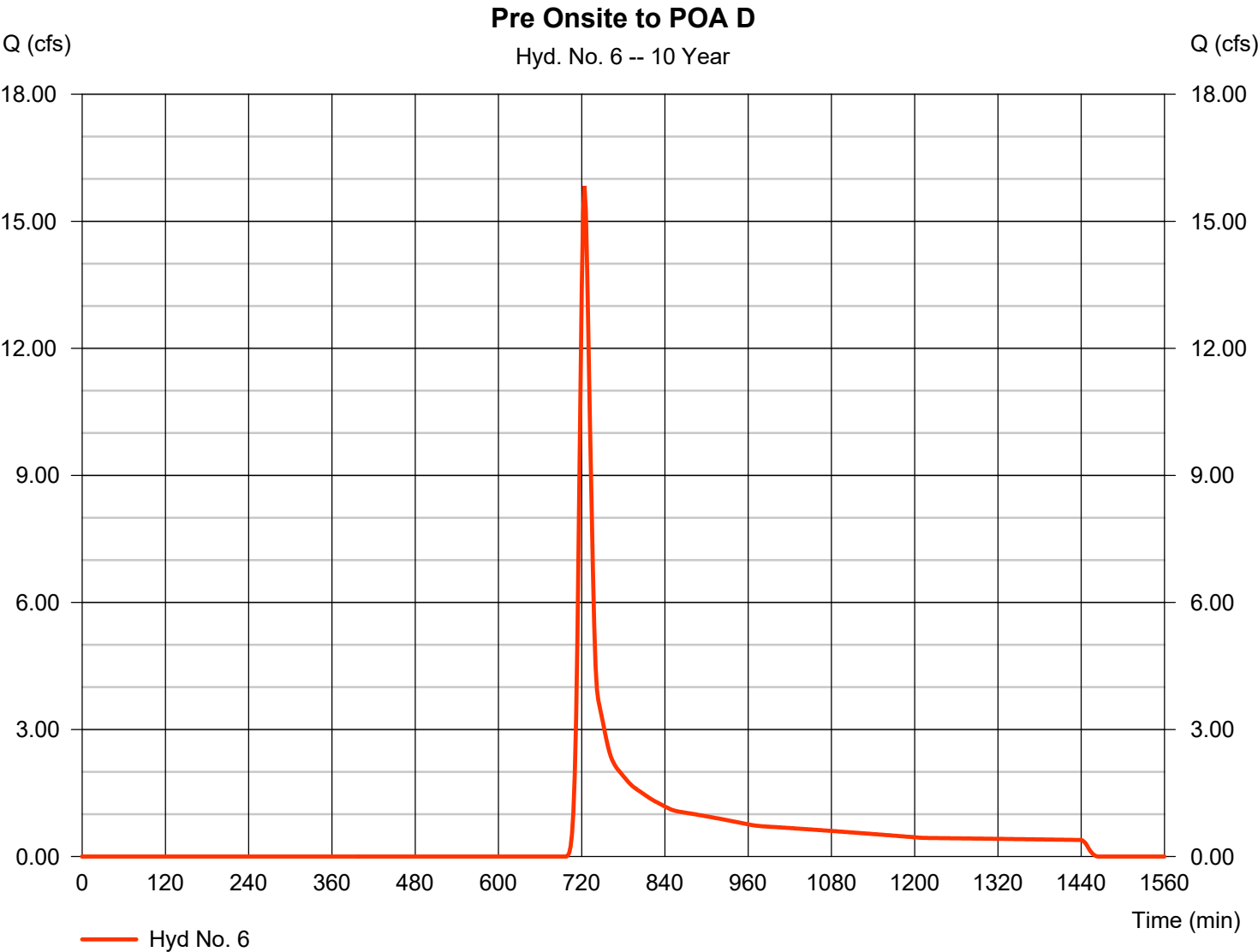
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 15.84 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 51,551 cuft
Drainage area	= 13.220 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 15.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

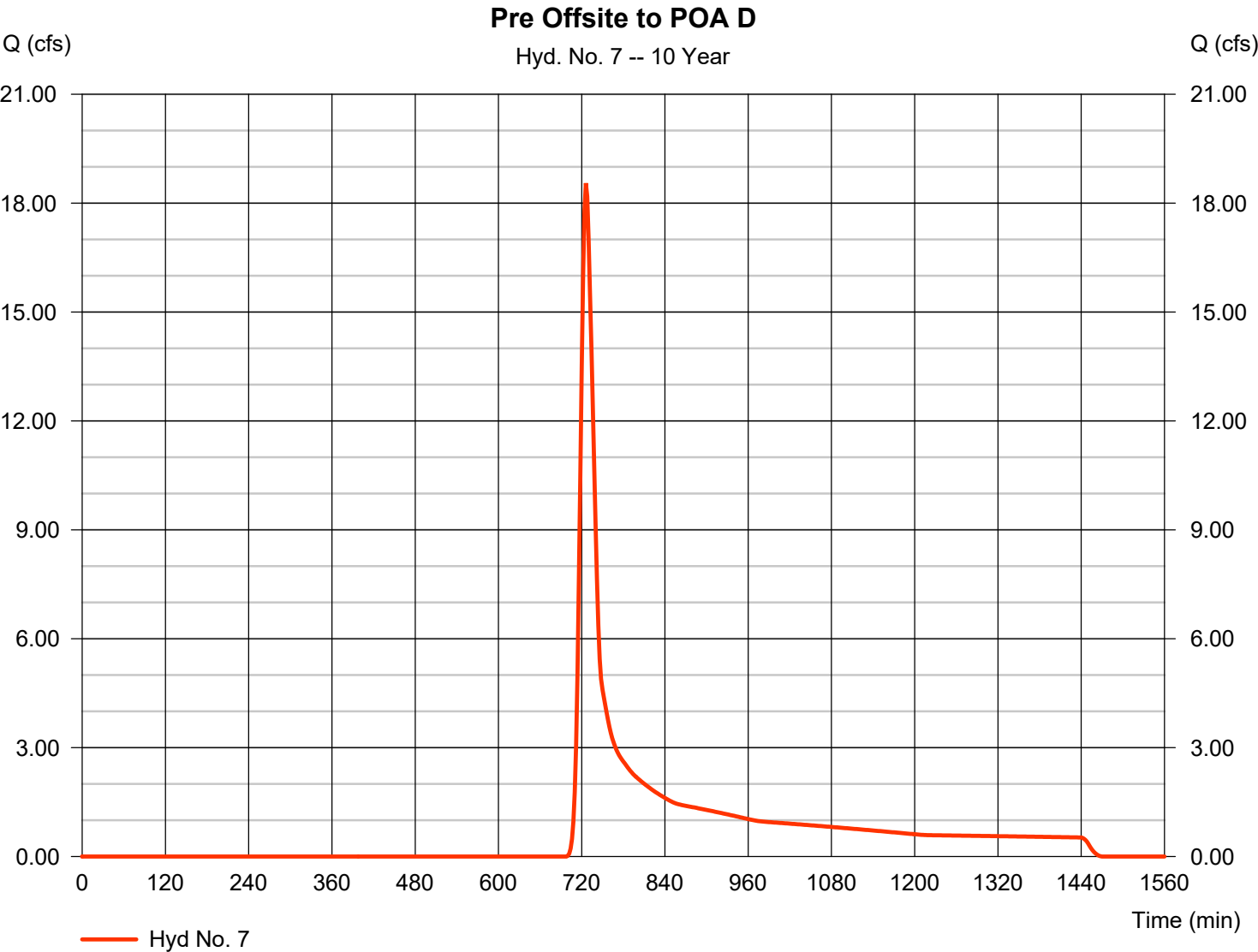
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 18.55 cfs
Storm frequency	= 10 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 69,151 cuft
Drainage area	= 17.290 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 17.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

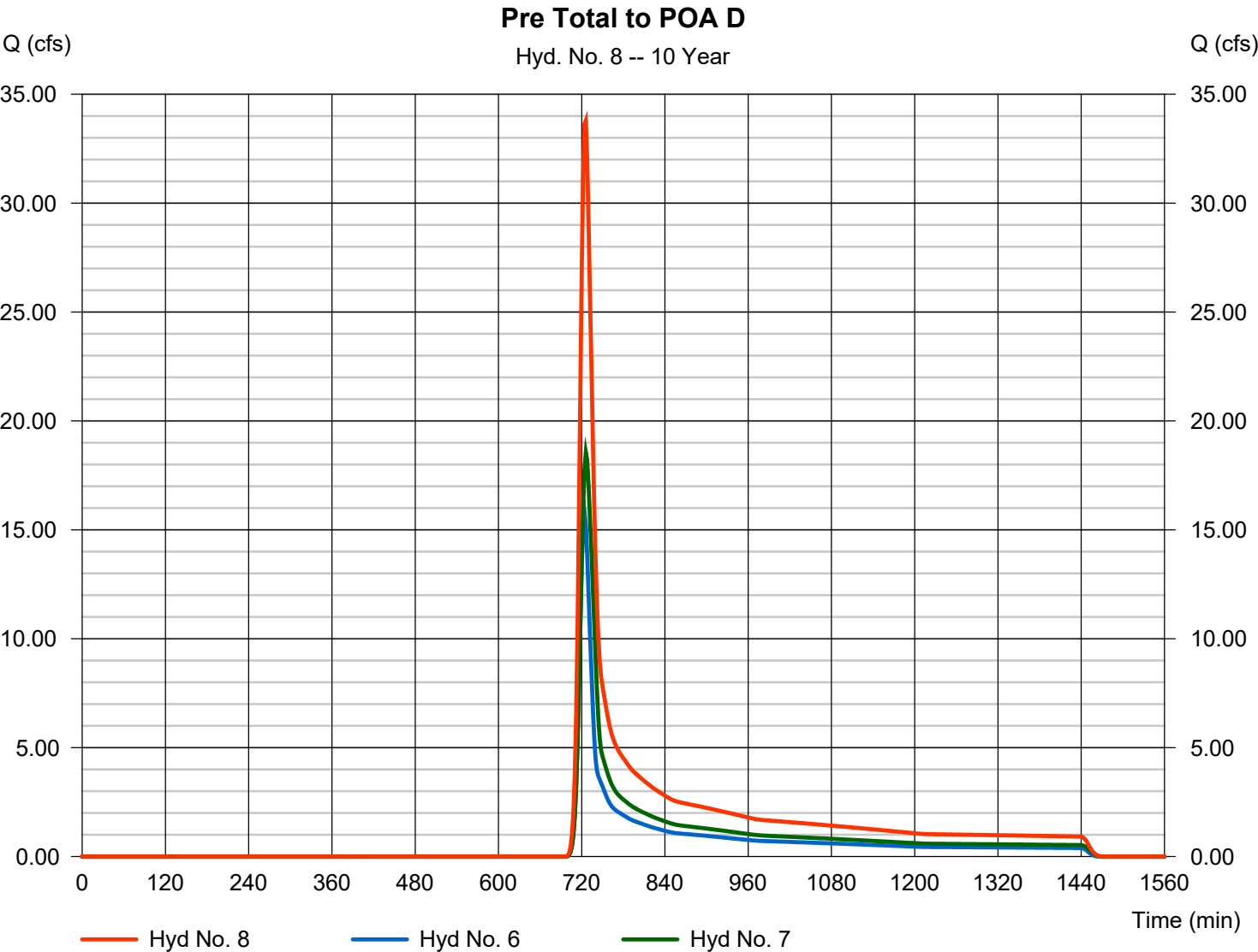


Hydrograph Report

Hyd. No. 8

Pre Total to POA D

Hydrograph type	= Combine	Peak discharge	= 33.79 cfs
Storm frequency	= 10 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 120,702 cuft
Inflow hyds.	= 6, 7	Contrib. drain. area	= 30.510 ac

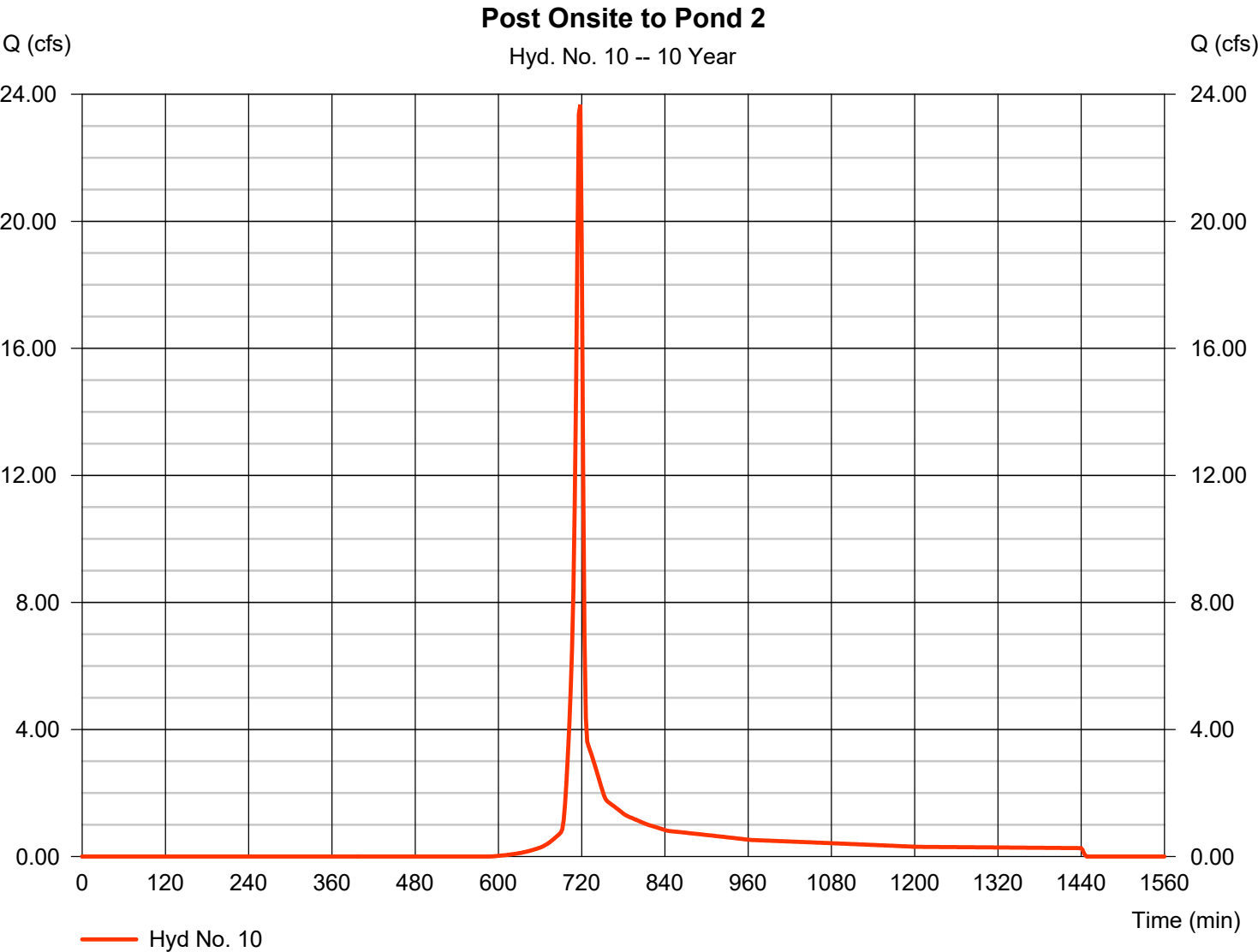


Hydrograph Report

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	=	SCS Runoff	Peak discharge	=	23.67 cfs
Storm frequency	=	10 yrs	Time to peak	=	718 min
Time interval	=	2 min	Hyd. volume	=	47,412 cuft
Drainage area	=	6.530 ac	Curve number	=	69
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	5.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



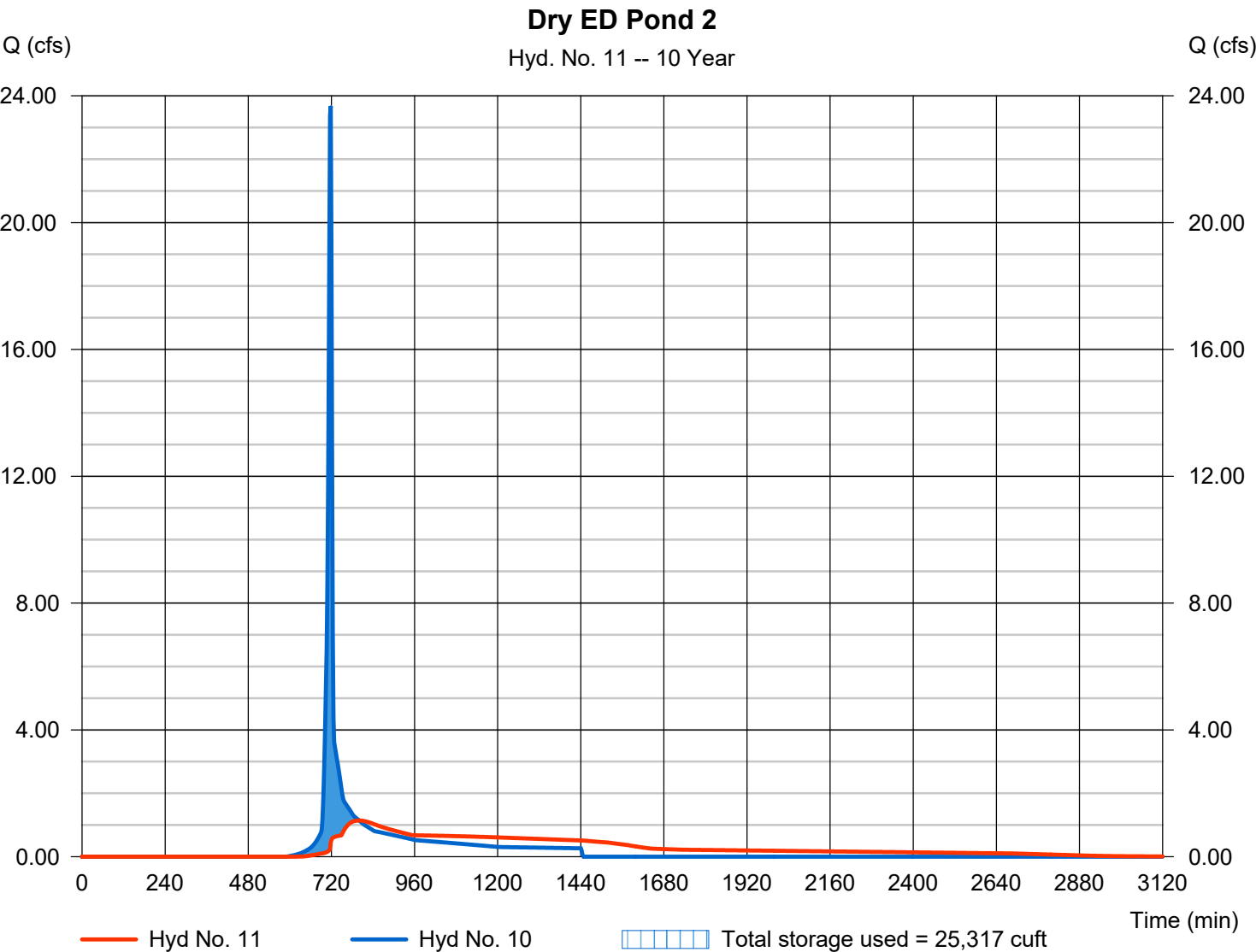
Hydrograph Report

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 1.142 cfs
Storm frequency	= 10 yrs	Time to peak	= 800 min
Time interval	= 2 min	Hyd. volume	= 47,387 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 697.15 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 25,317 cuft

Storage Indication method used.

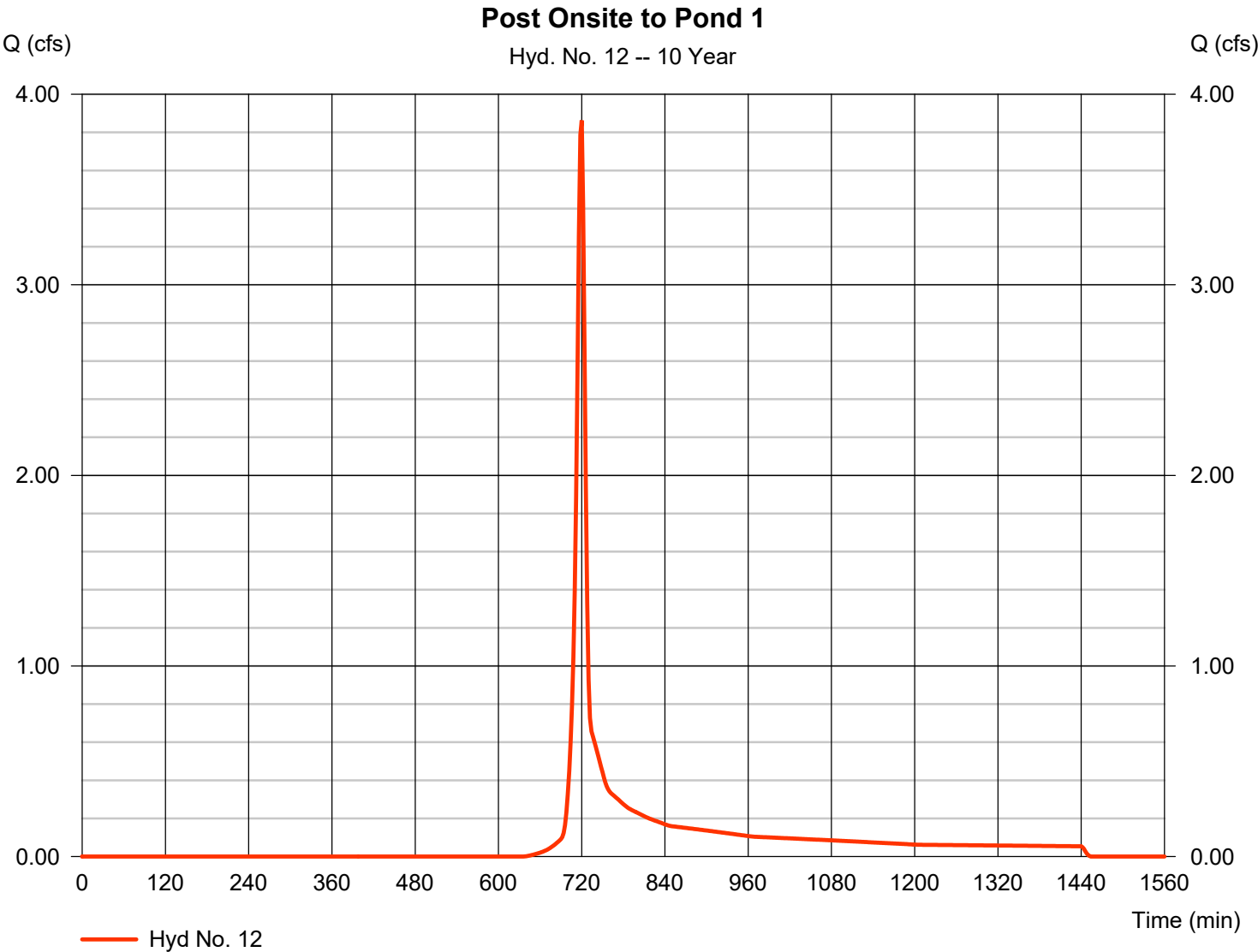


Hydrograph Report

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 3.862 cfs
Storm frequency	= 10 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 8,896 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

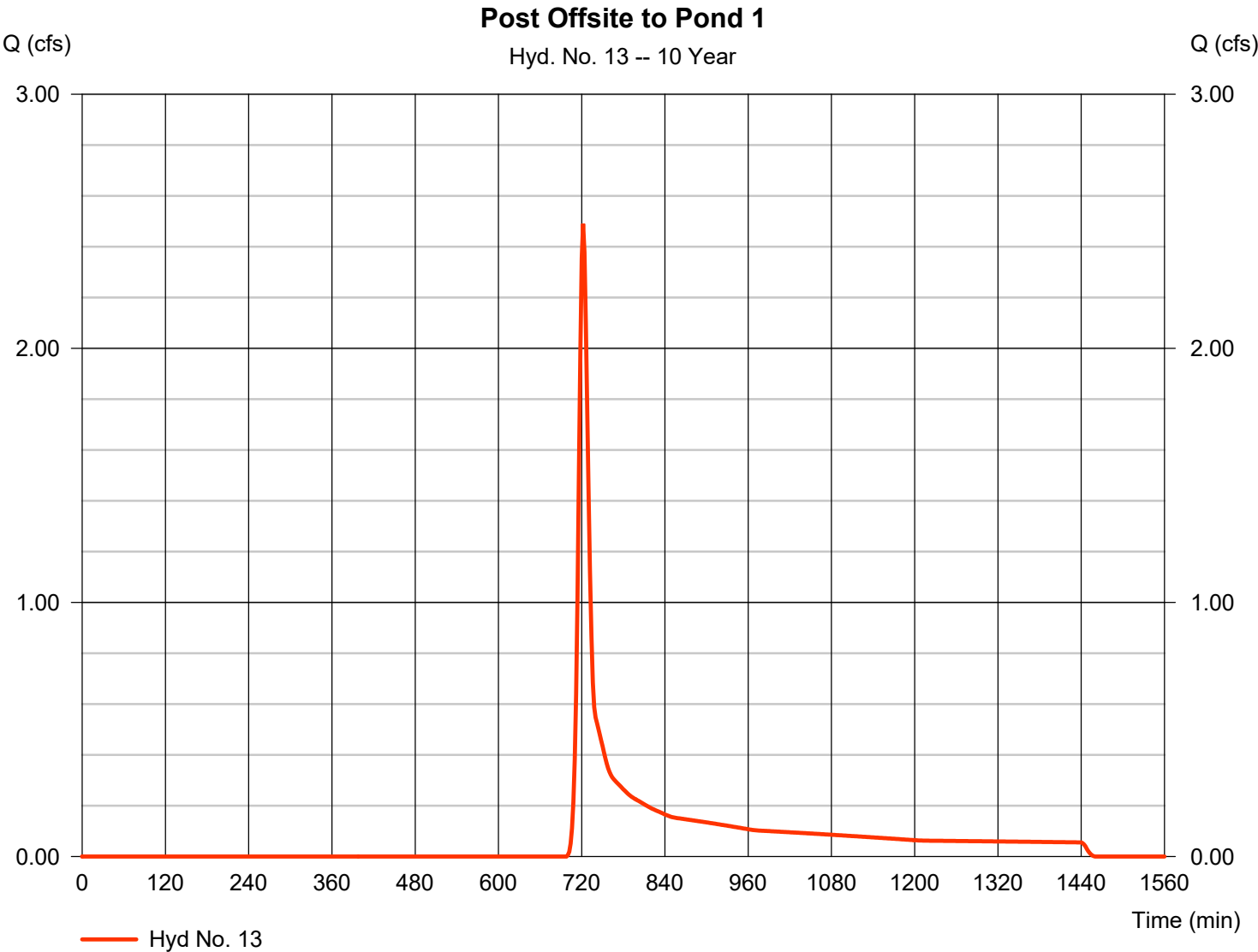


Hydrograph Report

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 2.490 cfs
Storm frequency	= 10 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 7,342 cuft
Drainage area	= 1.780 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

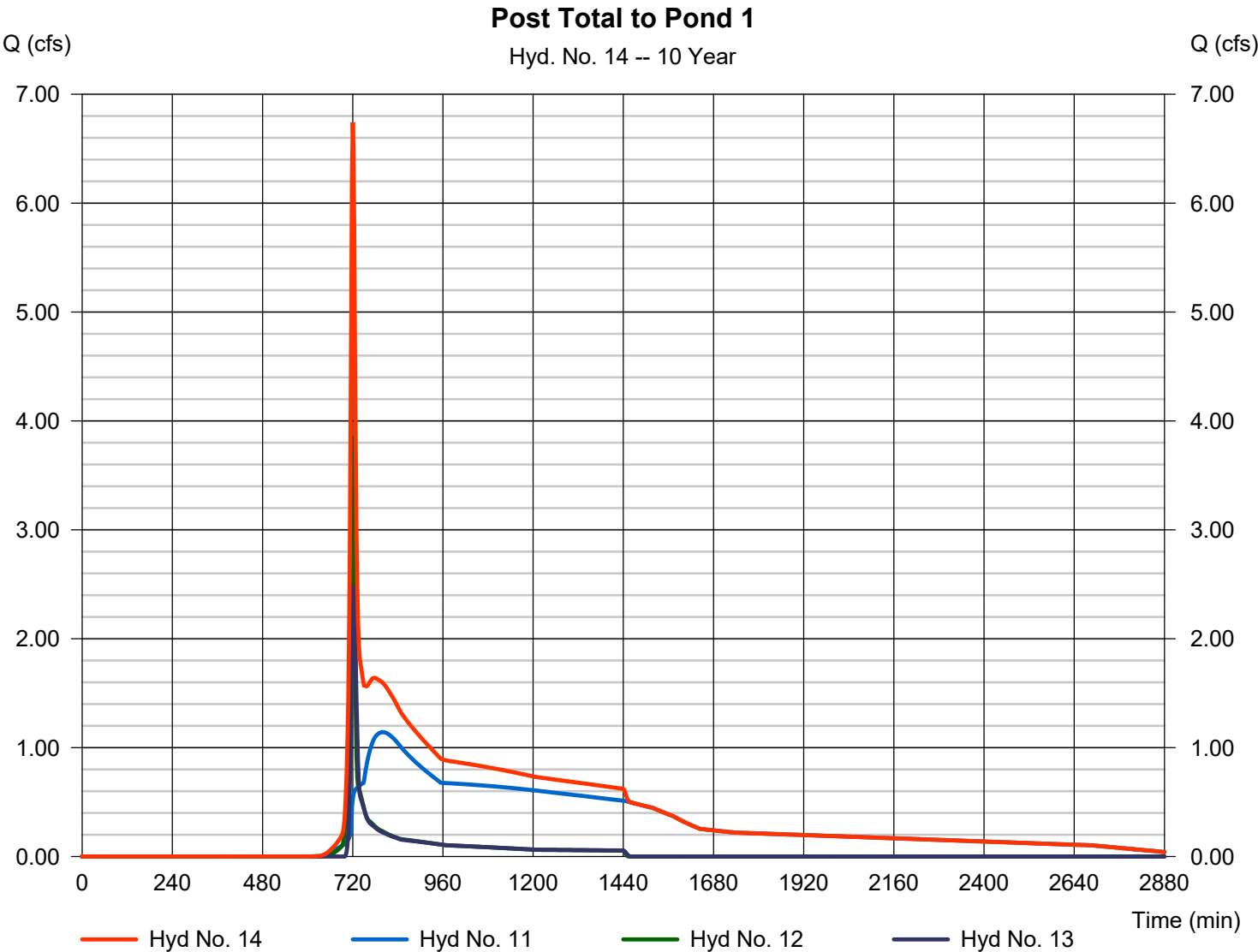


Hydrograph Report

Hyd. No. 14

Post Total to Pond 1

Hydrograph type	= Combine	Peak discharge	= 6.743 cfs
Storm frequency	= 10 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 63,624 cuft
Inflow hyds.	= 11, 12, 13	Contrib. drain. area	= 3.130 ac



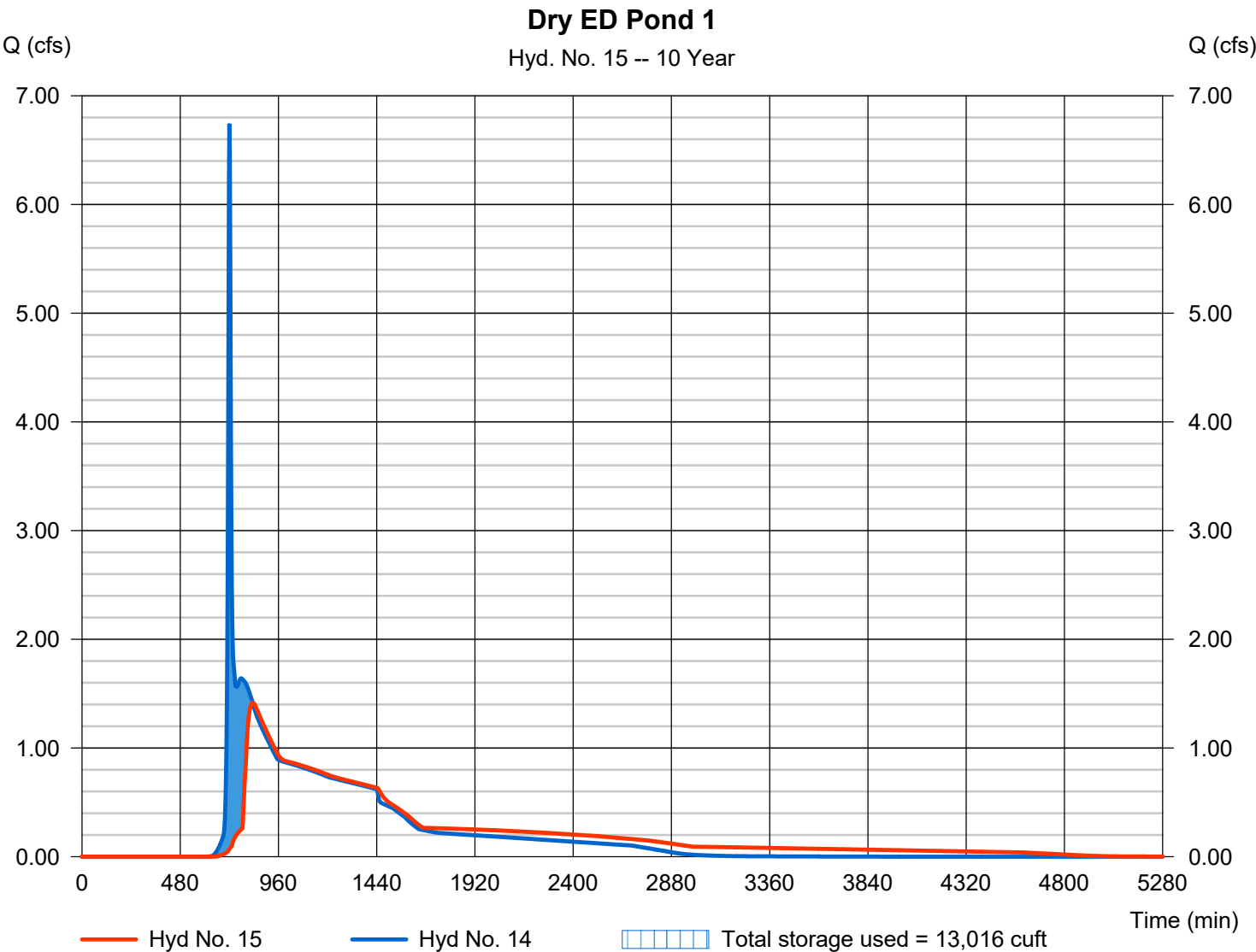
Hydrograph Report

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 1.416 cfs
Storm frequency	= 10 yrs	Time to peak	= 834 min
Time interval	= 2 min	Hyd. volume	= 63,608 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 685.30 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 13,016 cuft

Storage Indication method used.

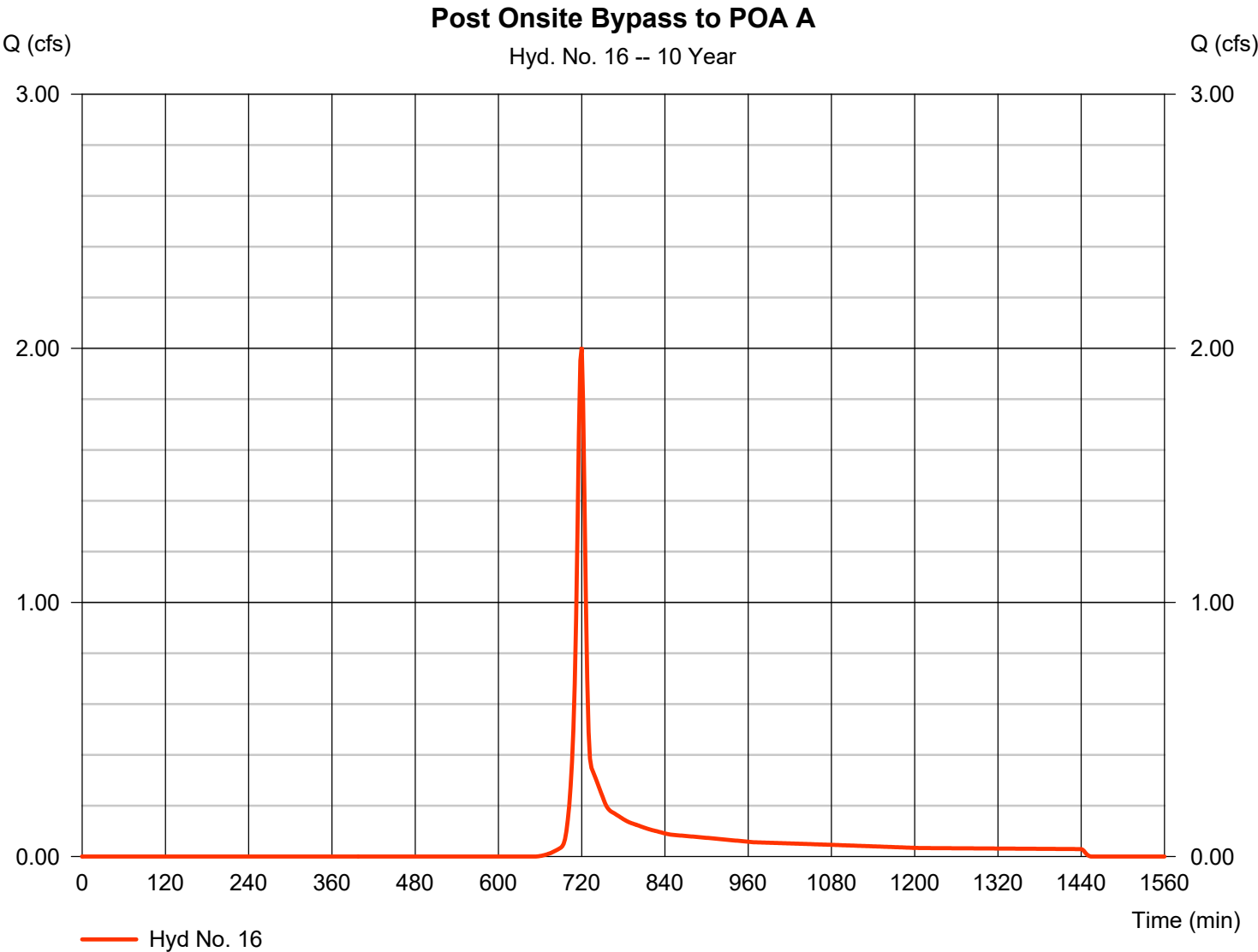


Hydrograph Report

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.005 cfs
Storm frequency	=	10 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	4,648 cuft
Drainage area	=	0.770 ac	Curve number	=	63
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

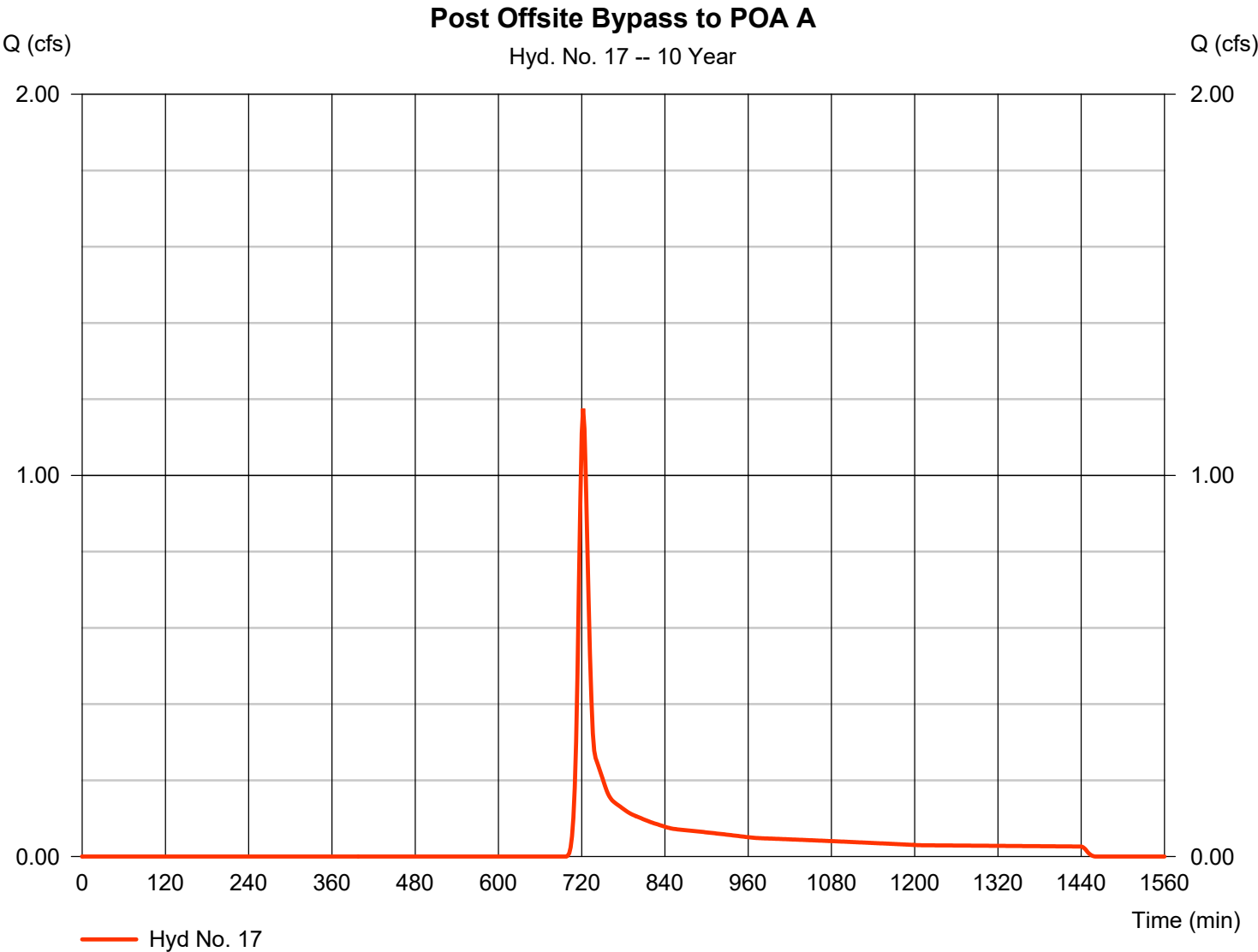
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.175 cfs
Storm frequency	=	10 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	3,465 cuft
Drainage area	=	0.840 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

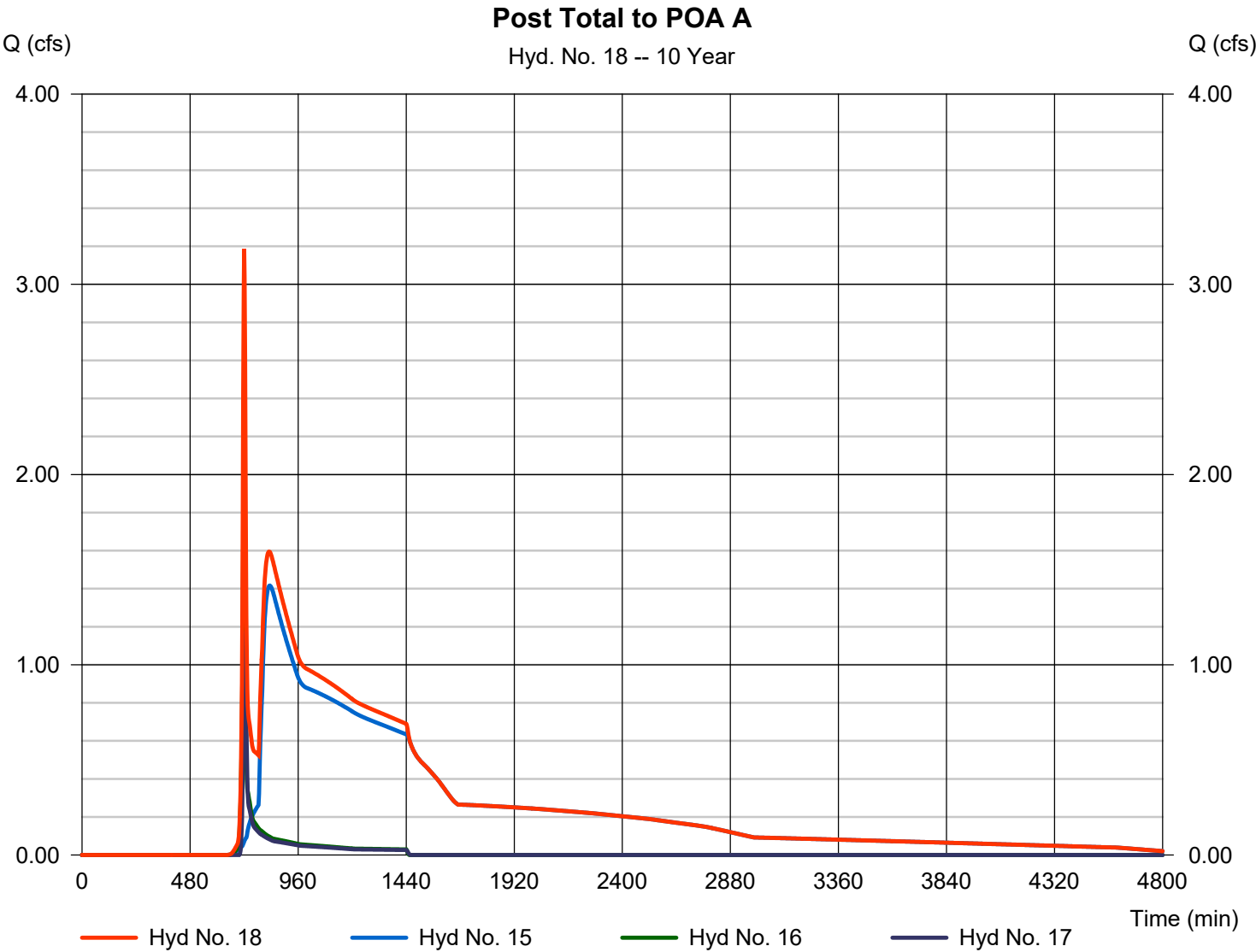


Hydrograph Report

Hyd. No. 18

Post Total to POA A

Hydrograph type	= Combine	Peak discharge	= 3.186 cfs
Storm frequency	= 10 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 71,720 cuft
Inflow hyds.	= 15, 16, 17	Contrib. drain. area	= 1.610 ac

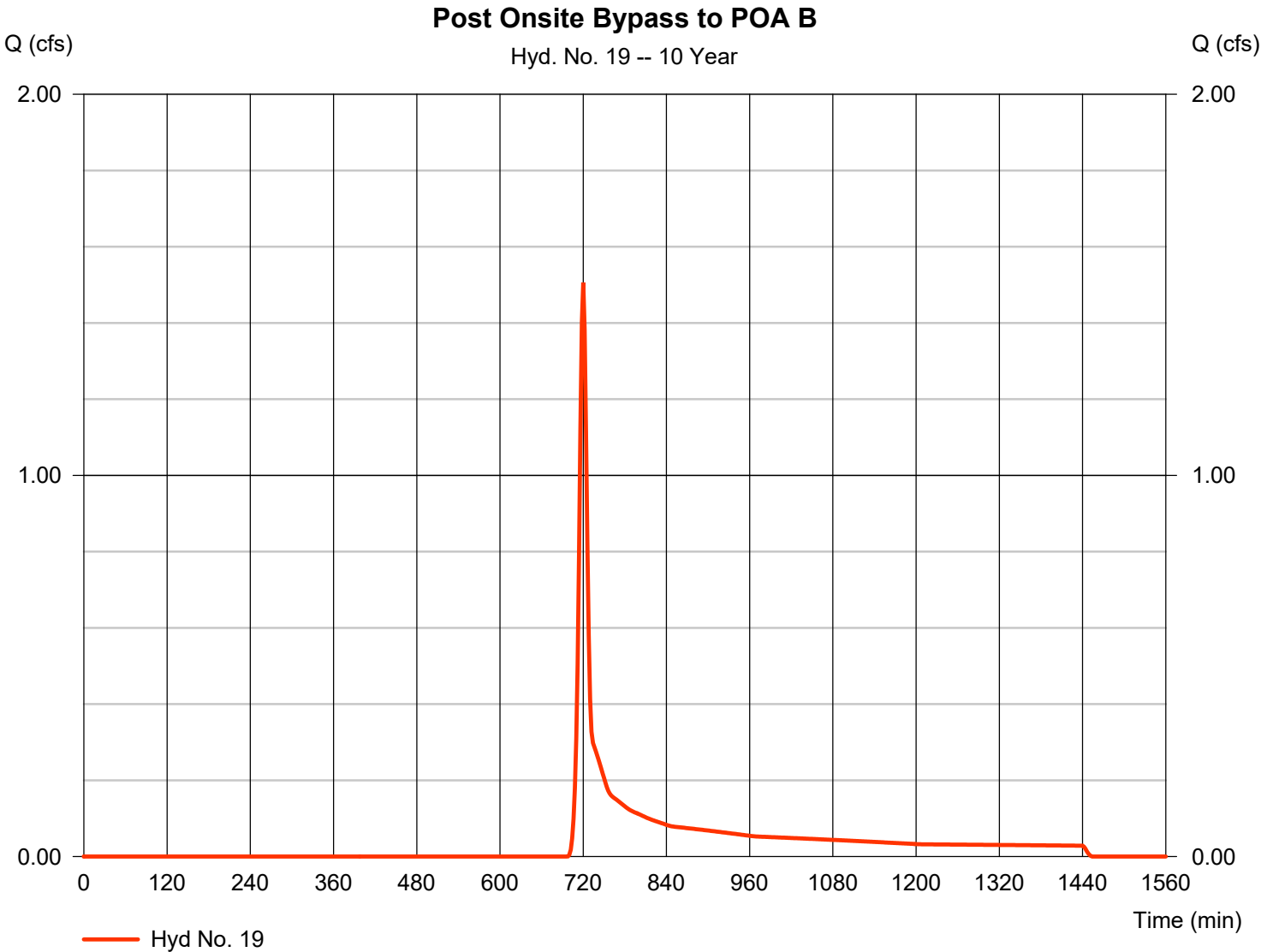


Hydrograph Report

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.506 cfs
Storm frequency	=	10 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	3,760 cuft
Drainage area	=	0.940 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

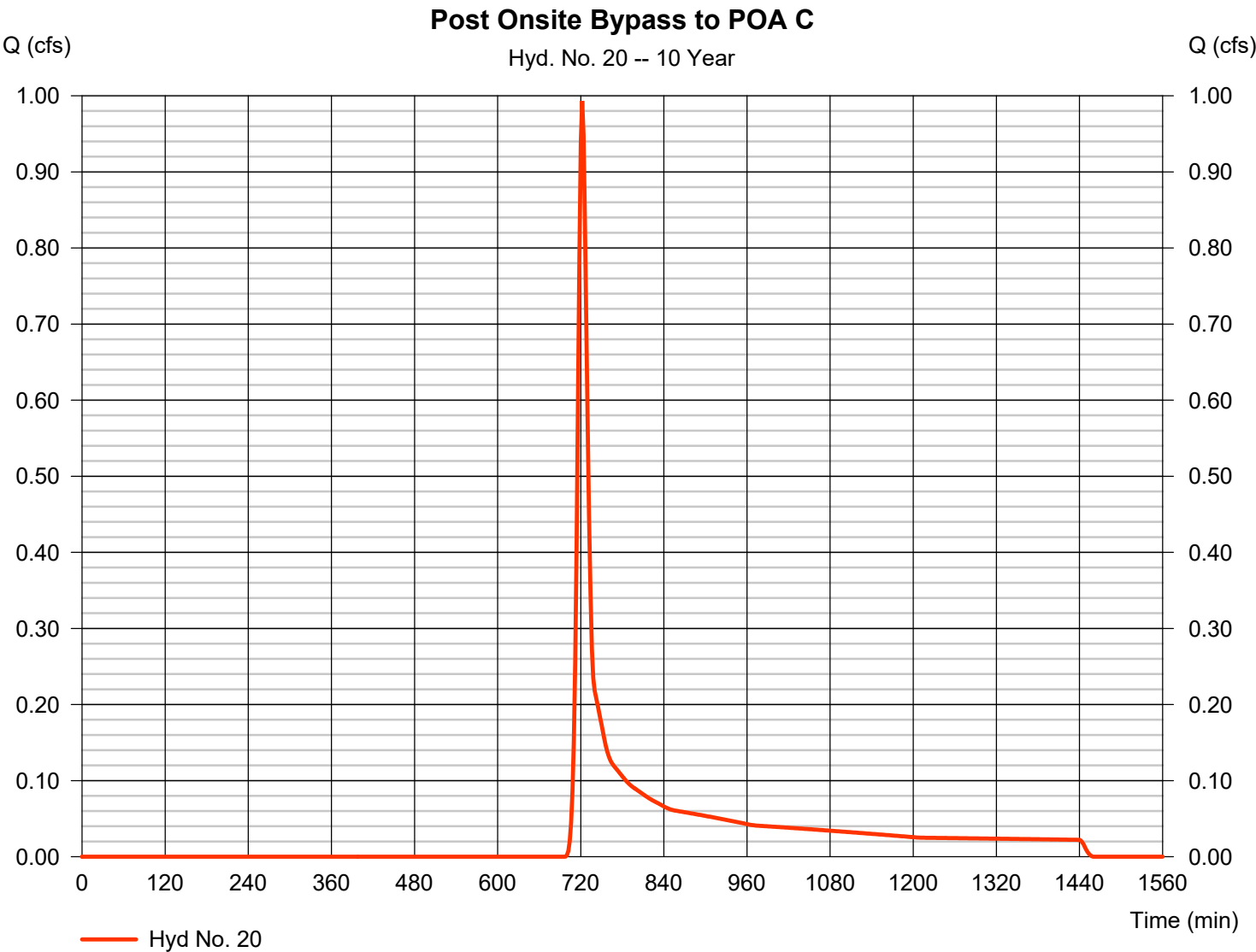
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	=	SCS Runoff	Peak discharge	=	0.993 cfs
Storm frequency	=	10 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	2,928 cuft
Drainage area	=	0.710 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

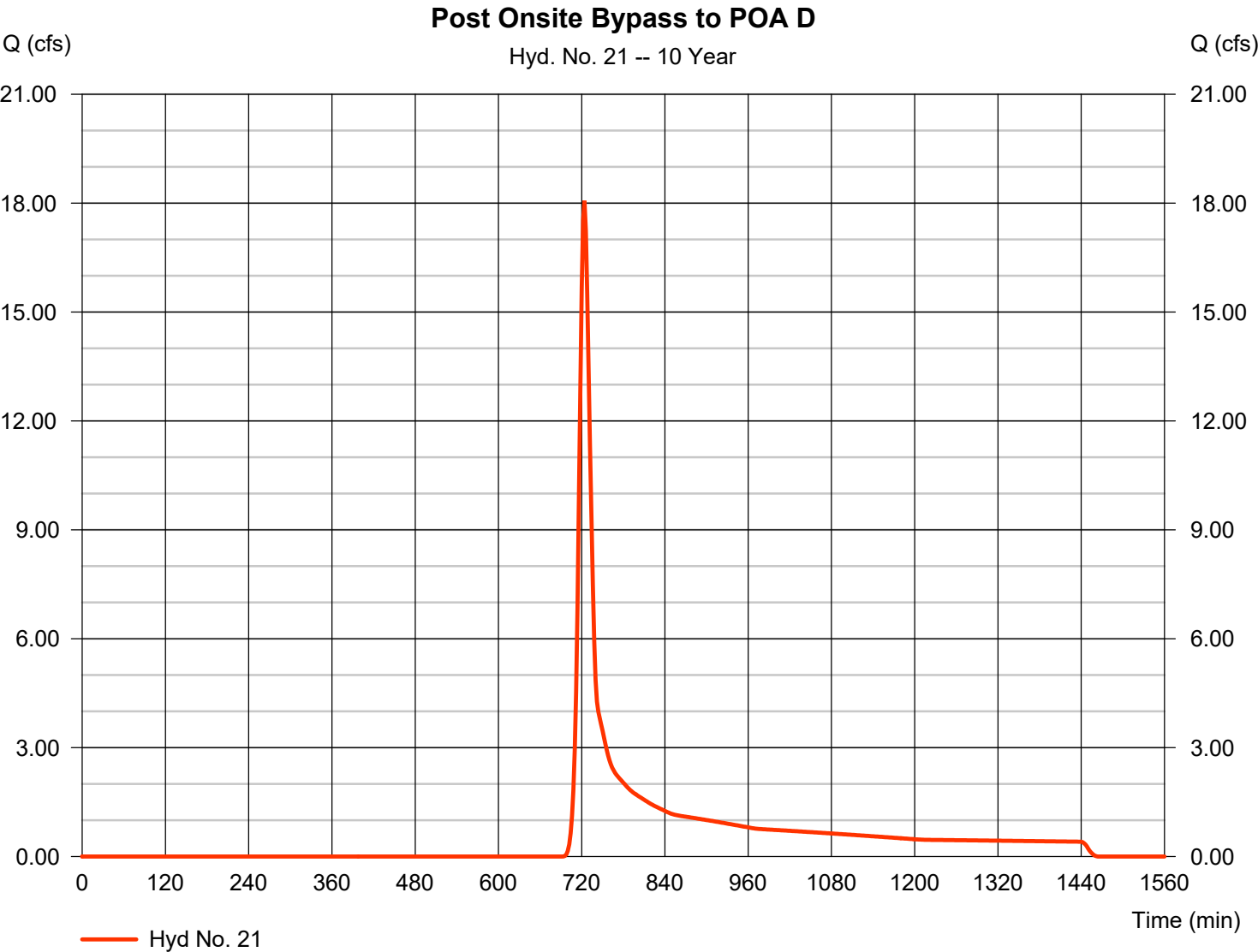


Hydrograph Report

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	18.06 cfs
Storm frequency	=	10 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	56,593 cuft
Drainage area	=	12.950 ac	Curve number	=	57
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	15.00 min
Total precip.	=	5.24 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

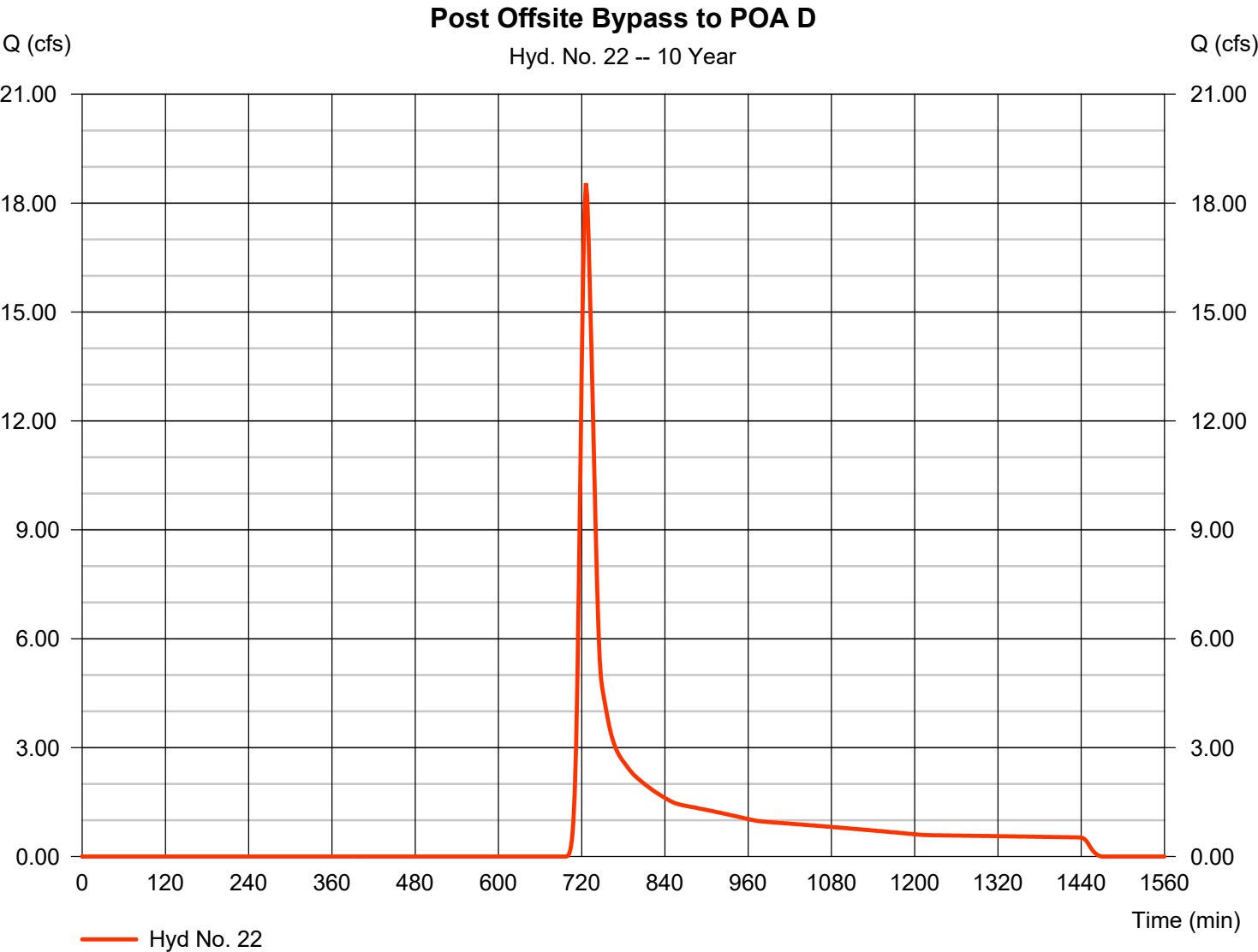


Hydrograph Report

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 18.55 cfs
Storm frequency	= 10 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 69,151 cuft
Drainage area	= 17.290 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 17.00 min
Total precip.	= 5.24 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

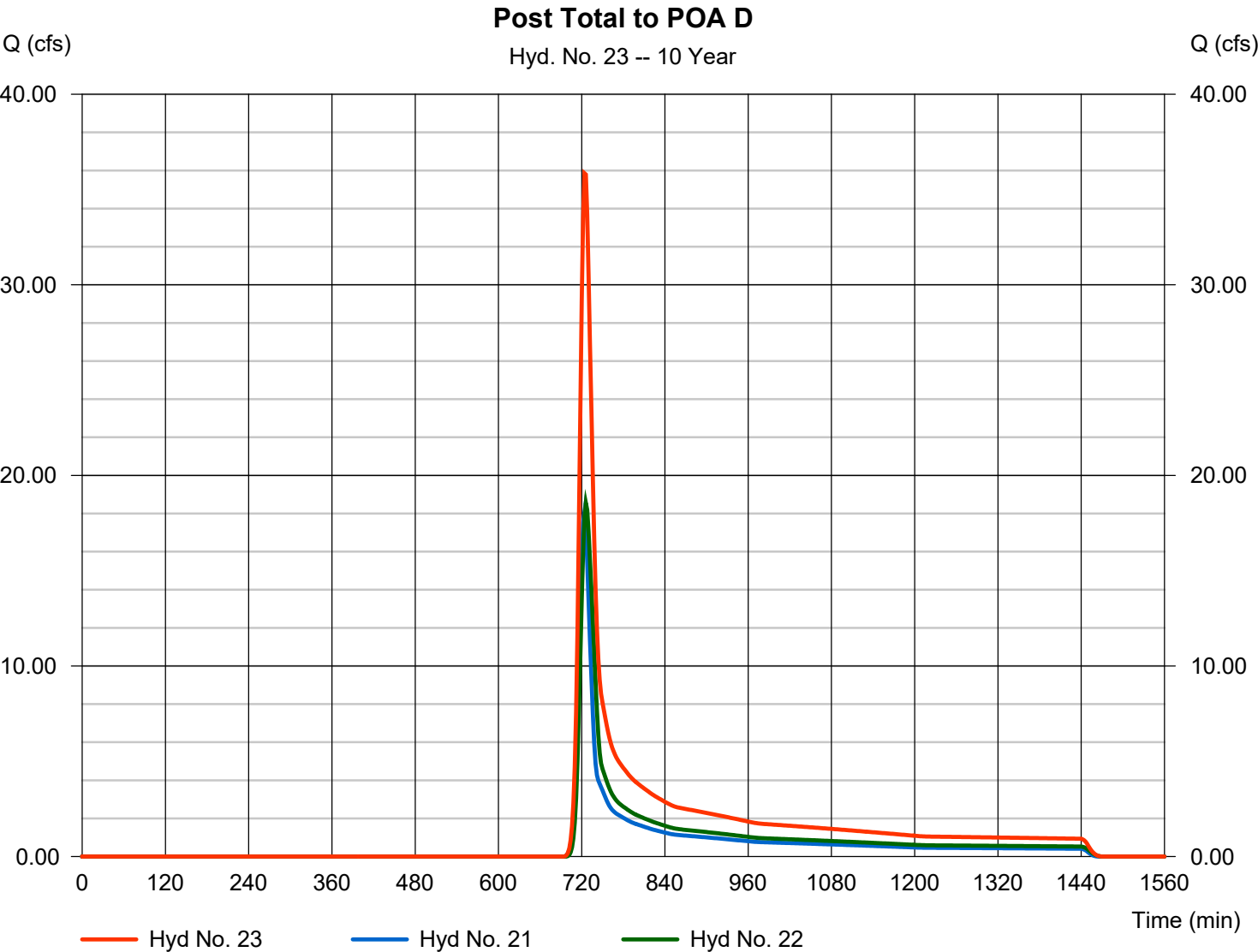
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type	= Combine	Peak discharge	= 35.86 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 125,744 cuft
Inflow hyds.	= 21, 22	Contrib. drain. area	= 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	5.211	2	720	12,367	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	5.859	2	722	16,203	-----	-----	-----	Pre Offsite to POA A
3	Combine	10.88	2	720	28,570	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	14.70	2	722	40,663	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	3.030	2	722	8,379	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	25.34	2	724	77,006	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	30.02	2	726	103,296	-----	-----	-----	Pre Offsite to POA D
8	Combine	54.68	2	724	180,302	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	31.81	2	718	64,120	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	3.757	2	738	64,094	10	697.58	29,483	Dry ED Pond 2
12	SCS Runoff	5.378	2	720	12,313	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	3.965	2	722	10,967	-----	-----	-----	Post Offsite to Pond 1
14	Combine	10.76	2	722	87,374	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	4.344	2	756	87,358	14	685.71	15,023	Dry ED Pond 1
16	SCS Runoff	2.841	2	720	6,517	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	1.871	2	722	5,175	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	4.881	2	754	99,050	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	2.367	2	720	5,616	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	1.582	2	722	4,374	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	27.86	2	724	83,010	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	30.02	2	726	103,296	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	57.21	2	724	186,307	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 25 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

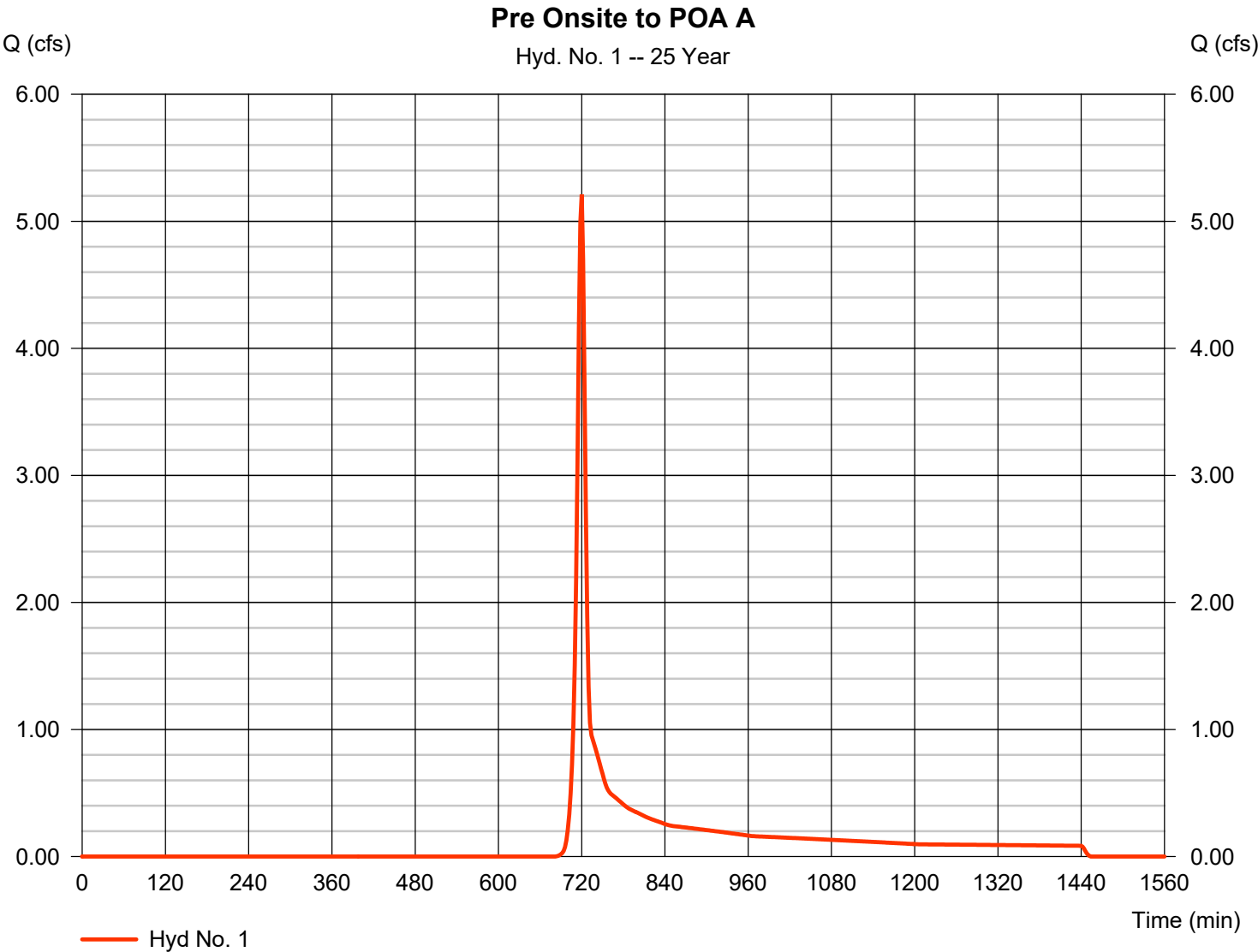
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 5.211 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 12,367 cuft
Drainage area	= 2.070 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 9.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

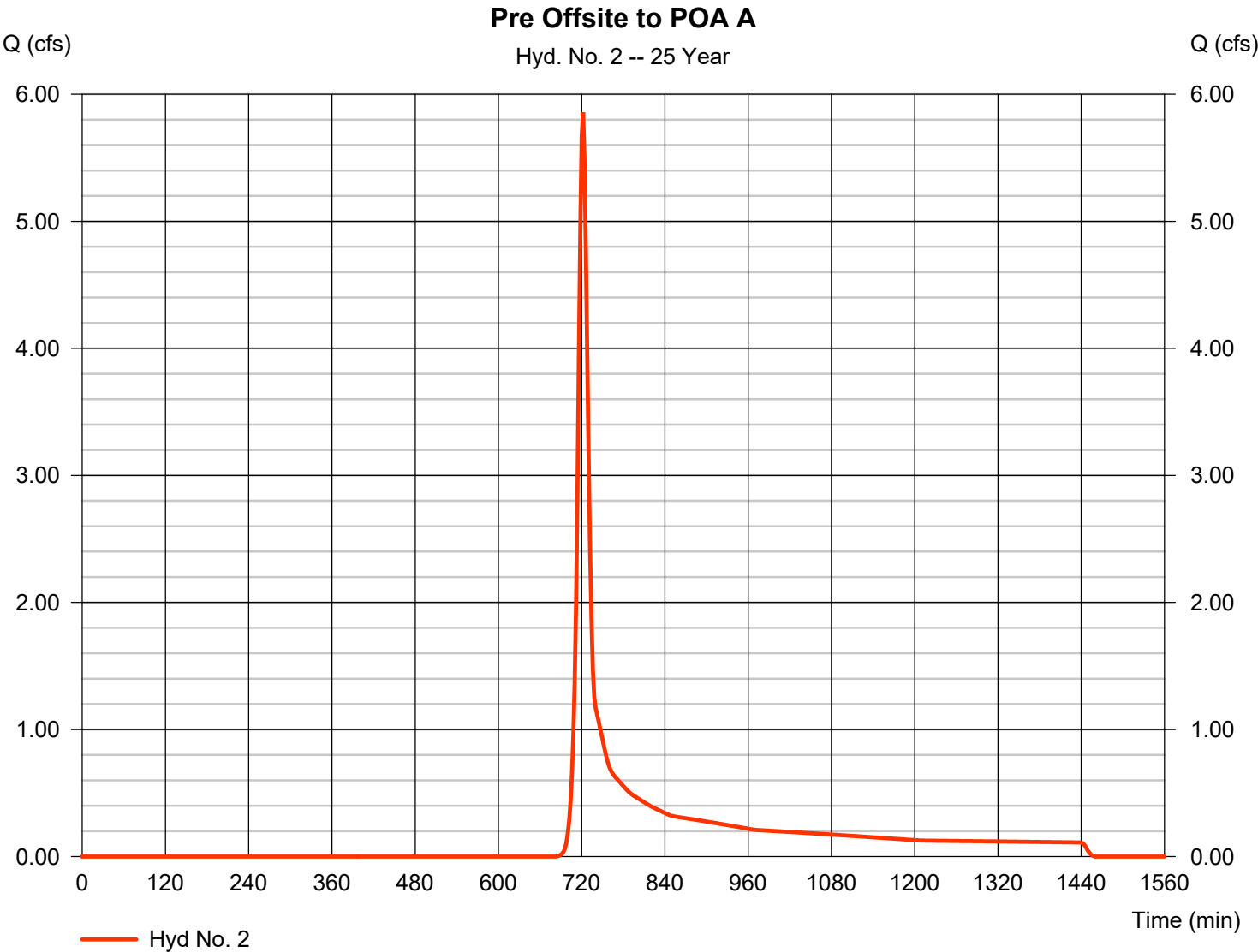


Hydrograph Report

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	5.859 cfs
Storm frequency	=	25 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	16,203 cuft
Drainage area	=	2.630 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.00 min
Total precip.	=	6.22 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

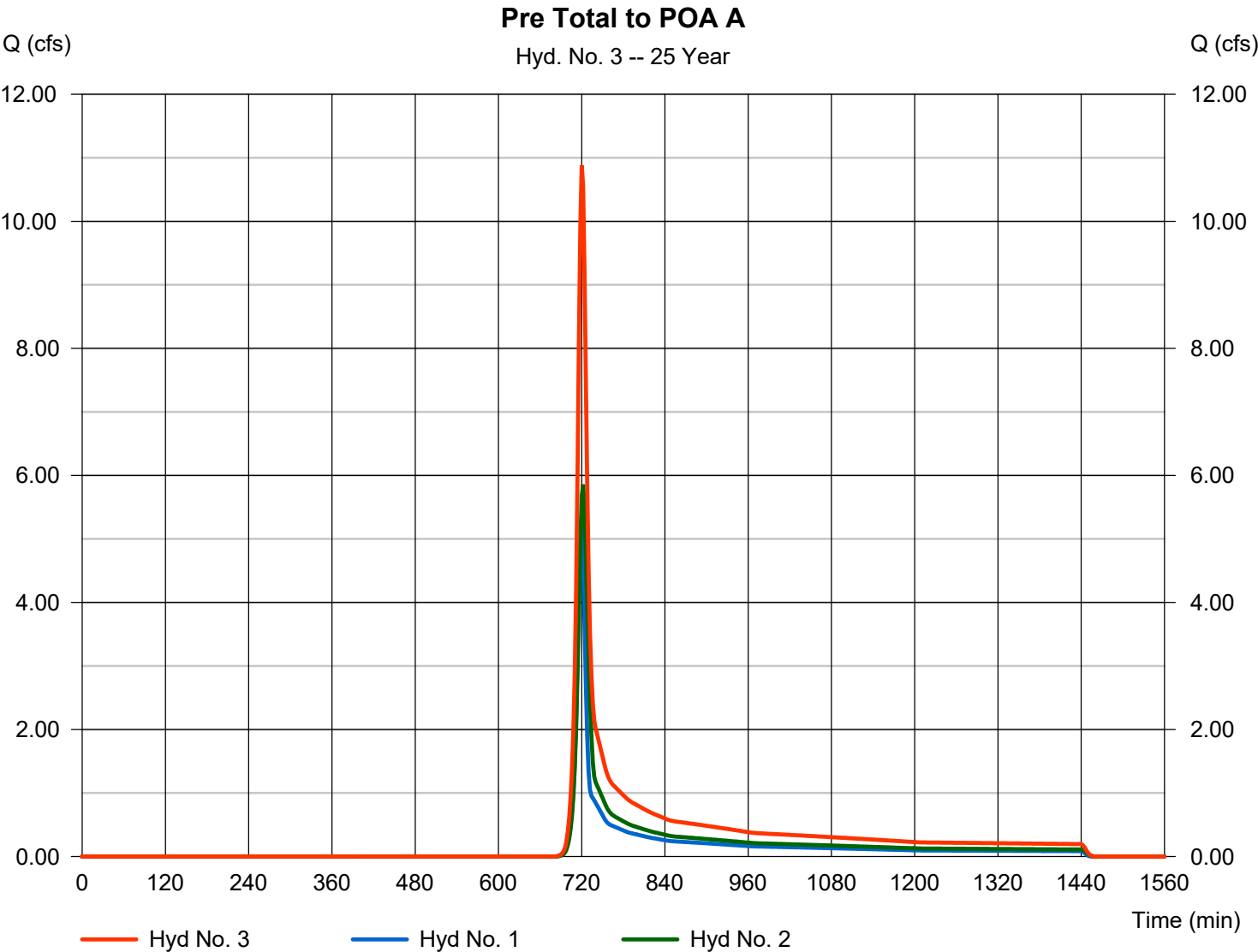


Hydrograph Report

Hyd. No. 3

Pre Total to POA A

Hydrograph type	= Combine	Peak discharge	= 10.88 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 28,570 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	= 4.700 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

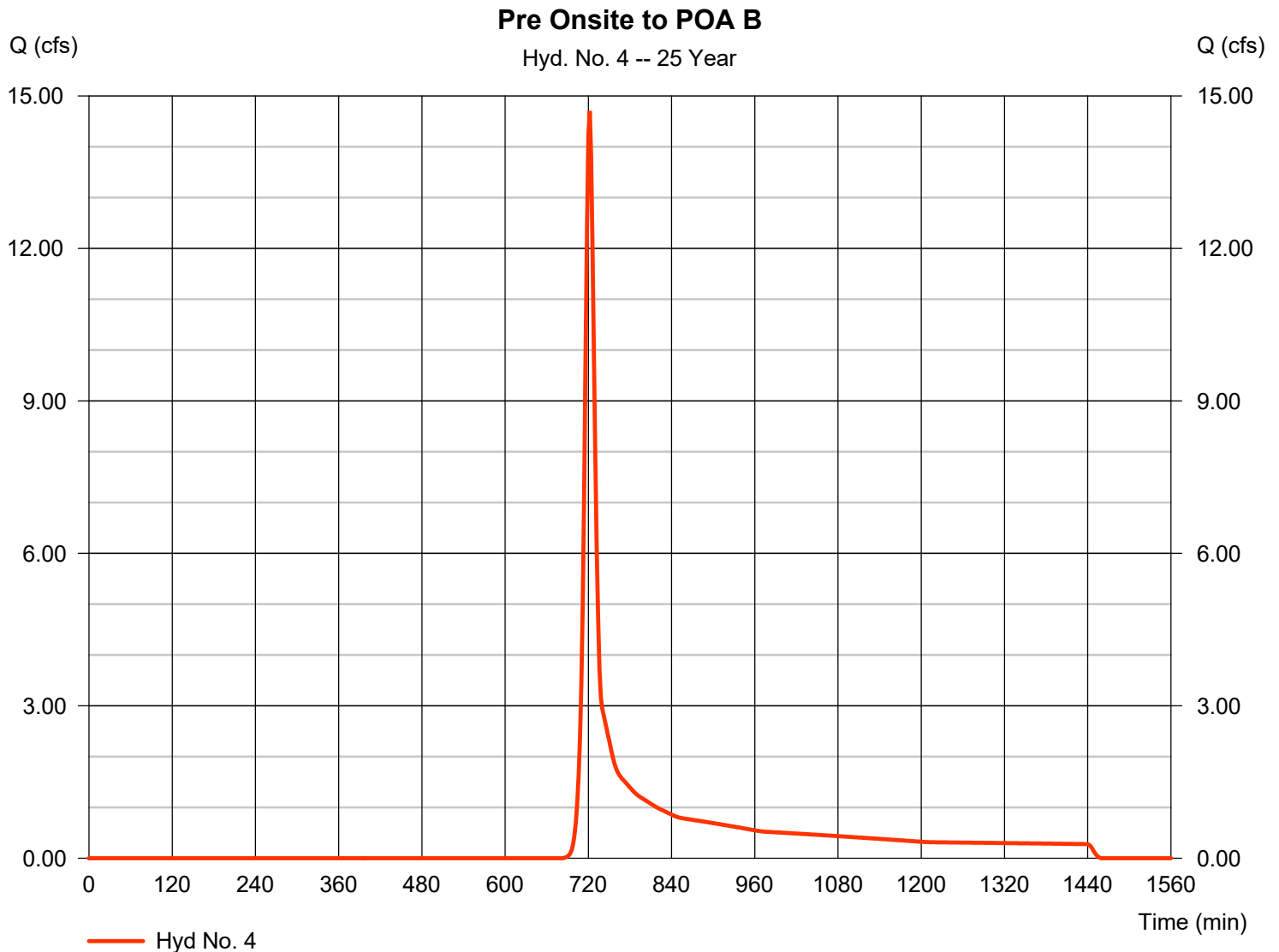
Wednesday, 06 / 3 / 2026

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type = SCS Runoff
Storm frequency = 25 yrs
Time interval = 2 min
Drainage area = 6.600 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 6.22 in
Storm duration = 24 hrs

Peak discharge = 14.70 cfs
Time to peak = 722 min
Hyd. volume = 40,663 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 11.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

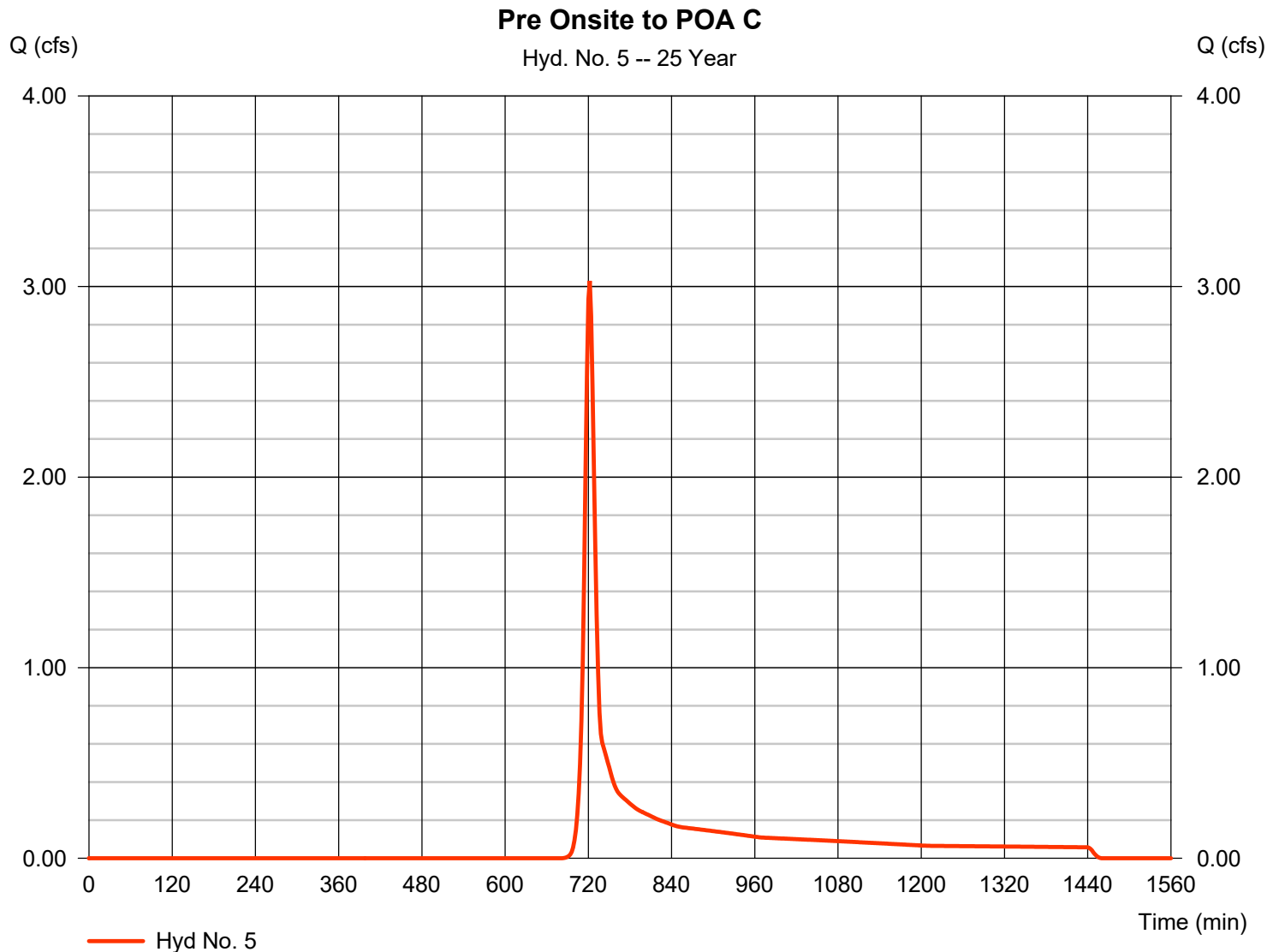
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 3.030 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 8,379 cuft
Drainage area	= 1.360 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

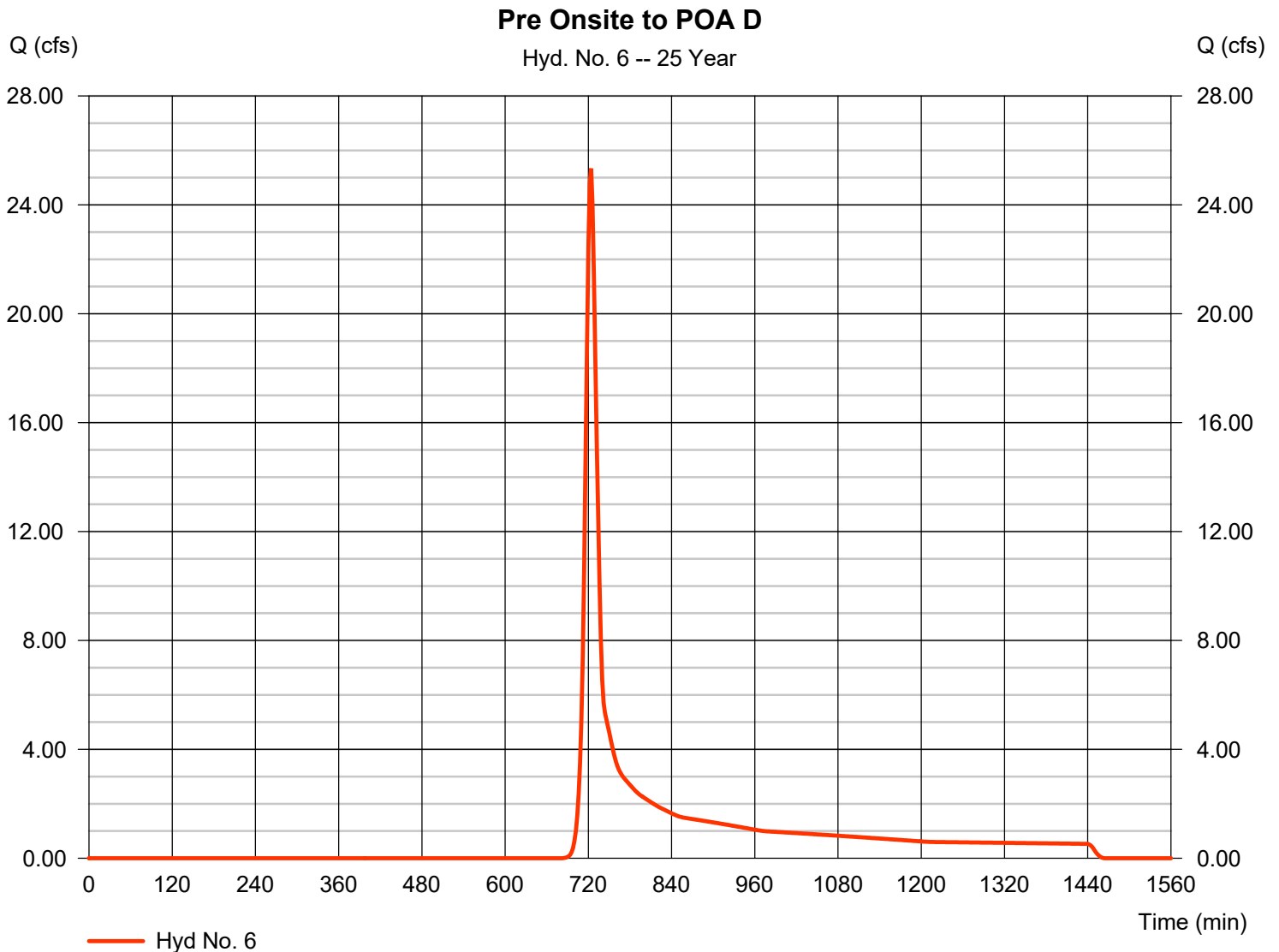
Wednesday, 06 / 3 / 2026

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type = SCS Runoff
Storm frequency = 25 yrs
Time interval = 2 min
Drainage area = 13.220 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 6.22 in
Storm duration = 24 hrs

Peak discharge = 25.34 cfs
Time to peak = 724 min
Hyd. volume = 77,006 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 15.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

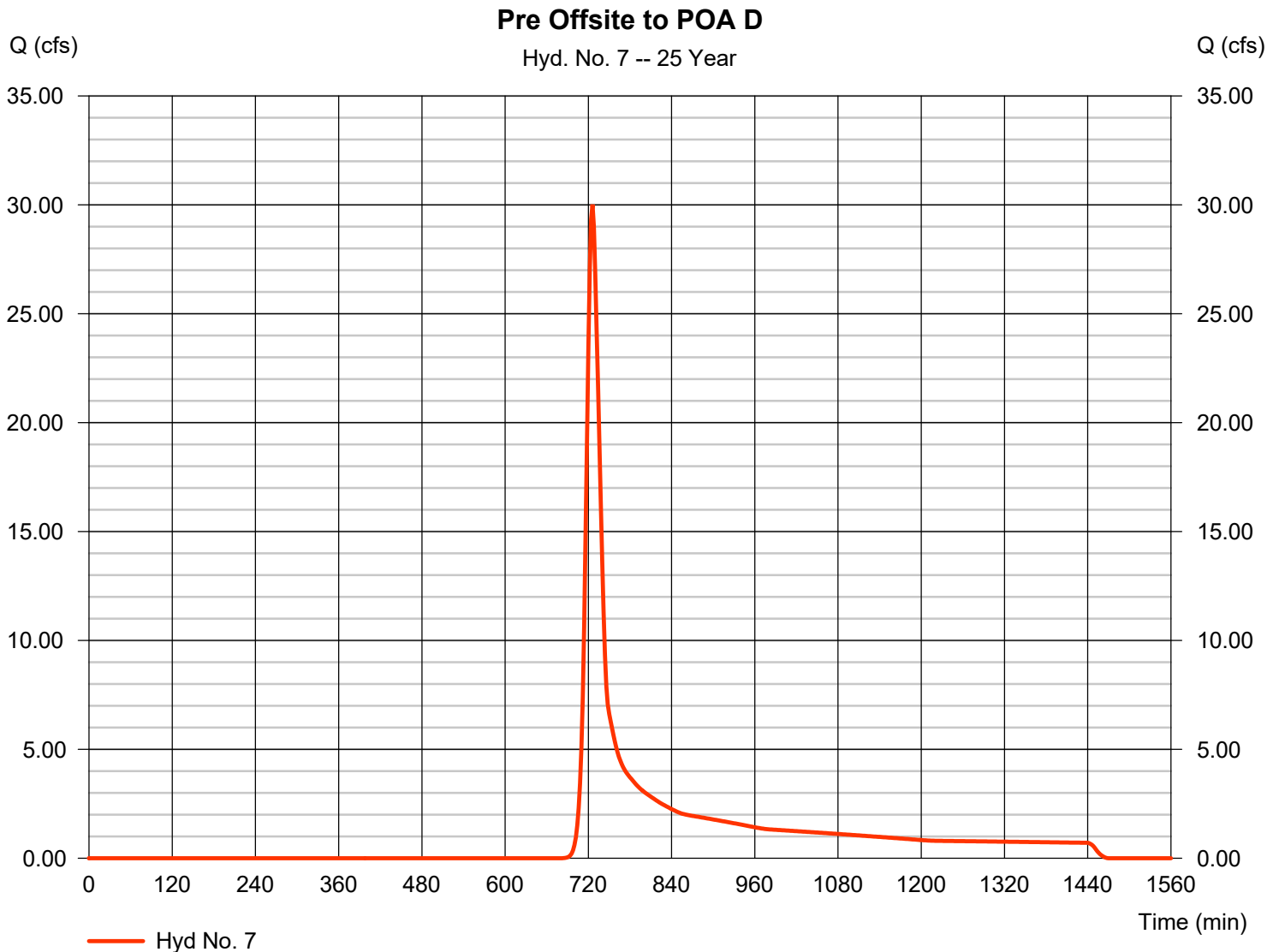
Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type = SCS Runoff
 Storm frequency = 25 yrs
 Time interval = 2 min
 Drainage area = 17.290 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 6.22 in
 Storm duration = 24 hrs

Peak discharge = 30.02 cfs
 Time to peak = 726 min
 Hyd. volume = 103,296 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 17.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

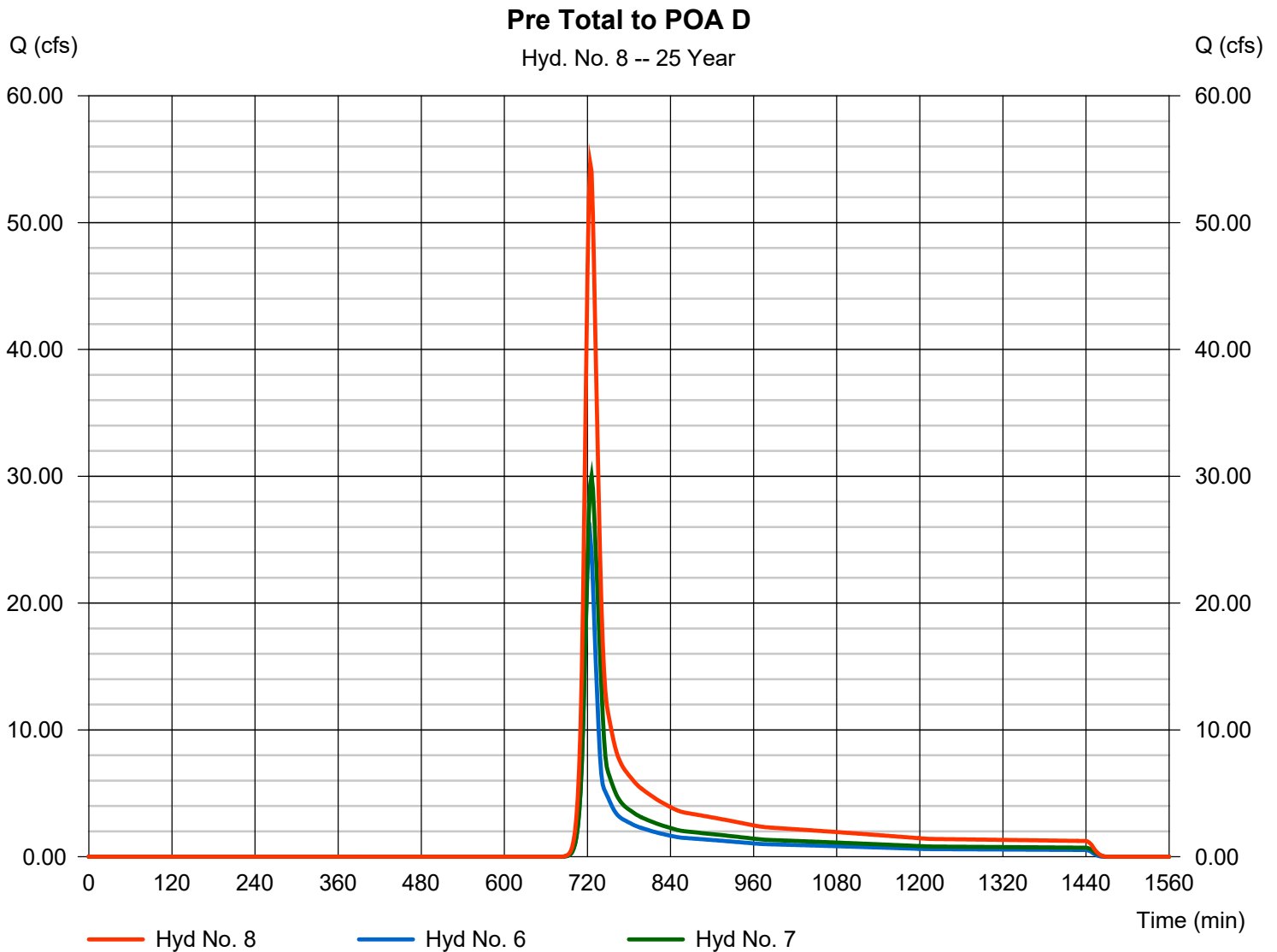
Wednesday, 06 / 3 / 2026

Hyd. No. 8

Pre Total to POA D

Hydrograph type = Combine
Storm frequency = 25 yrs
Time interval = 2 min
Inflow hyds. = 6, 7

Peak discharge = 54.68 cfs
Time to peak = 724 min
Hyd. volume = 180,302 cuft
Contrib. drain. area = 30.510 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

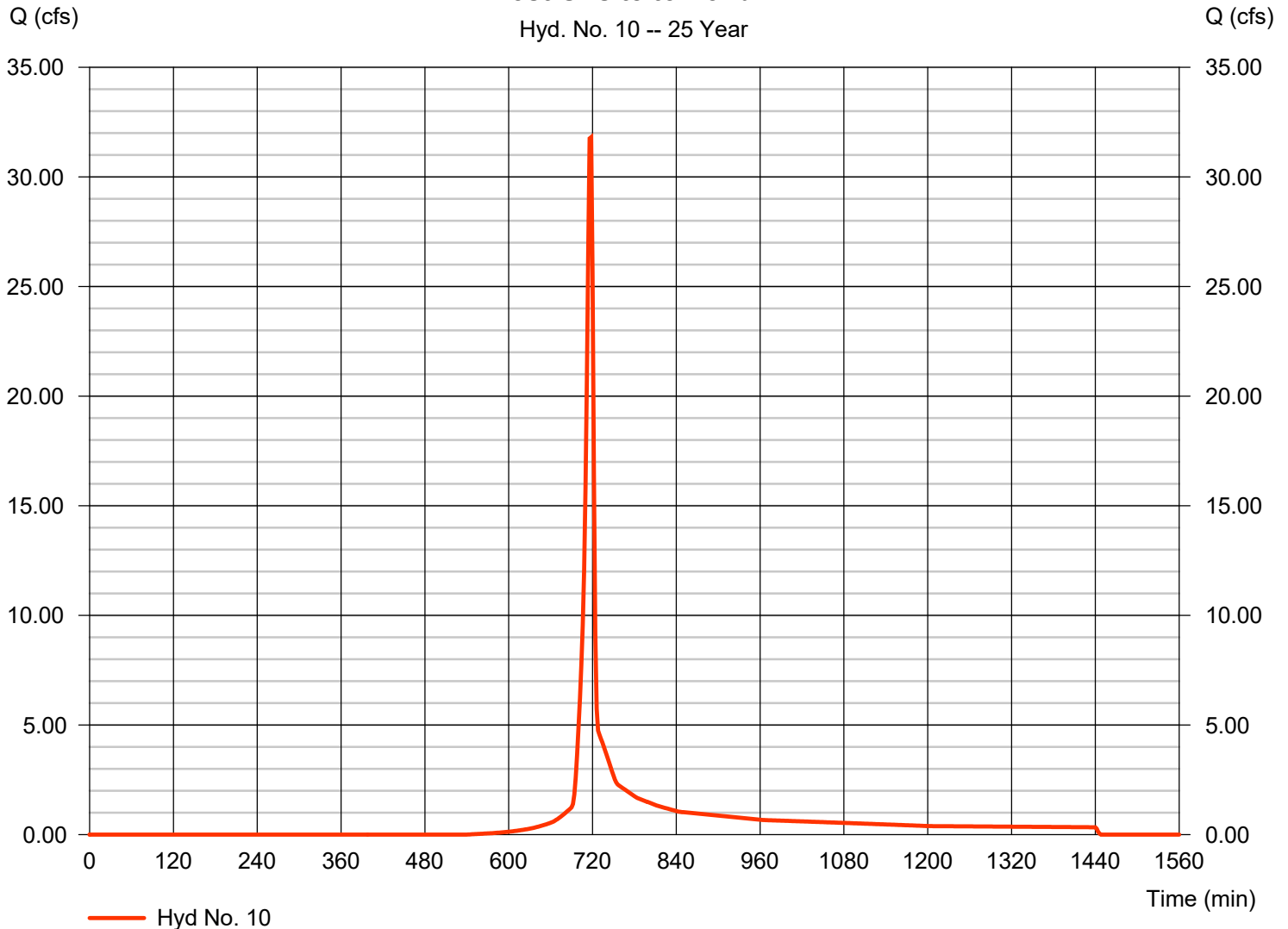
Wednesday, 06 / 3 / 2026

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	= SCS Runoff	Peak discharge	= 31.81 cfs
Storm frequency	= 25 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 64,120 cuft
Drainage area	= 6.530 ac	Curve number	= 69
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

Post Onsite to Pond 2



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

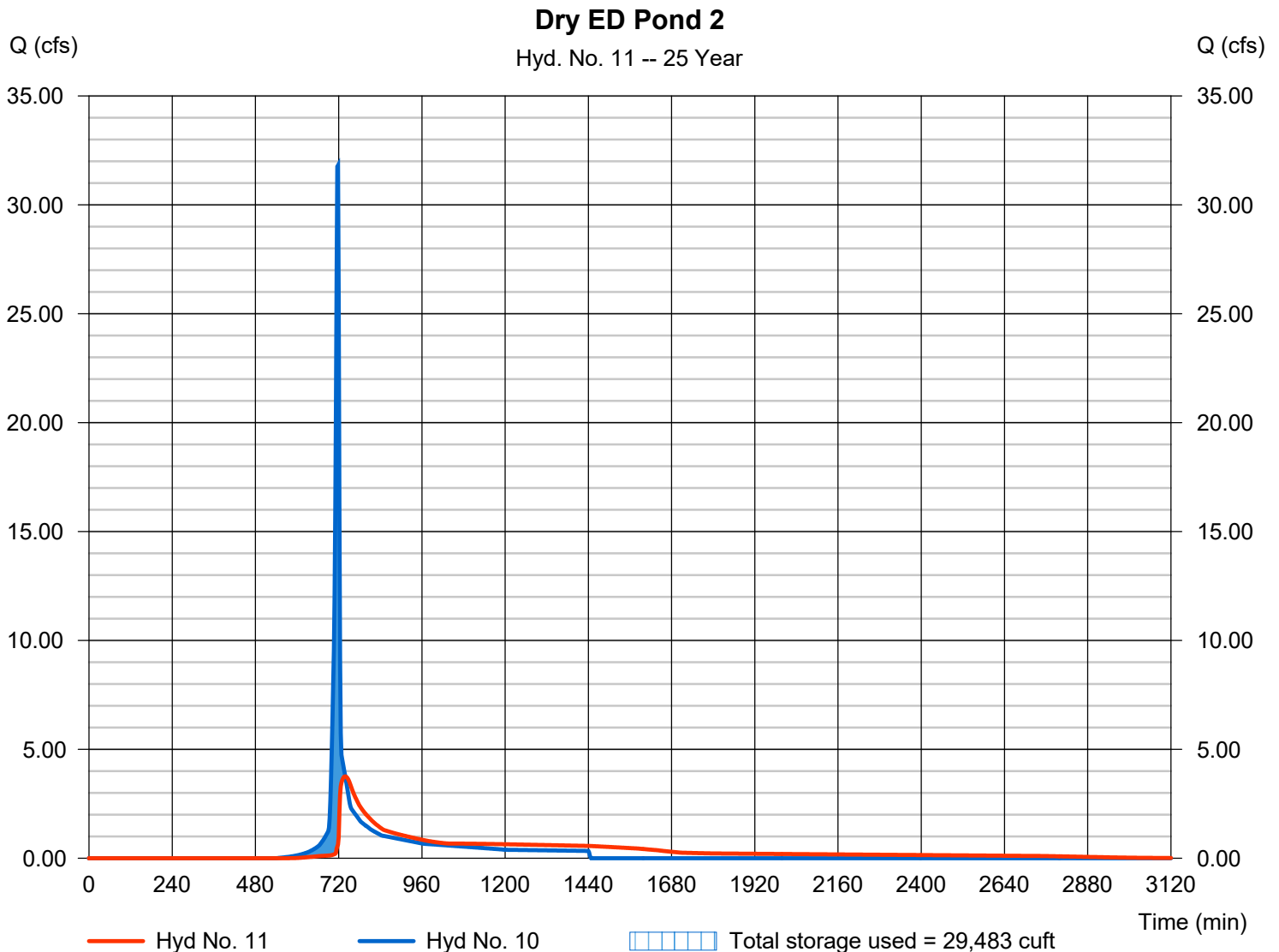
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 3.757 cfs
Storm frequency	= 25 yrs	Time to peak	= 738 min
Time interval	= 2 min	Hyd. volume	= 64,094 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 697.58 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 29,483 cuft

Storage Indication method used.



Hydrograph Report

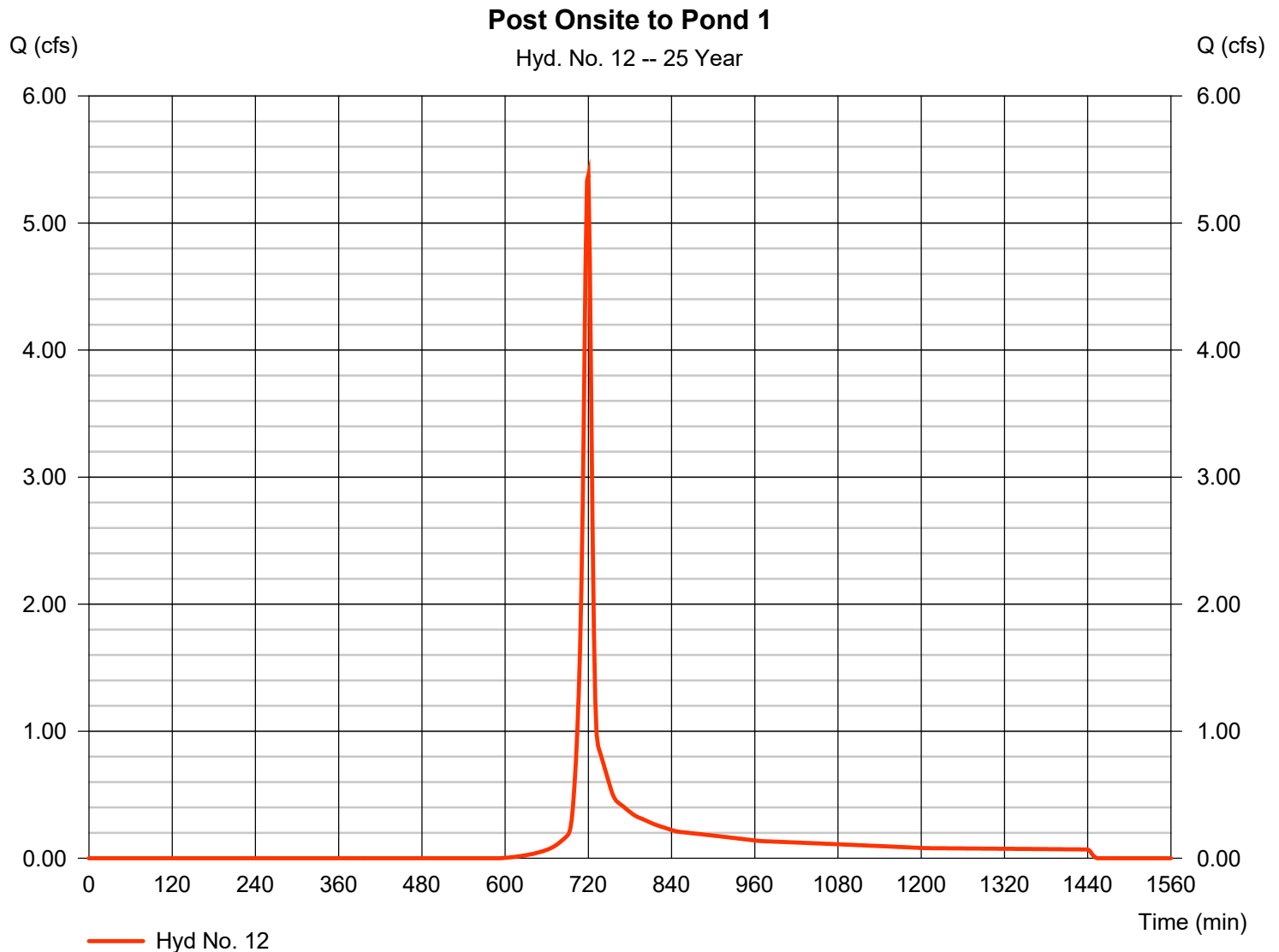
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 5.378 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 12,313 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

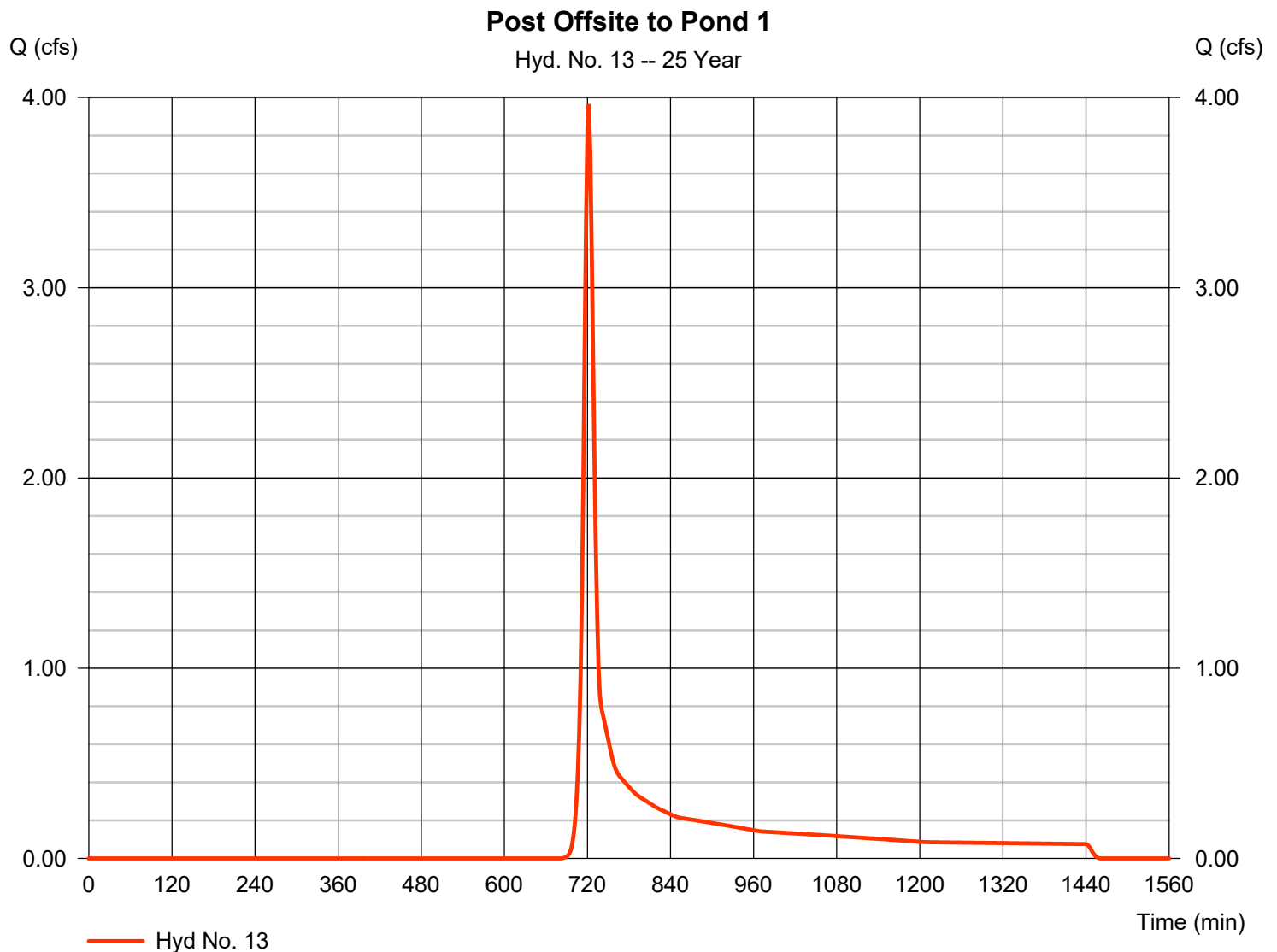
Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type = SCS Runoff
Storm frequency = 25 yrs
Time interval = 2 min
Drainage area = 1.780 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 6.22 in
Storm duration = 24 hrs

Peak discharge = 3.965 cfs
Time to peak = 722 min
Hyd. volume = 10,967 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 12.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

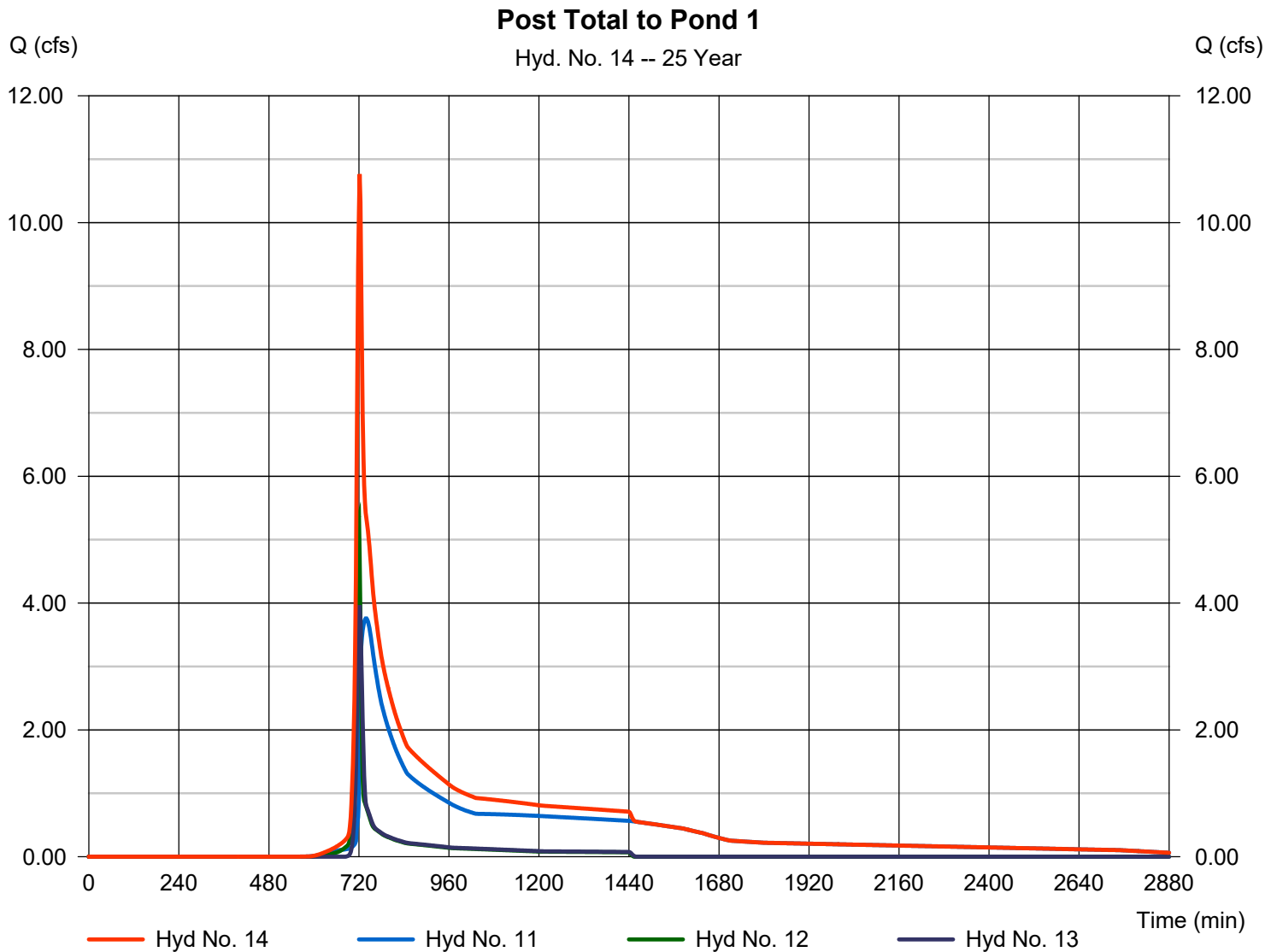
Wednesday, 06 / 3 / 2026

Hyd. No. 14

Post Total to Pond 1

Hydrograph type = Combine
Storm frequency = 25 yrs
Time interval = 2 min
Inflow hyds. = 11, 12, 13

Peak discharge = 10.76 cfs
Time to peak = 722 min
Hyd. volume = 87,374 cuft
Contrib. drain. area = 3.130 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

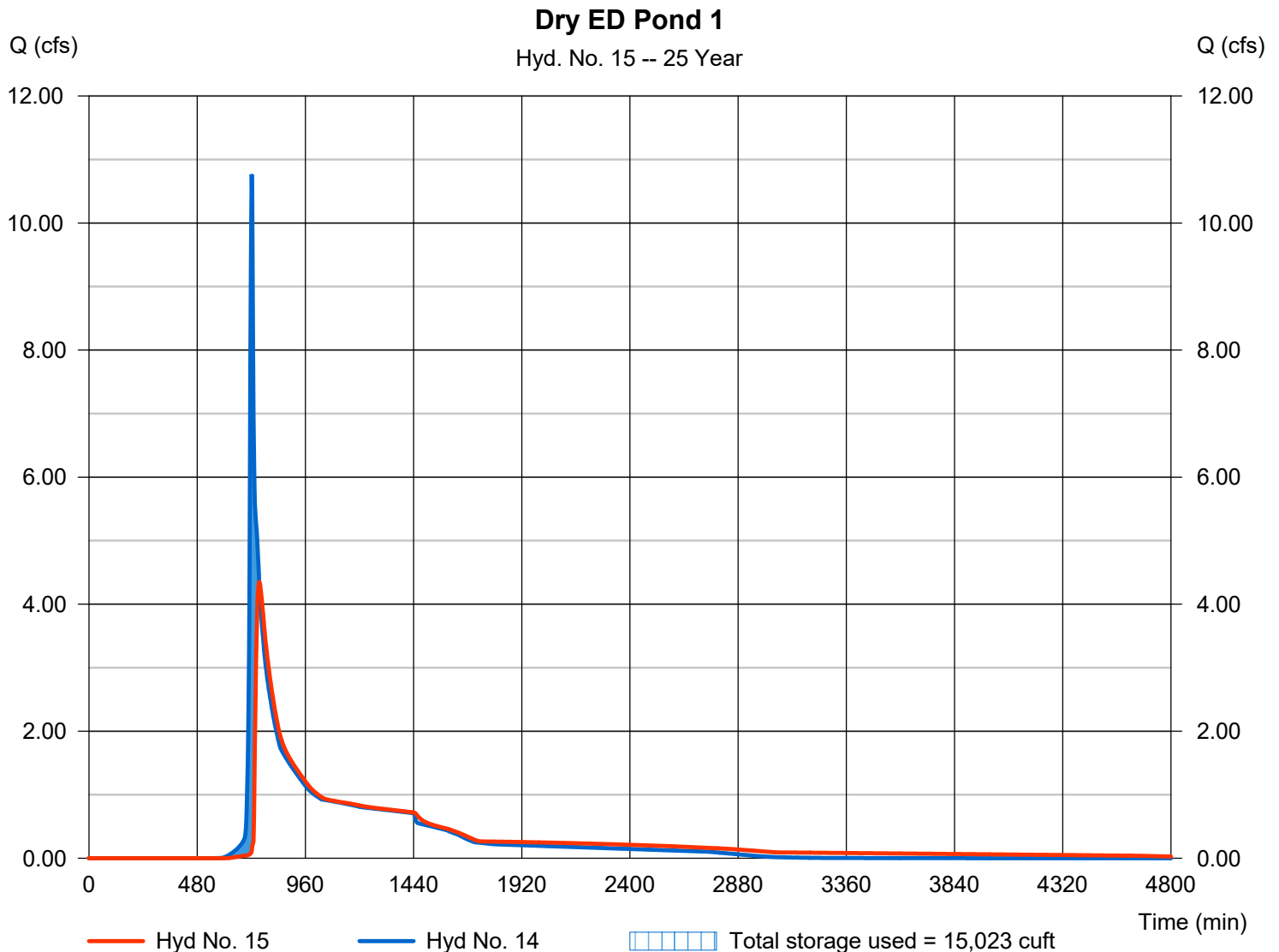
Wednesday, 06 / 3 / 2026

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 4.344 cfs
Storm frequency	= 25 yrs	Time to peak	= 756 min
Time interval	= 2 min	Hyd. volume	= 87,358 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 685.71 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 15,023 cuft

Storage Indication method used.



Hydrograph Report

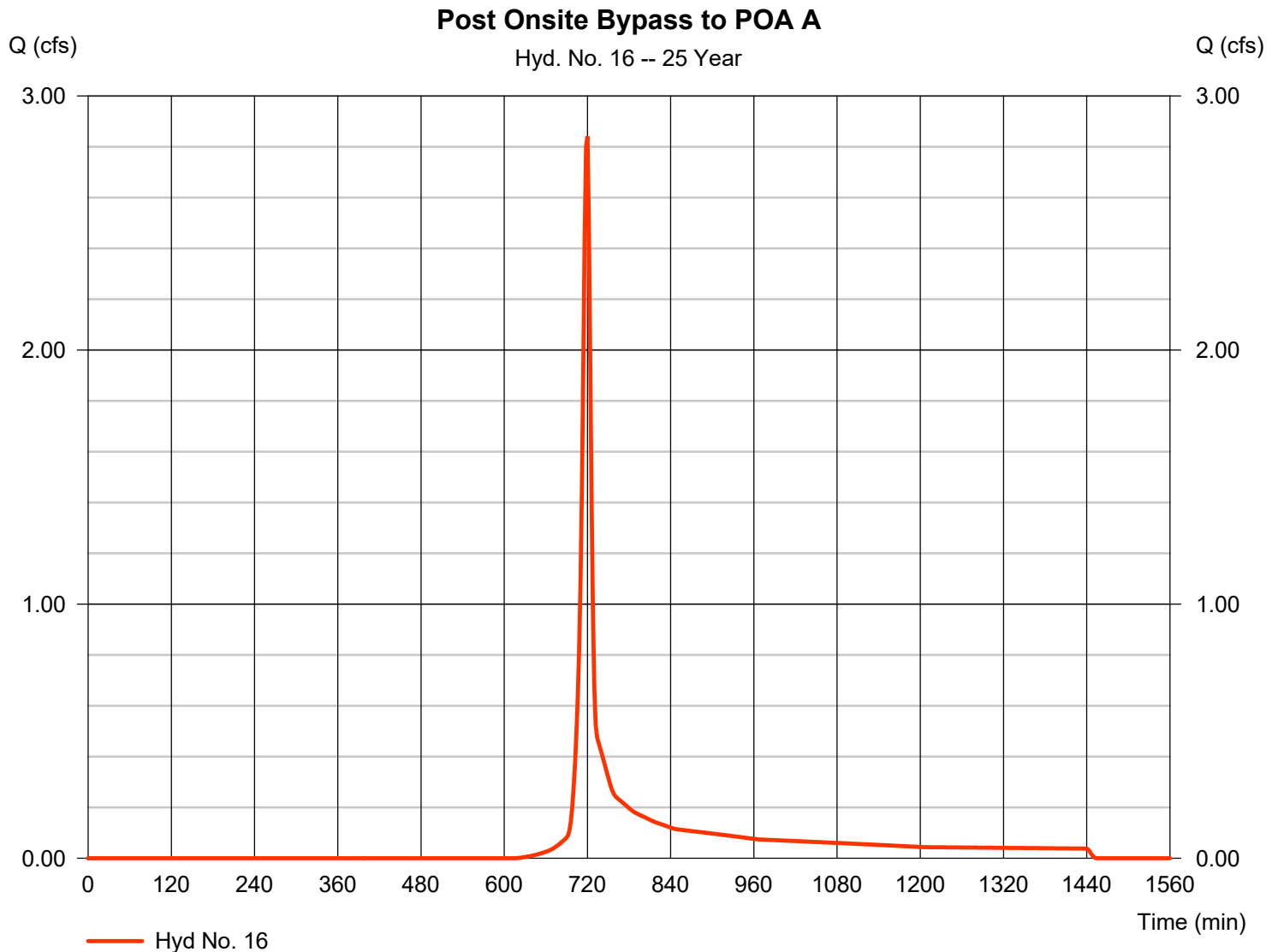
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.841 cfs
Storm frequency	=	25 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	6,517 cuft
Drainage area	=	0.770 ac	Curve number	=	63
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	6.22 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

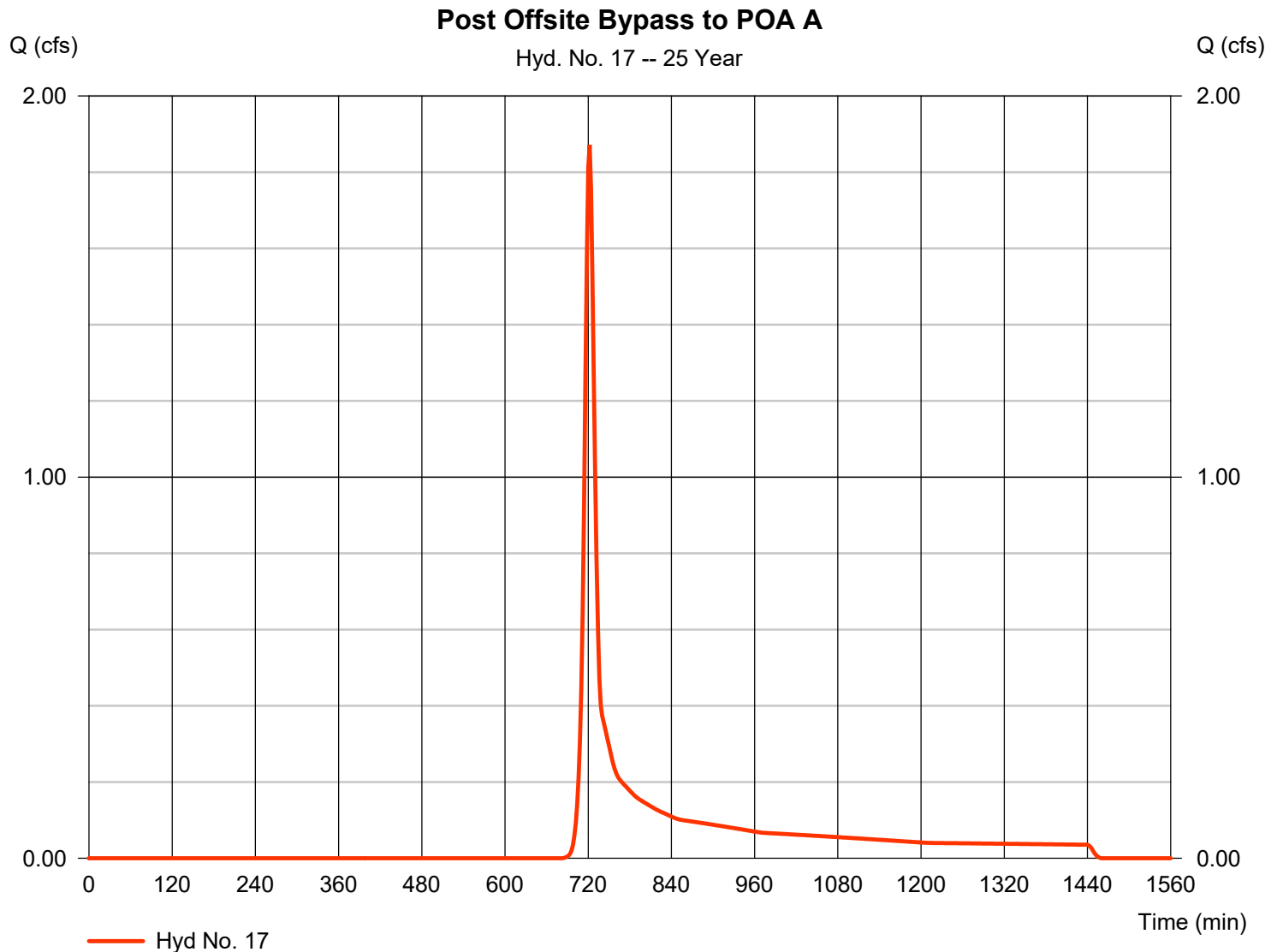
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.871 cfs
Storm frequency	=	25 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	5,175 cuft
Drainage area	=	0.840 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	6.22 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

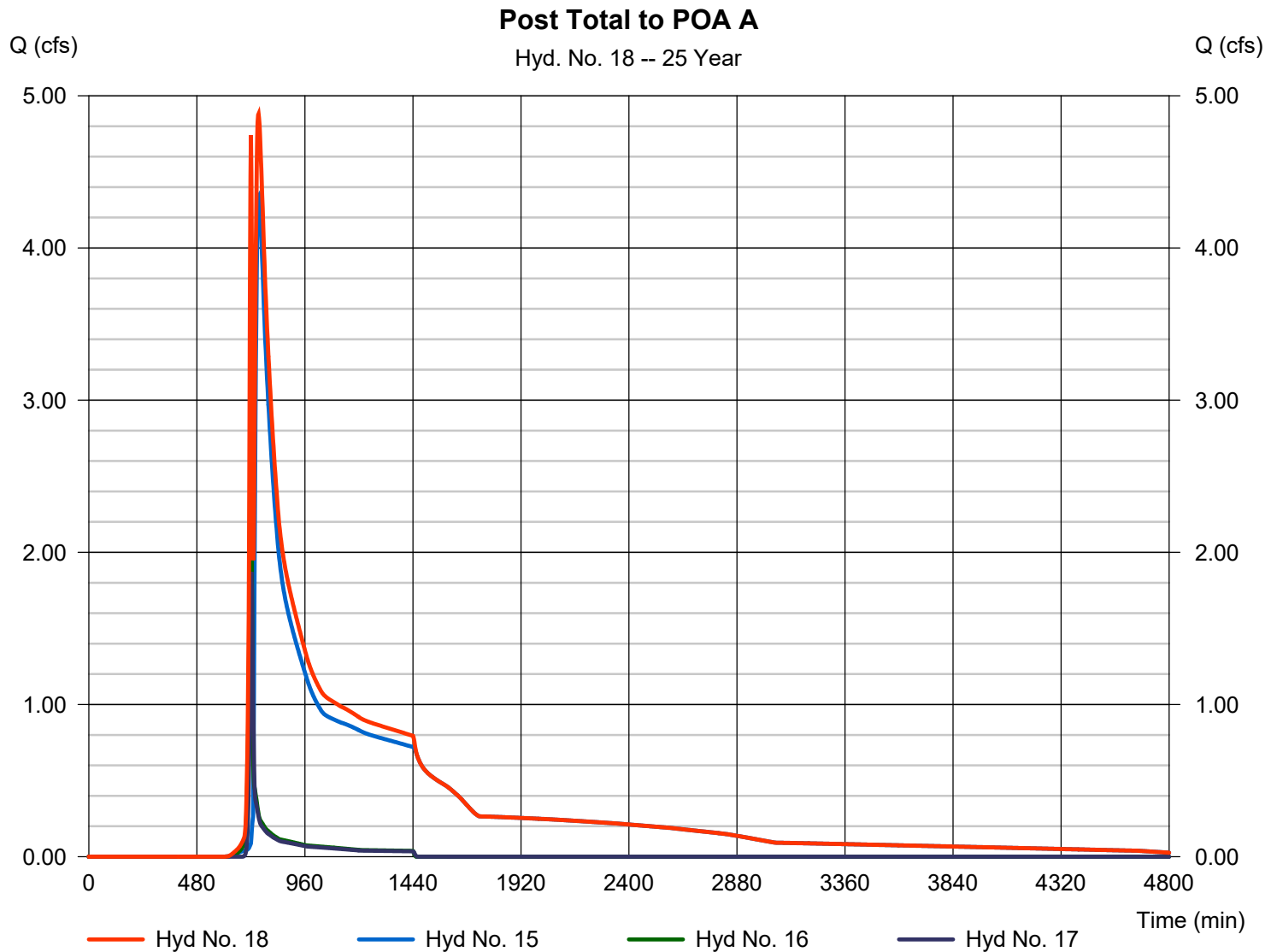
Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type = Combine
Storm frequency = 25 yrs
Time interval = 2 min
Inflow hyds. = 15, 16, 17

Peak discharge = 4.881 cfs
Time to peak = 754 min
Hyd. volume = 99,050 cuft
Contrib. drain. area = 1.610 ac



Hydrograph Report

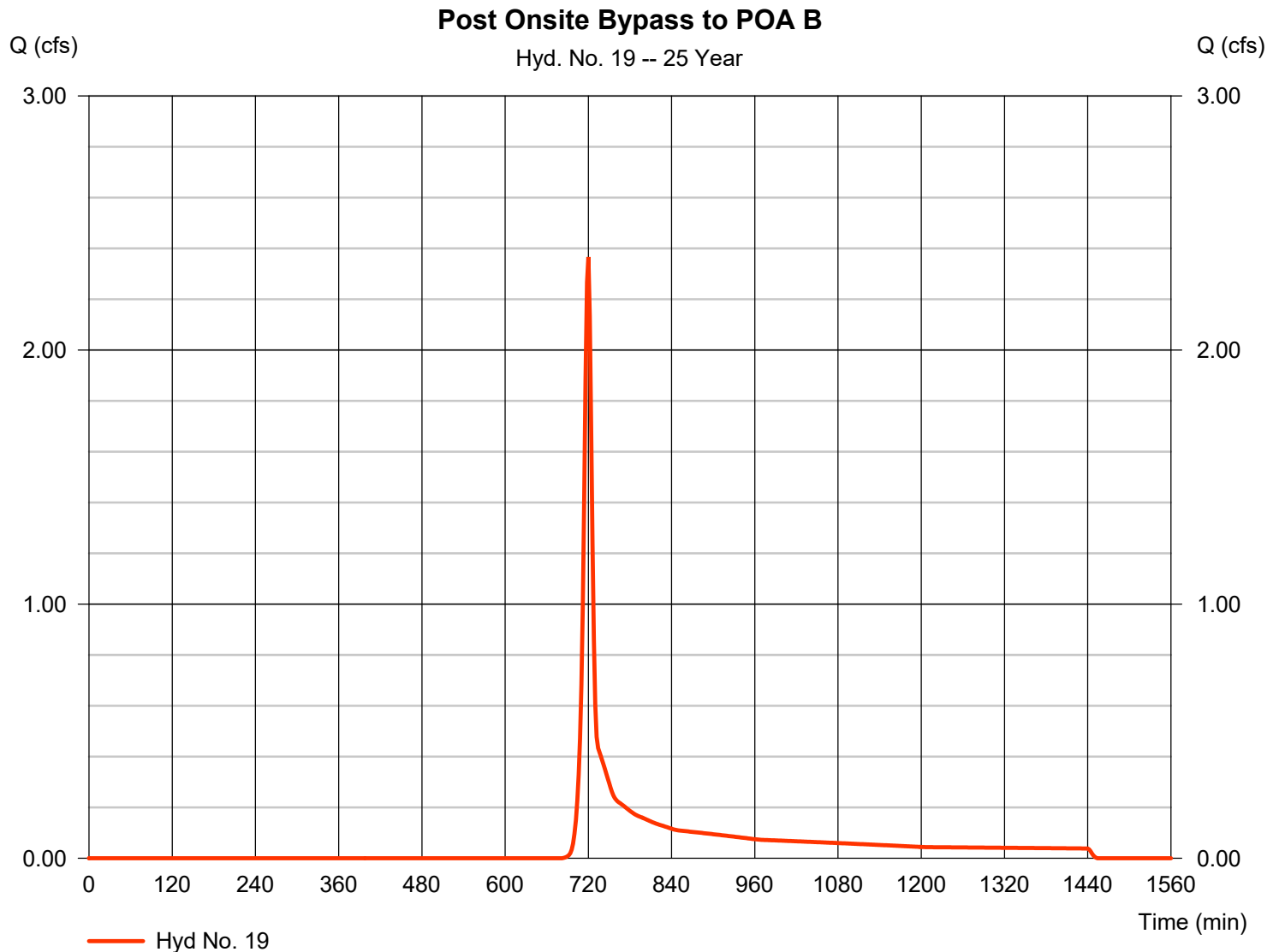
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.367 cfs
Storm frequency	=	25 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	5,616 cuft
Drainage area	=	0.940 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	8.00 min
Total precip.	=	6.22 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

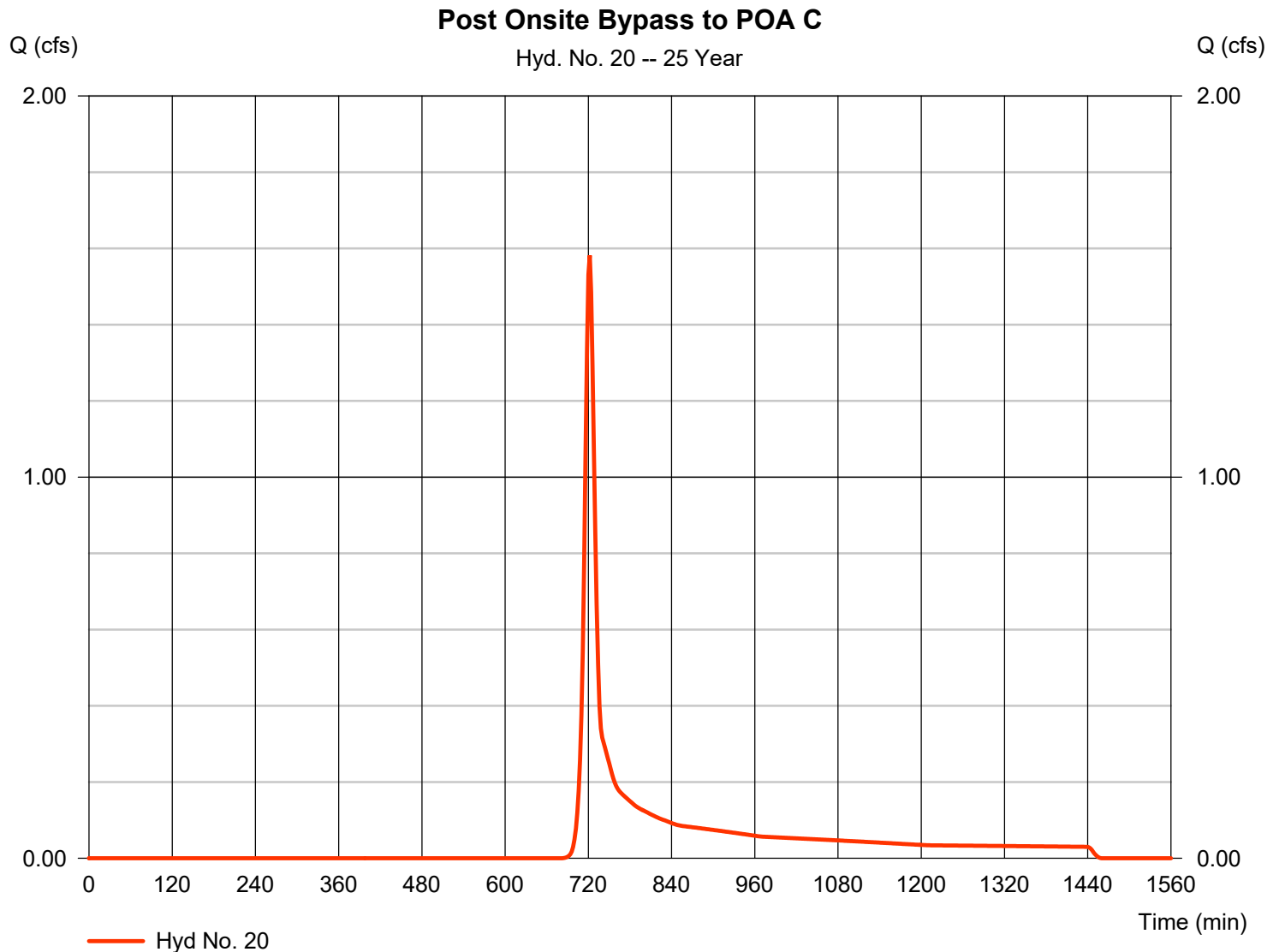
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 1.582 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 4,374 cuft
Drainage area	= 0.710 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

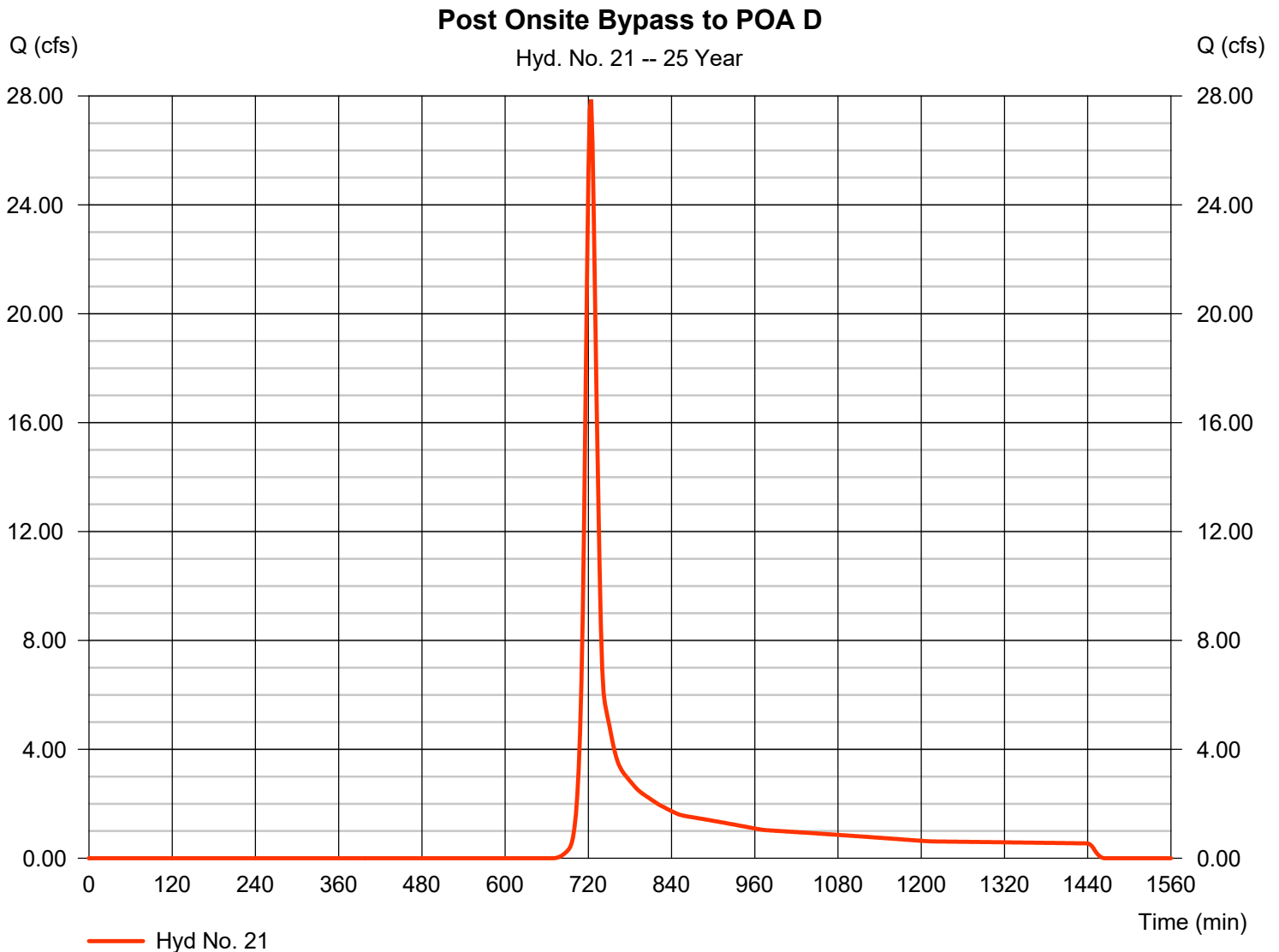
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type	= SCS Runoff	Peak discharge	= 27.86 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 83,010 cuft
Drainage area	= 12.950 ac	Curve number	= 57
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 15.00 min
Total precip.	= 6.22 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

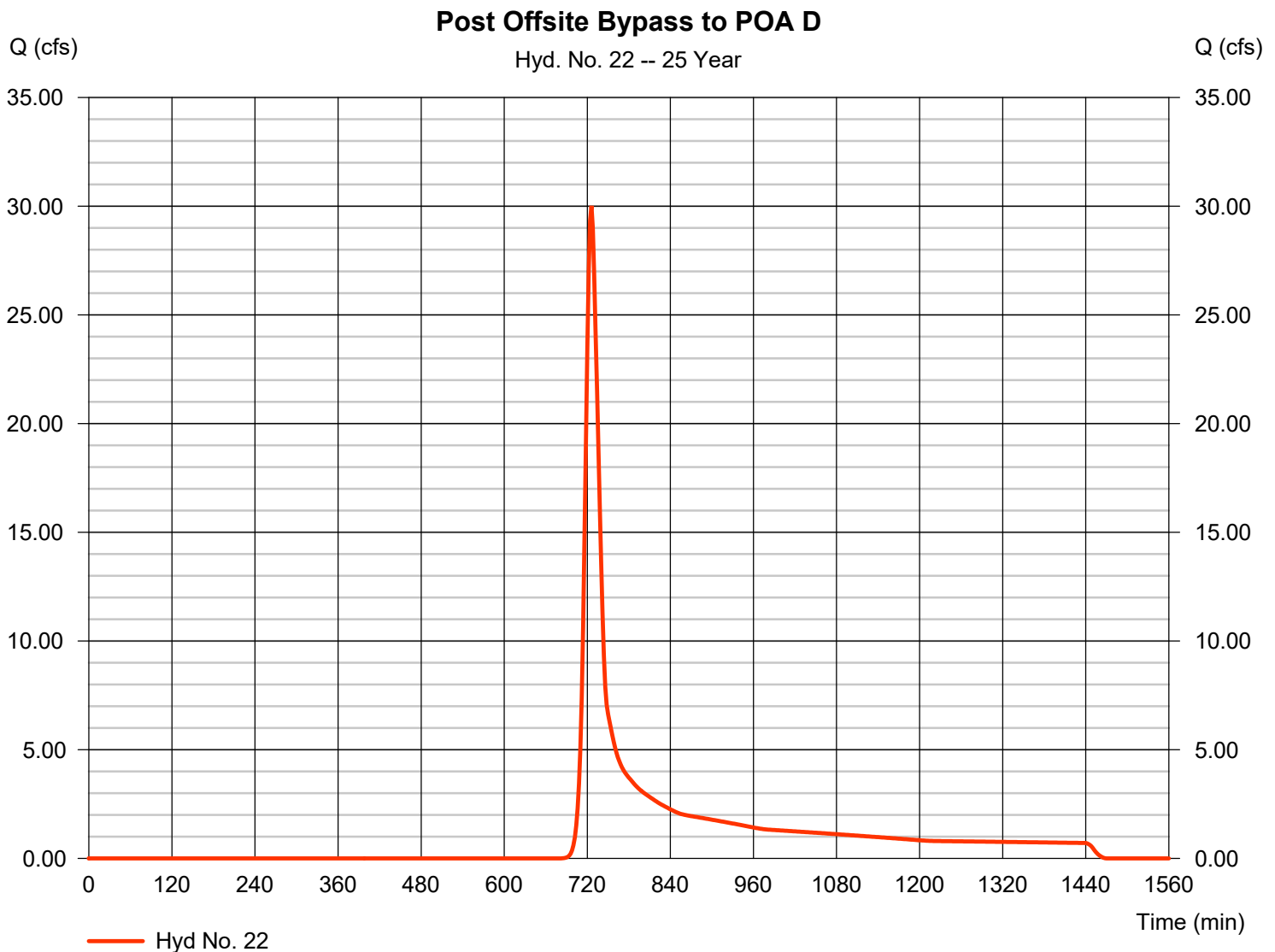
Wednesday, 06 / 3 / 2026

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type = SCS Runoff
 Storm frequency = 25 yrs
 Time interval = 2 min
 Drainage area = 17.290 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 6.22 in
 Storm duration = 24 hrs

Peak discharge = 30.02 cfs
 Time to peak = 726 min
 Hyd. volume = 103,296 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 17.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

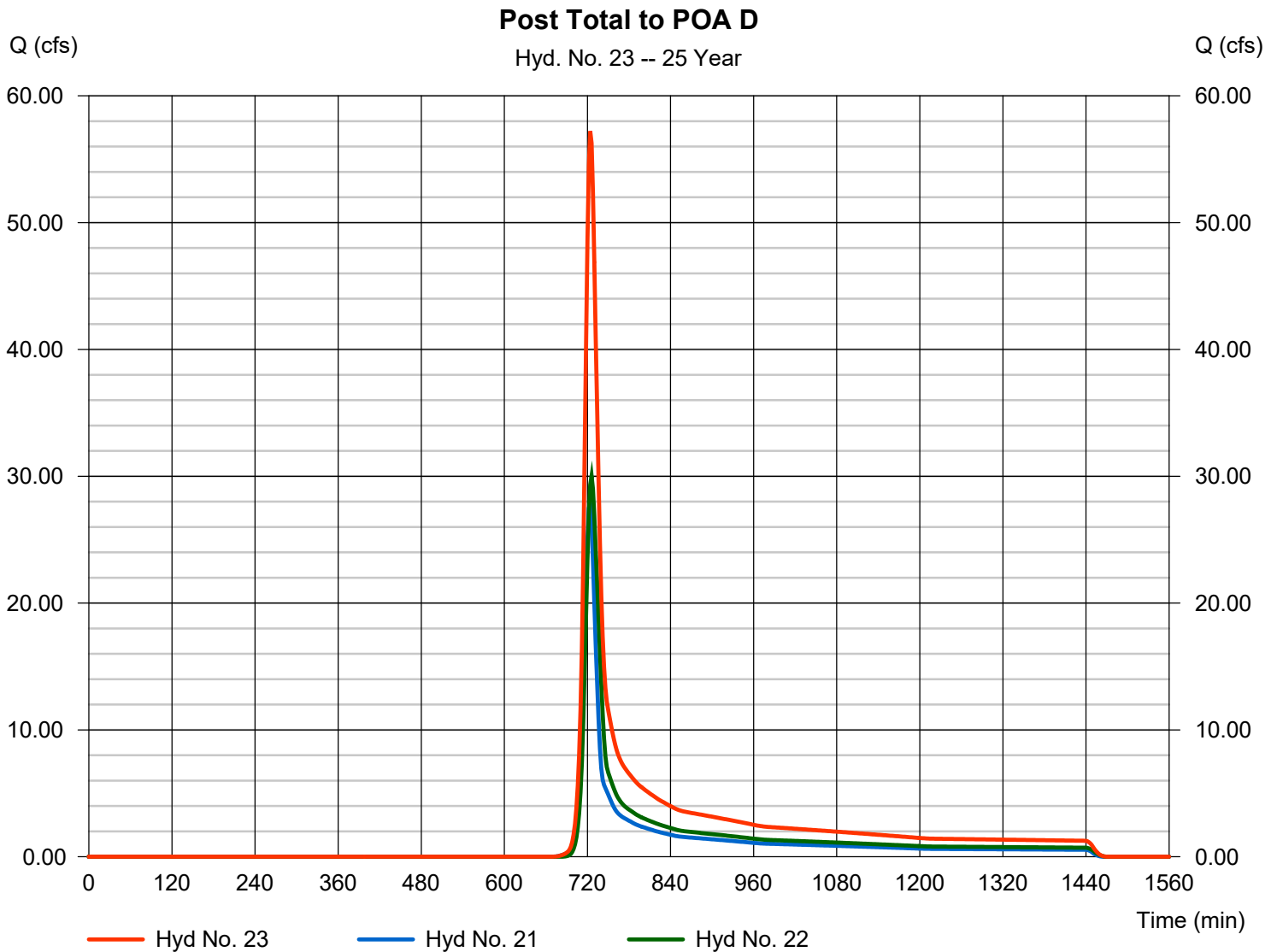
Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type = Combine
Storm frequency = 25 yrs
Time interval = 2 min
Inflow hyds. = 21, 22

Peak discharge = 57.21 cfs
Time to peak = 724 min
Hyd. volume = 186,307 cuft
Contrib. drain. area = 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.873	2	720	16,007	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	7.776	2	722	20,972	-----	-----	-----	Pre Offsite to POA A
3	Combine	14.48	2	720	36,979	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	19.51	2	722	52,630	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	4.021	2	722	10,845	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	33.71	2	724	99,670	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	40.14	2	726	133,698	-----	-----	-----	Pre Offsite to POA D
8	Combine	73.29	2	724	233,368	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	38.75	2	716	78,271	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	8.251	2	726	78,245	10	698.07	34,368	Dry ED Pond 2
12	SCS Runoff	6.657	2	720	15,242	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	5.263	2	722	14,194	-----	-----	-----	Post Offsite to Pond 1
14	Combine	18.50	2	722	107,681	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	8.679	2	738	107,666	14	686.16	17,515	Dry ED Pond 1
16	SCS Runoff	3.552	2	720	8,129	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	2.484	2	722	6,698	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	9.747	2	736	122,493	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	3.121	2	720	7,269	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	2.099	2	722	5,662	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	36.42	2	724	106,330	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	40.14	2	726	133,698	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	76.00	2	724	240,027	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 50 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

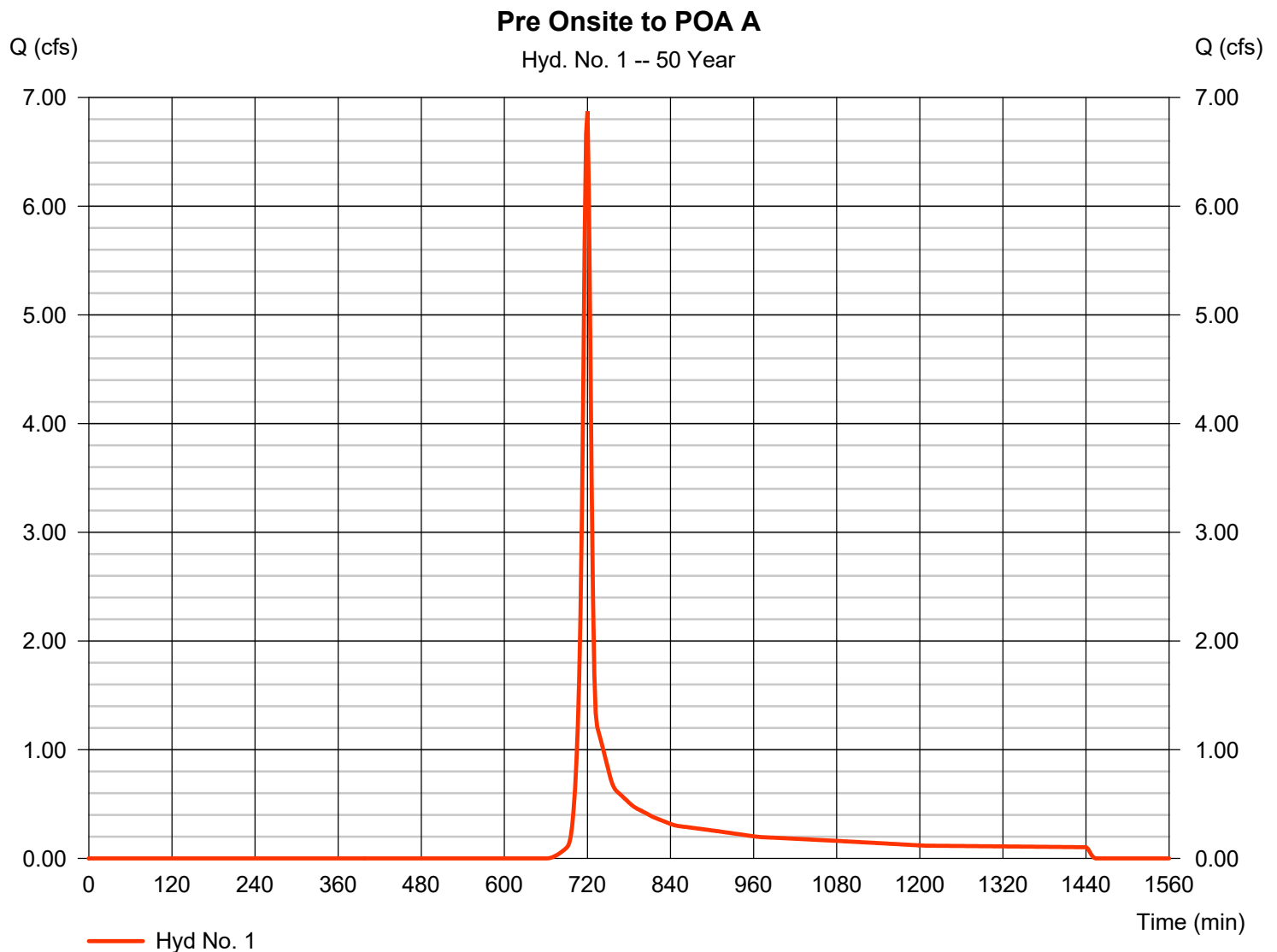
Wednesday, 06 / 3 / 2026

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 2.070 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 6.873 cfs
Time to peak = 720 min
Hyd. volume = 16,007 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 9.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

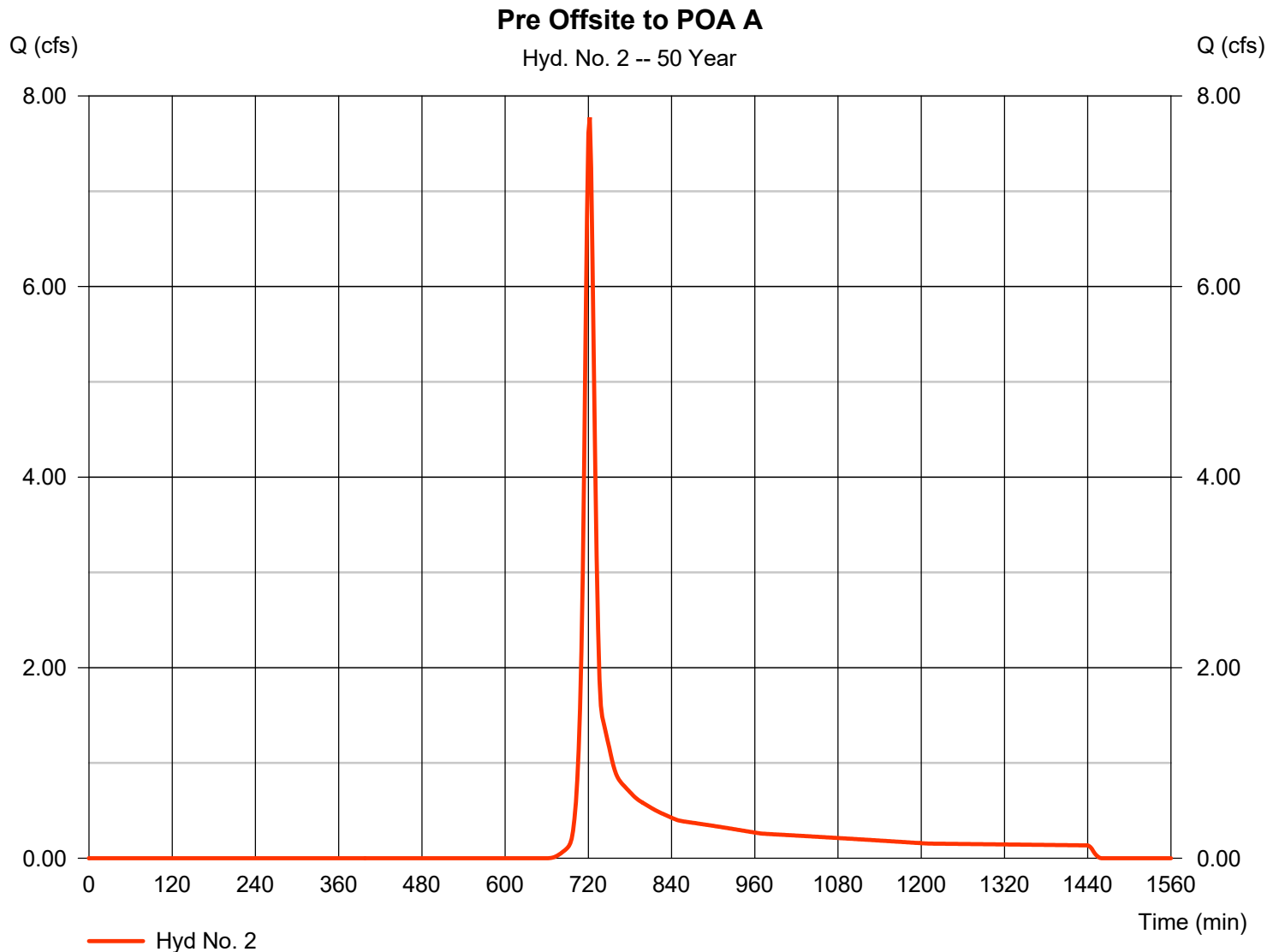
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 7.776 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 20,972 cuft
Drainage area	= 2.630 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

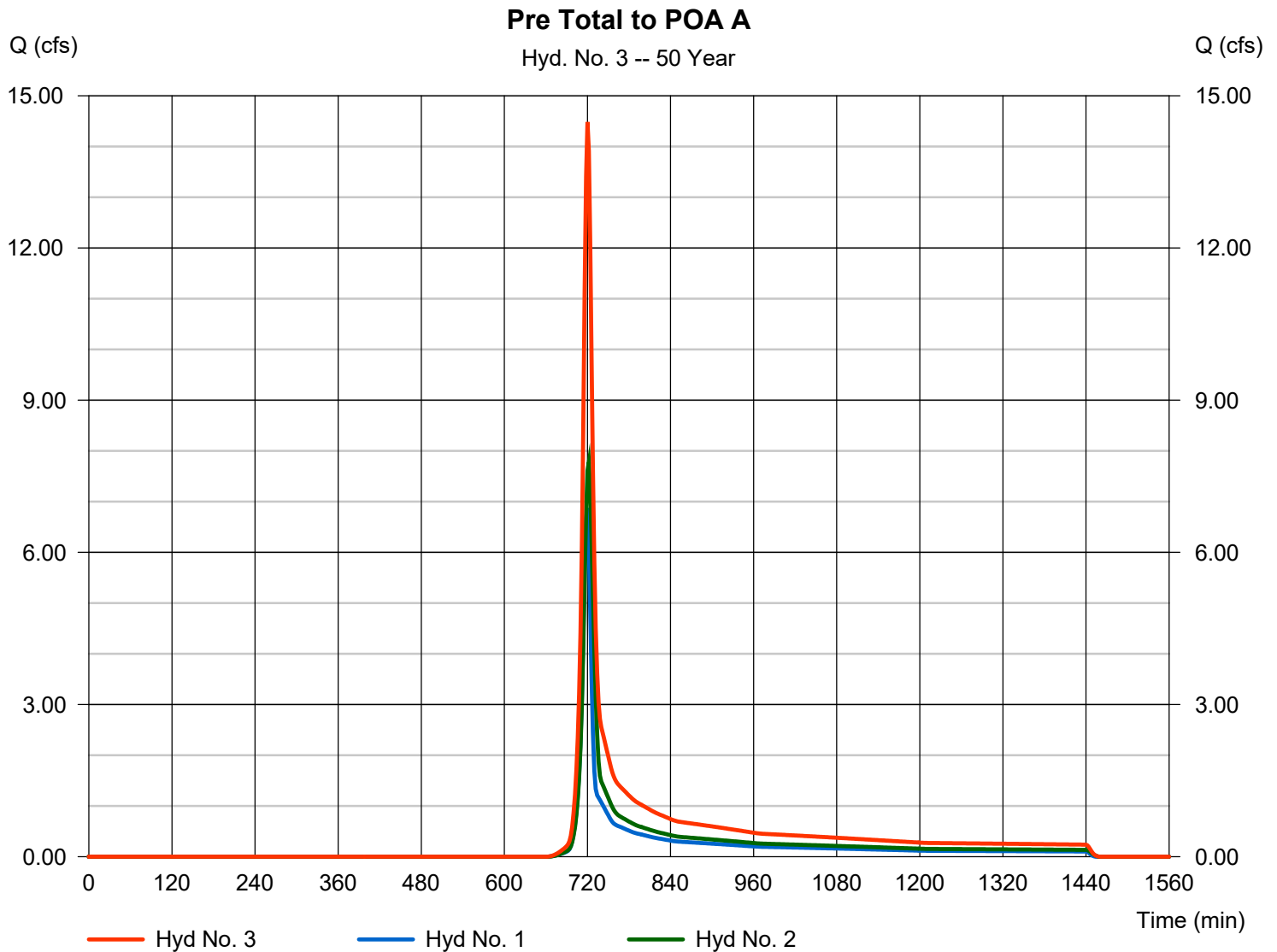
Wednesday, 06 / 3 / 2026

Hyd. No. 3

Pre Total to POA A

Hydrograph type = Combine
Storm frequency = 50 yrs
Time interval = 2 min
Inflow hyds. = 1, 2

Peak discharge = 14.48 cfs
Time to peak = 720 min
Hyd. volume = 36,979 cuft
Contrib. drain. area = 4.700 ac



Hydrograph Report

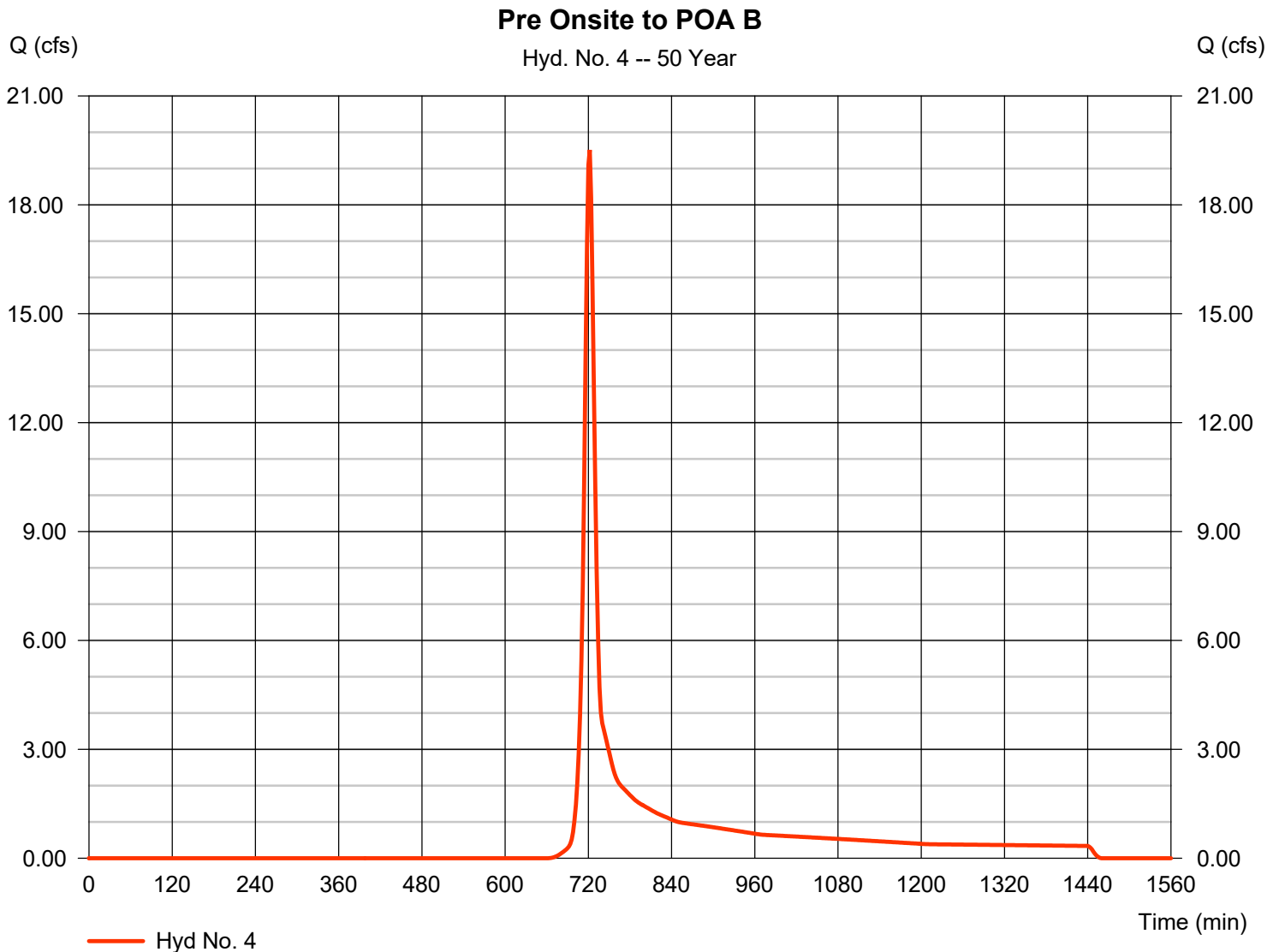
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 19.51 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 52,630 cuft
Drainage area	= 6.600 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

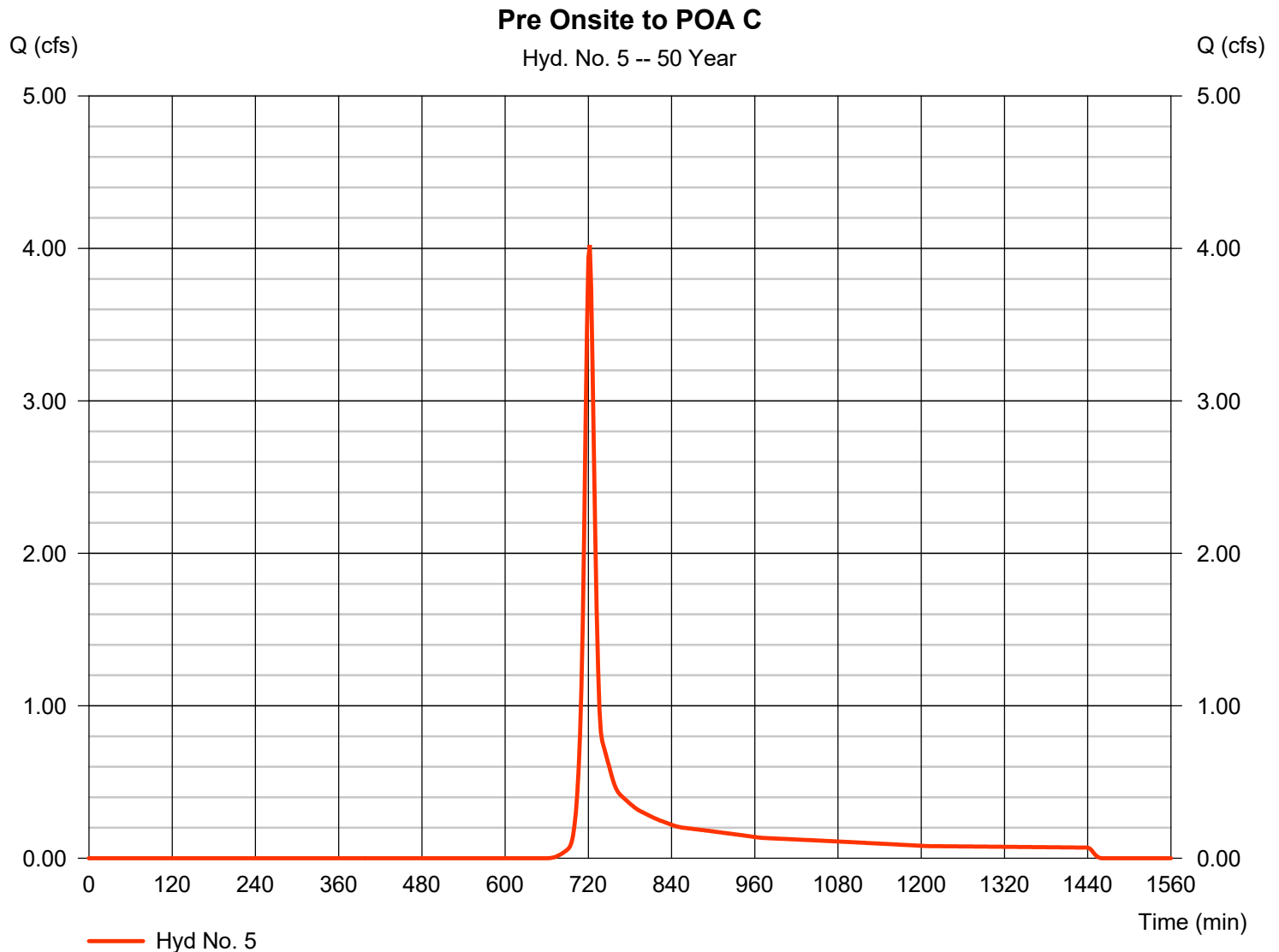
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 4.021 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 10,845 cuft
Drainage area	= 1.360 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

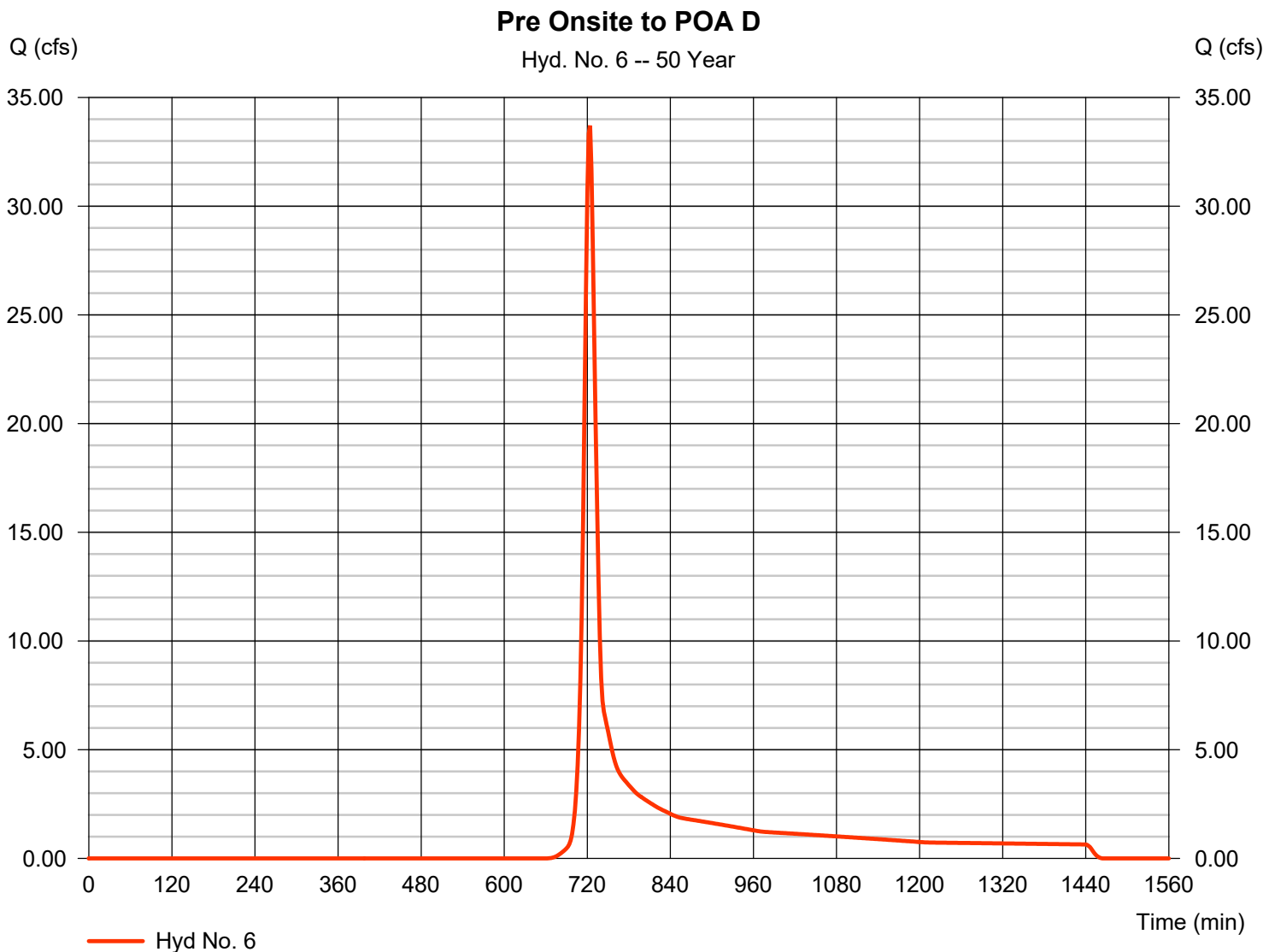
Wednesday, 06 / 3 / 2026

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 13.220 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 33.71 cfs
Time to peak = 724 min
Hyd. volume = 99,670 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 15.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

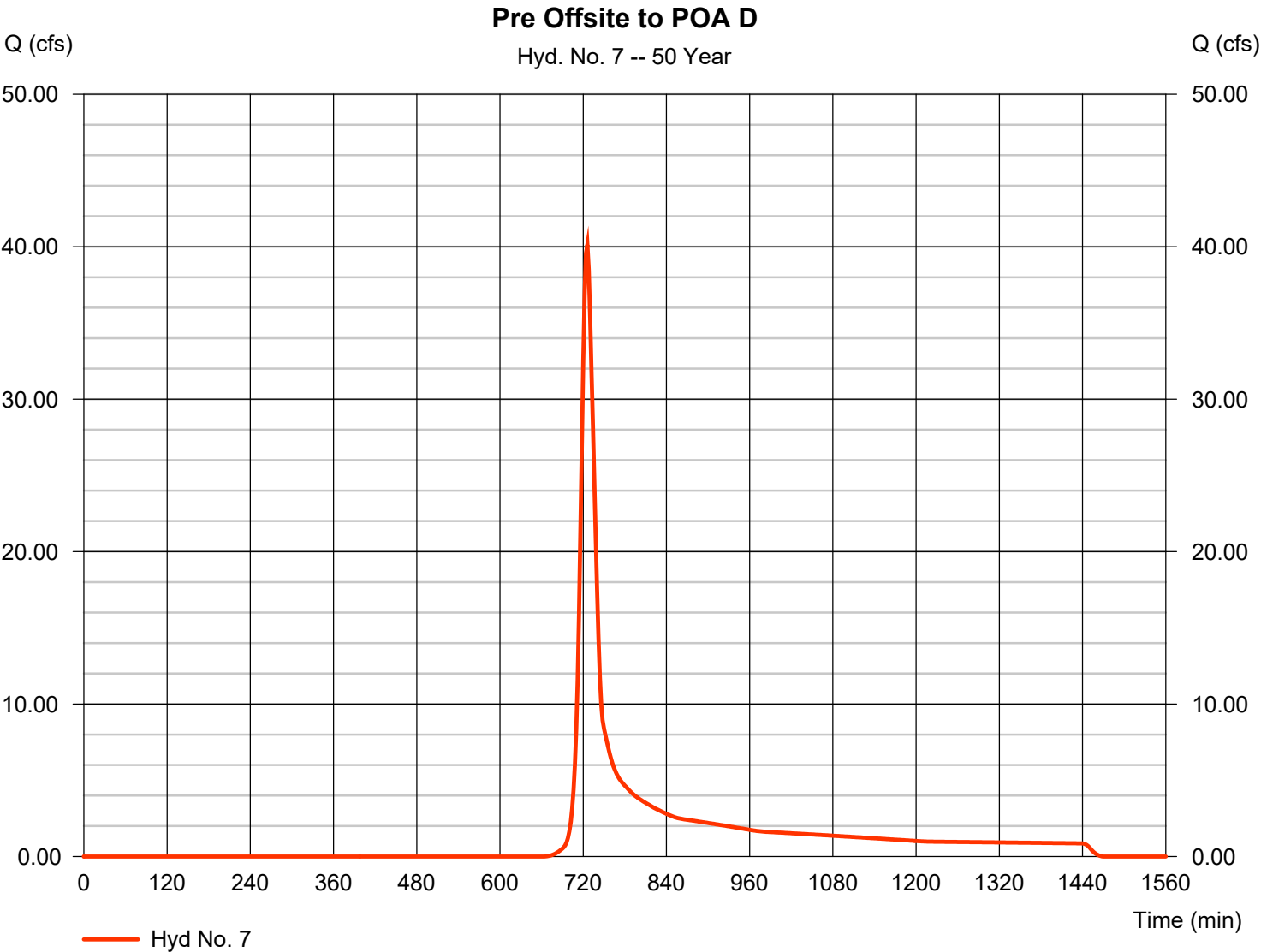
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type	=	SCS Runoff	Peak discharge	=	40.14 cfs
Storm frequency	=	50 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	133,698 cuft
Drainage area	=	17.290 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	17.00 min
Total precip.	=	7.01 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

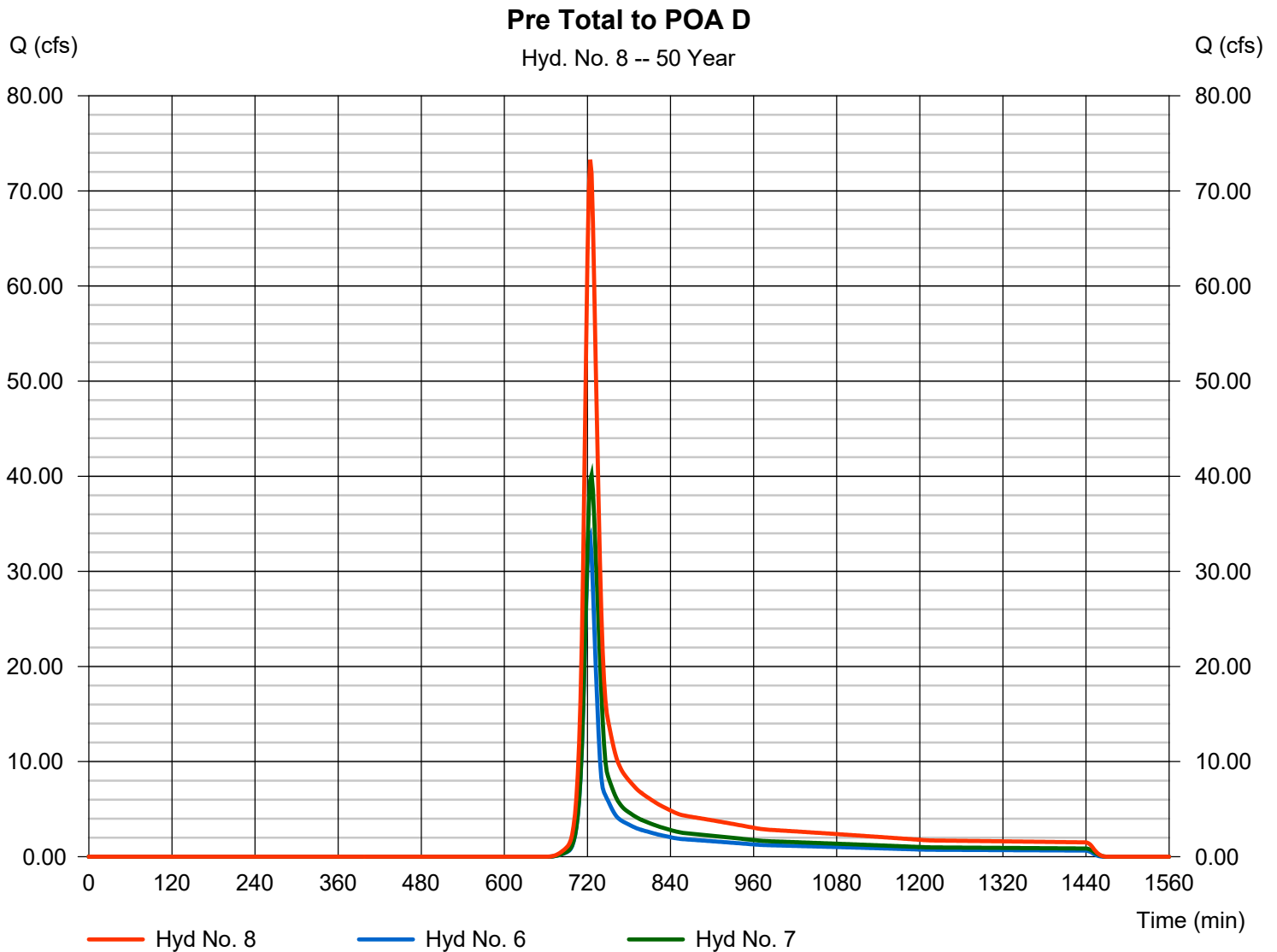
Wednesday, 06 / 3 / 2026

Hyd. No. 8

Pre Total to POA D

Hydrograph type = Combine
Storm frequency = 50 yrs
Time interval = 2 min
Inflow hyds. = 6, 7

Peak discharge = 73.29 cfs
Time to peak = 724 min
Hyd. volume = 233,368 cuft
Contrib. drain. area = 30.510 ac



Hydrograph Report

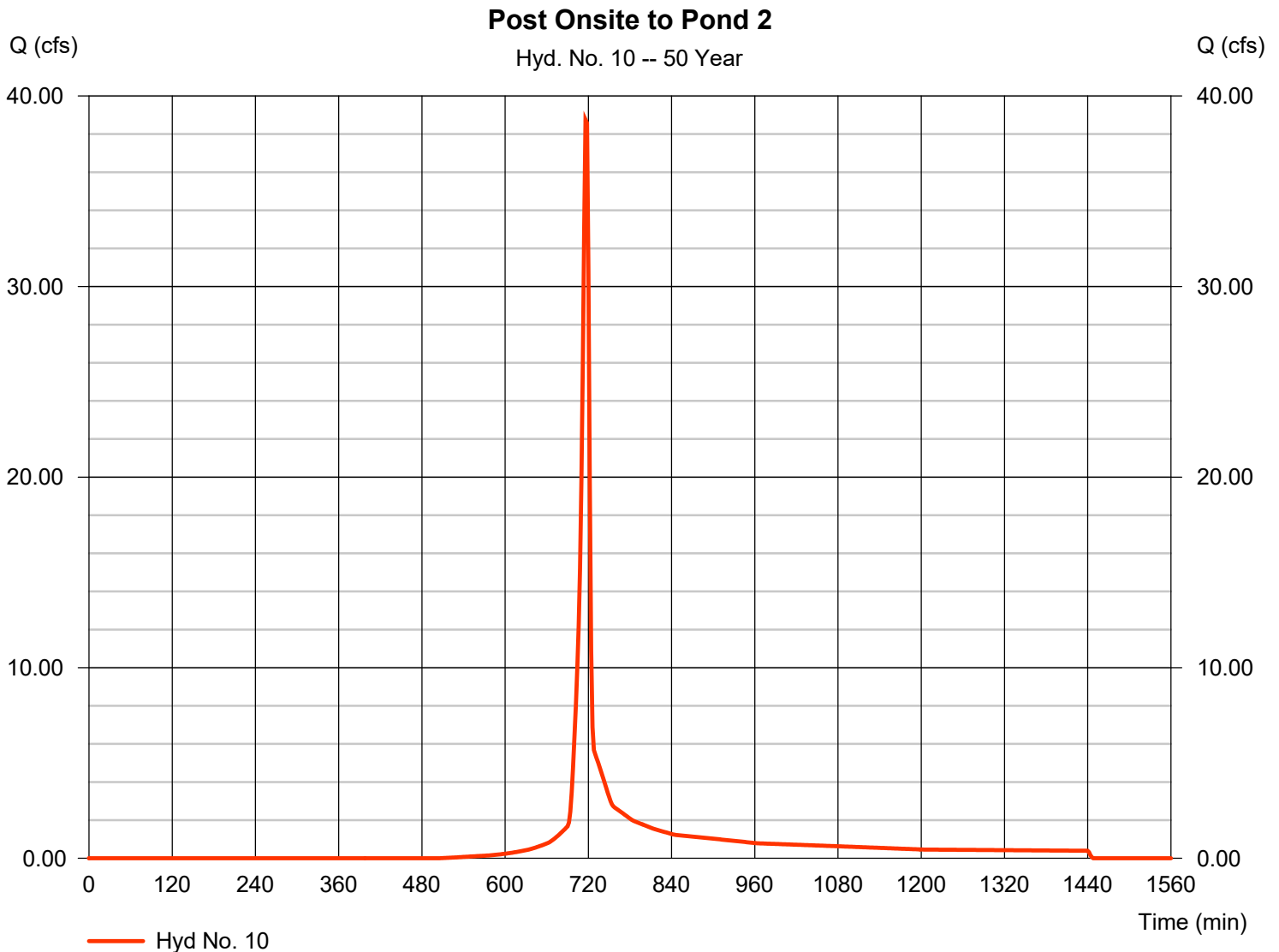
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	= SCS Runoff	Peak discharge	= 38.75 cfs
Storm frequency	= 50 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 78,271 cuft
Drainage area	= 6.530 ac	Curve number	= 69
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

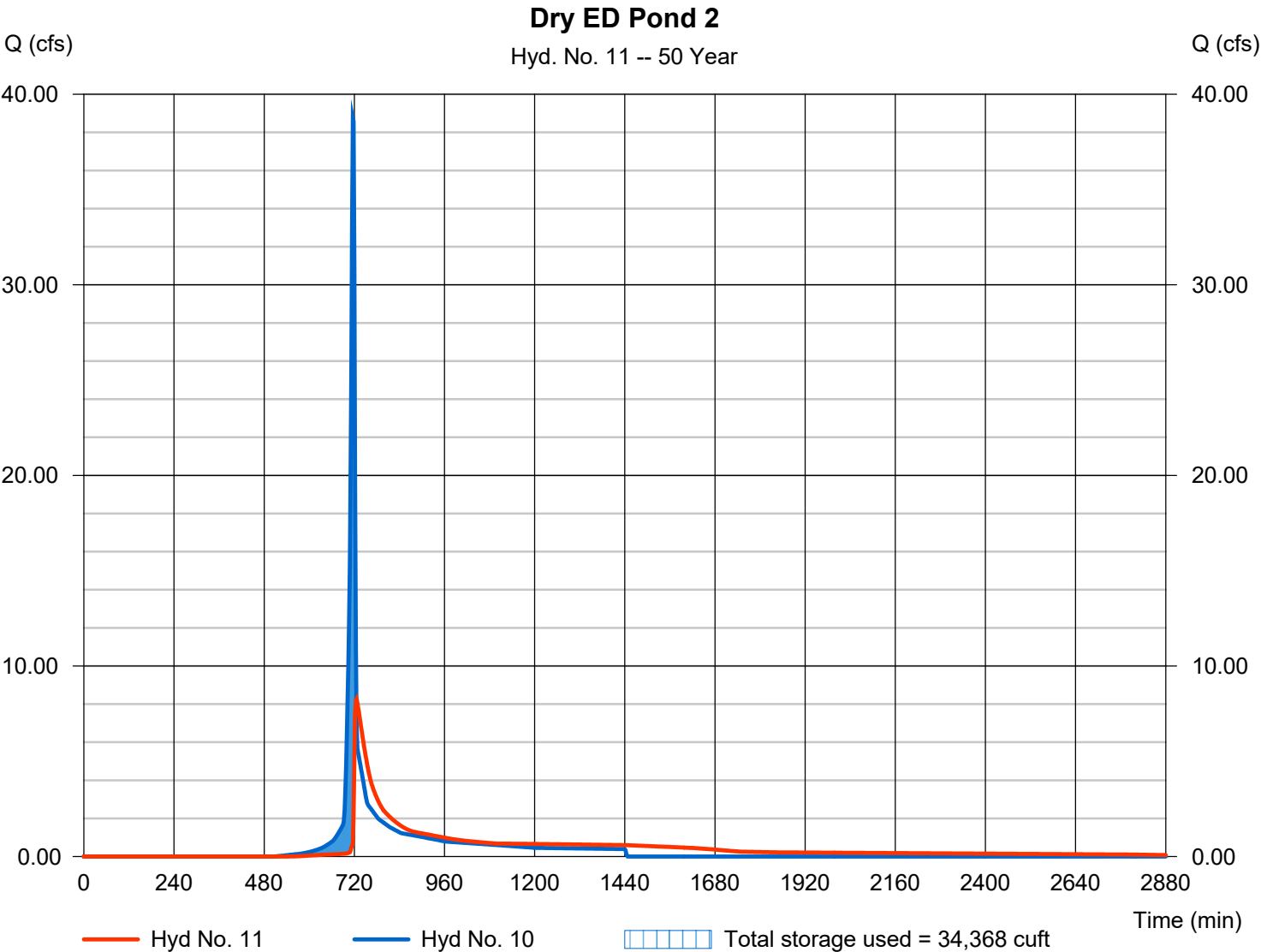
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 8.251 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 78,245 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 698.07 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 34,368 cuft

Storage Indication method used.



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

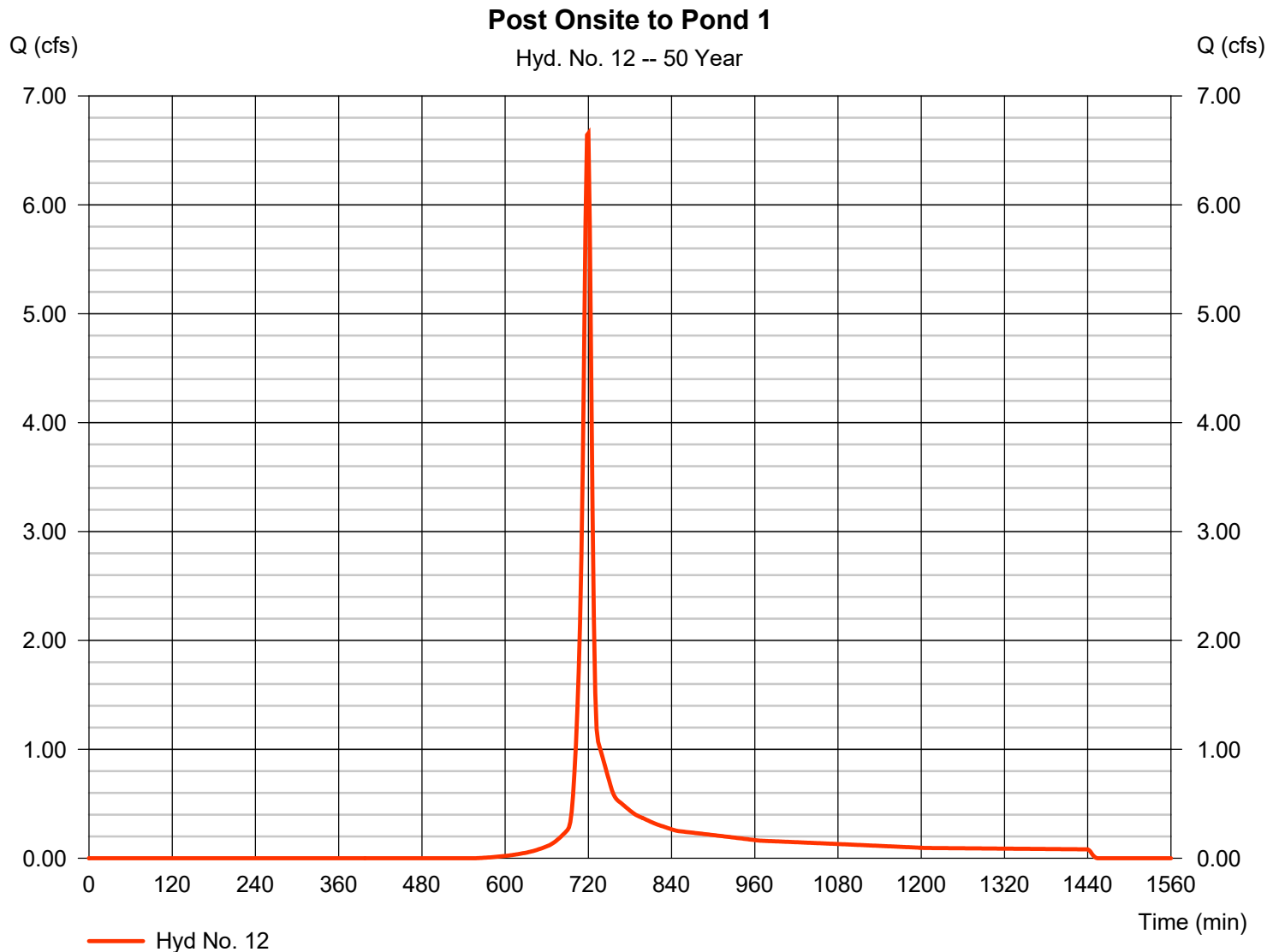
Wednesday, 06 / 3 / 2026

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 1.350 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 6.657 cfs
Time to peak = 720 min
Hyd. volume = 15,242 cuft
Curve number = 65
Hydraulic length = 0 ft
Time of conc. (Tc) = 7.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

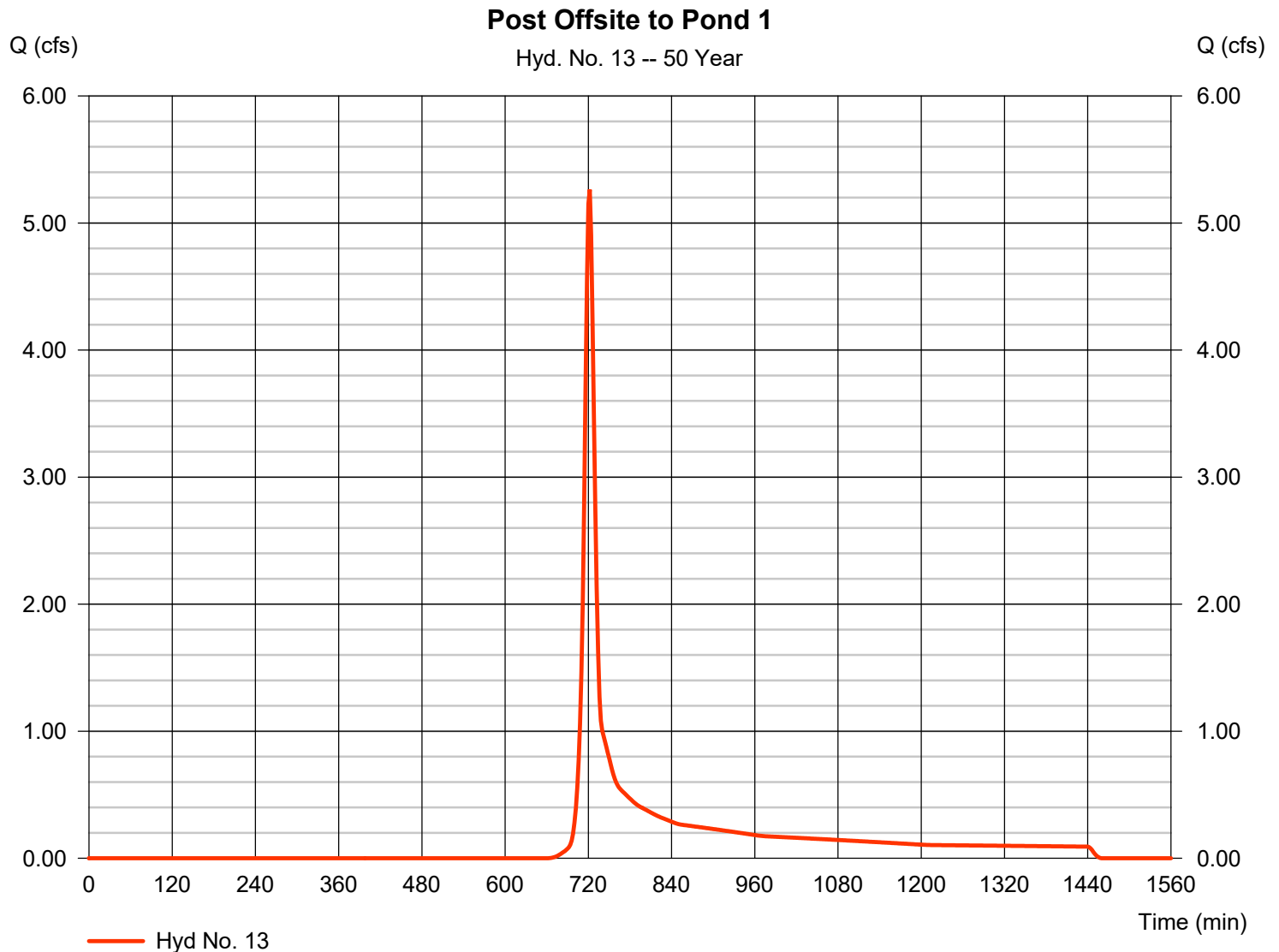
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 5.263 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 14,194 cuft
Drainage area	= 1.780 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

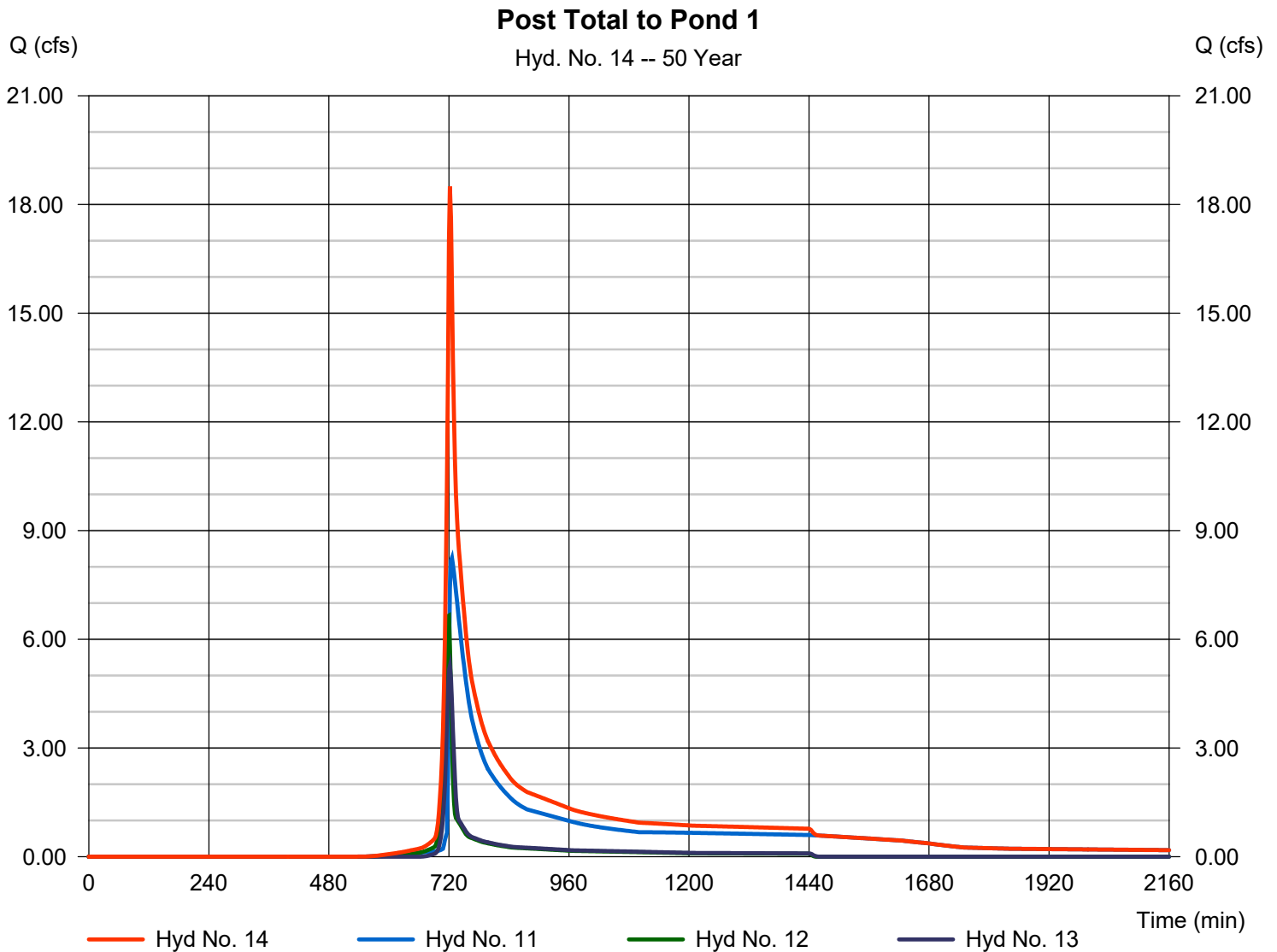
Wednesday, 06 / 3 / 2026

Hyd. No. 14

Post Total to Pond 1

Hydrograph type = Combine
Storm frequency = 50 yrs
Time interval = 2 min
Inflow hyds. = 11, 12, 13

Peak discharge = 18.50 cfs
Time to peak = 722 min
Hyd. volume = 107,681 cuft
Contrib. drain. area = 3.130 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

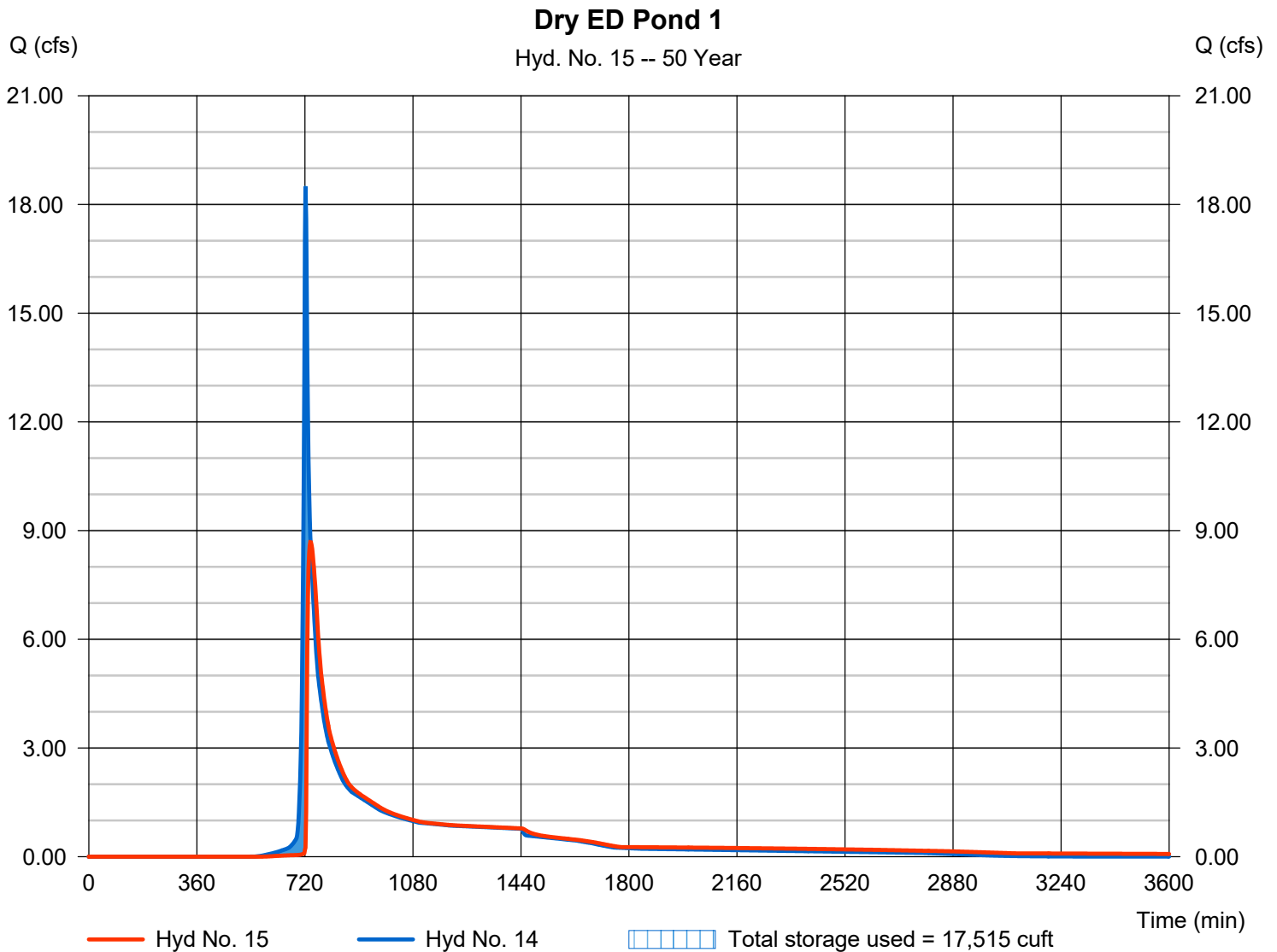
Wednesday, 06 / 3 / 2026

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 8.679 cfs
Storm frequency	= 50 yrs	Time to peak	= 738 min
Time interval	= 2 min	Hyd. volume	= 107,666 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 686.16 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 17,515 cuft

Storage Indication method used.



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

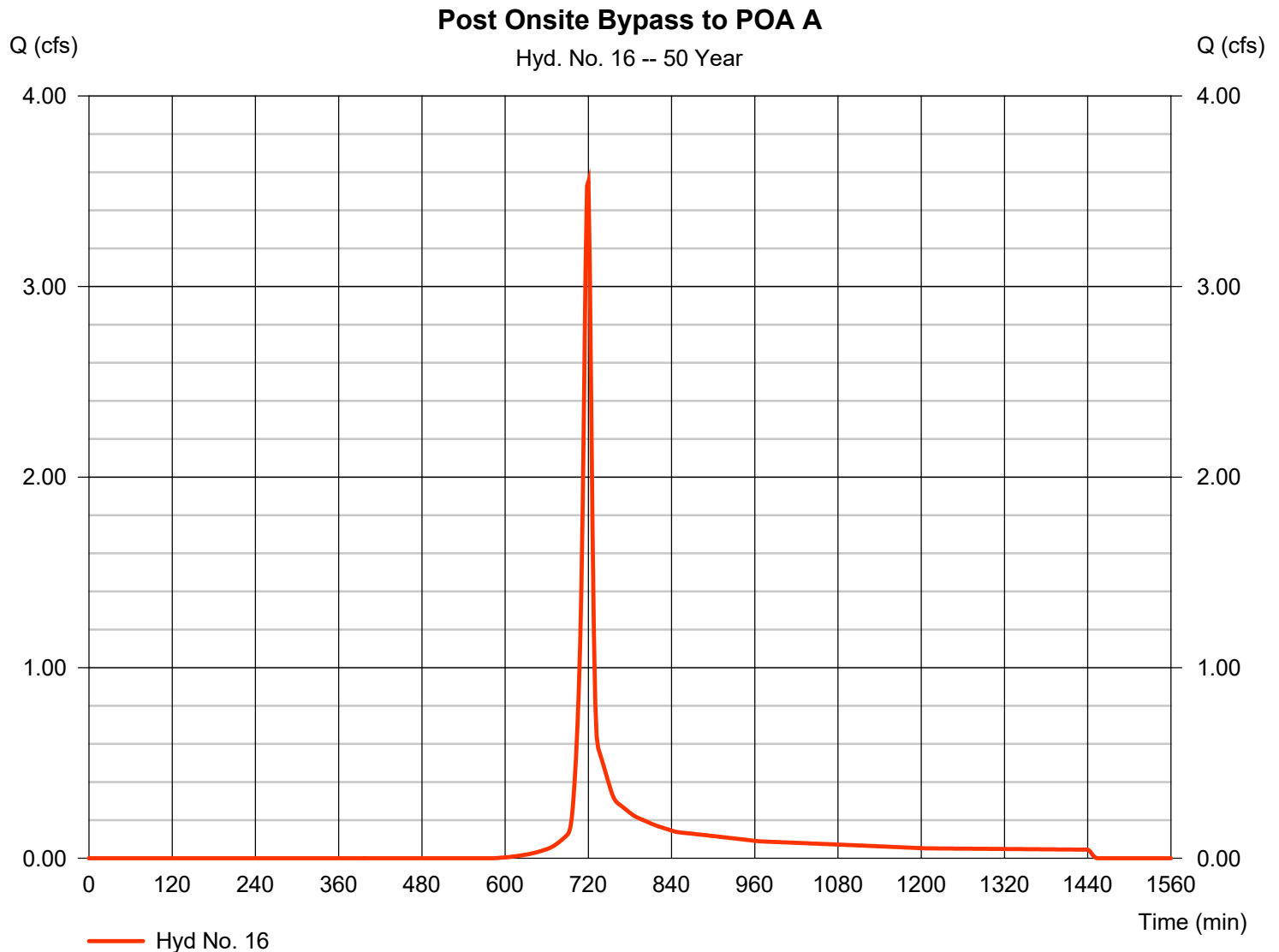
Wednesday, 06 / 3 / 2026

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 0.770 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 3.552 cfs
Time to peak = 720 min
Hyd. volume = 8,129 cuft
Curve number = 63
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

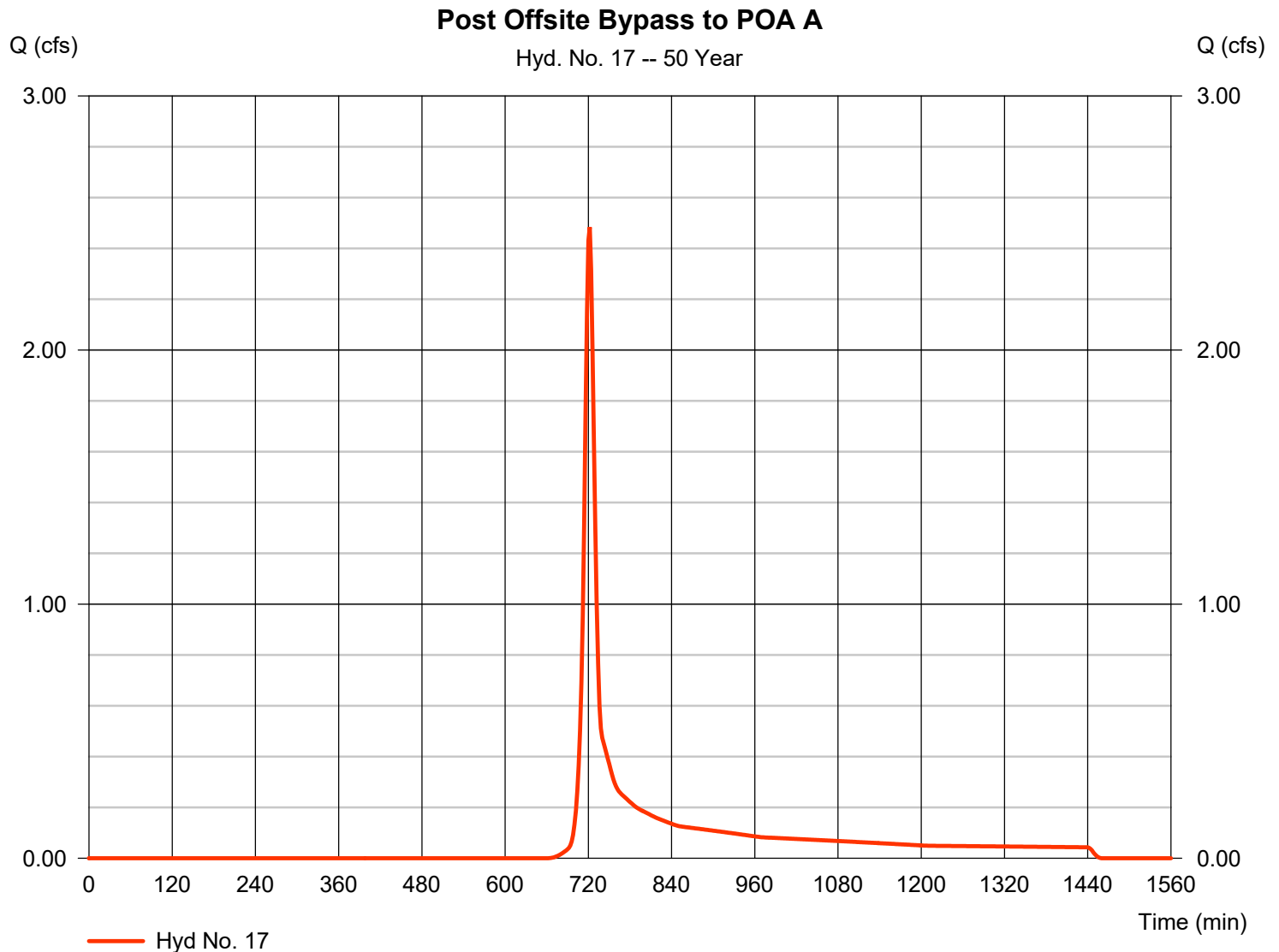
Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 0.840 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 2.484 cfs
Time to peak = 722 min
Hyd. volume = 6,698 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 11.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

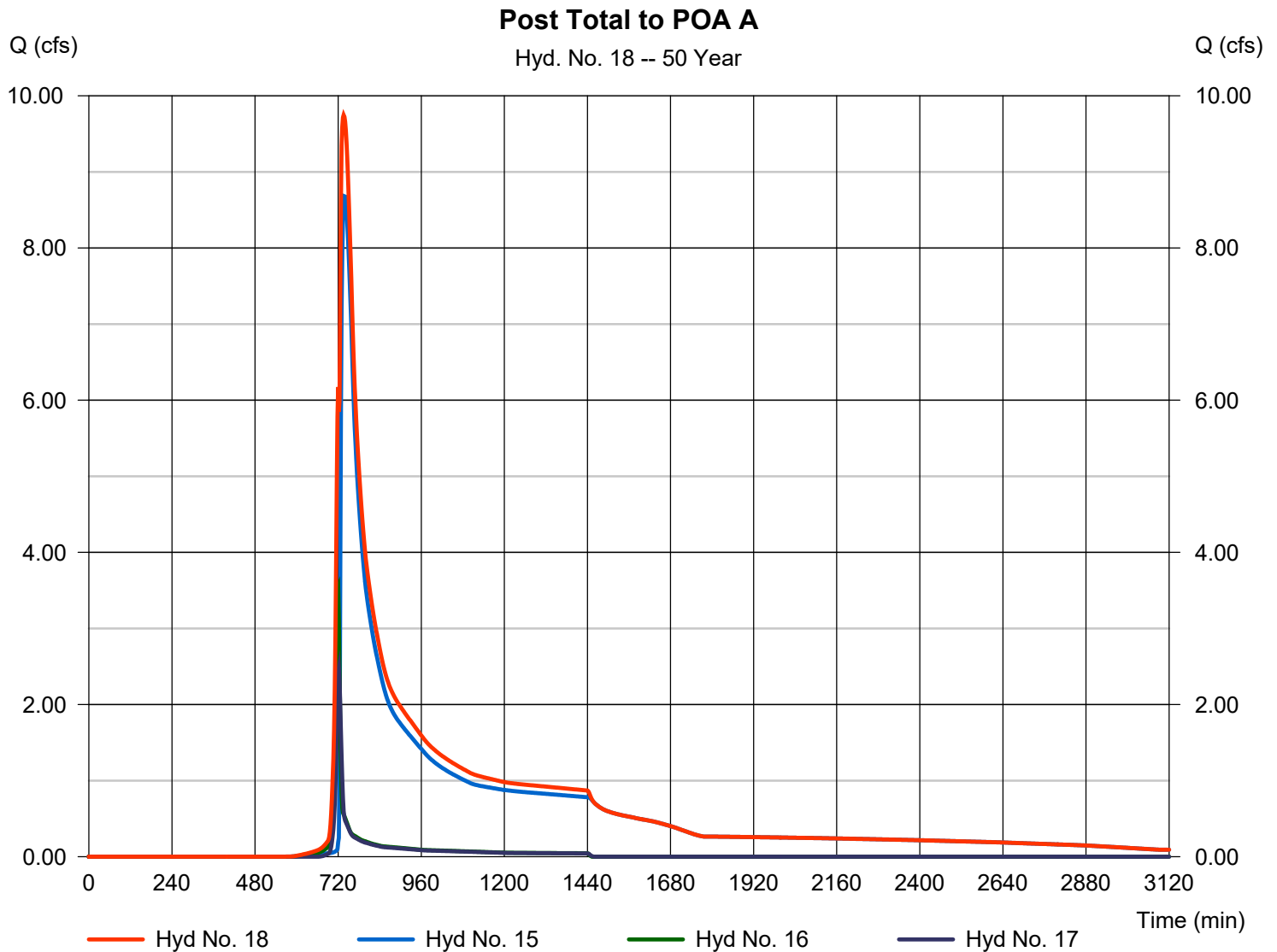
Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type = Combine
Storm frequency = 50 yrs
Time interval = 2 min
Inflow hyds. = 15, 16, 17

Peak discharge = 9.747 cfs
Time to peak = 736 min
Hyd. volume = 122,493 cuft
Contrib. drain. area = 1.610 ac



Hydrograph Report

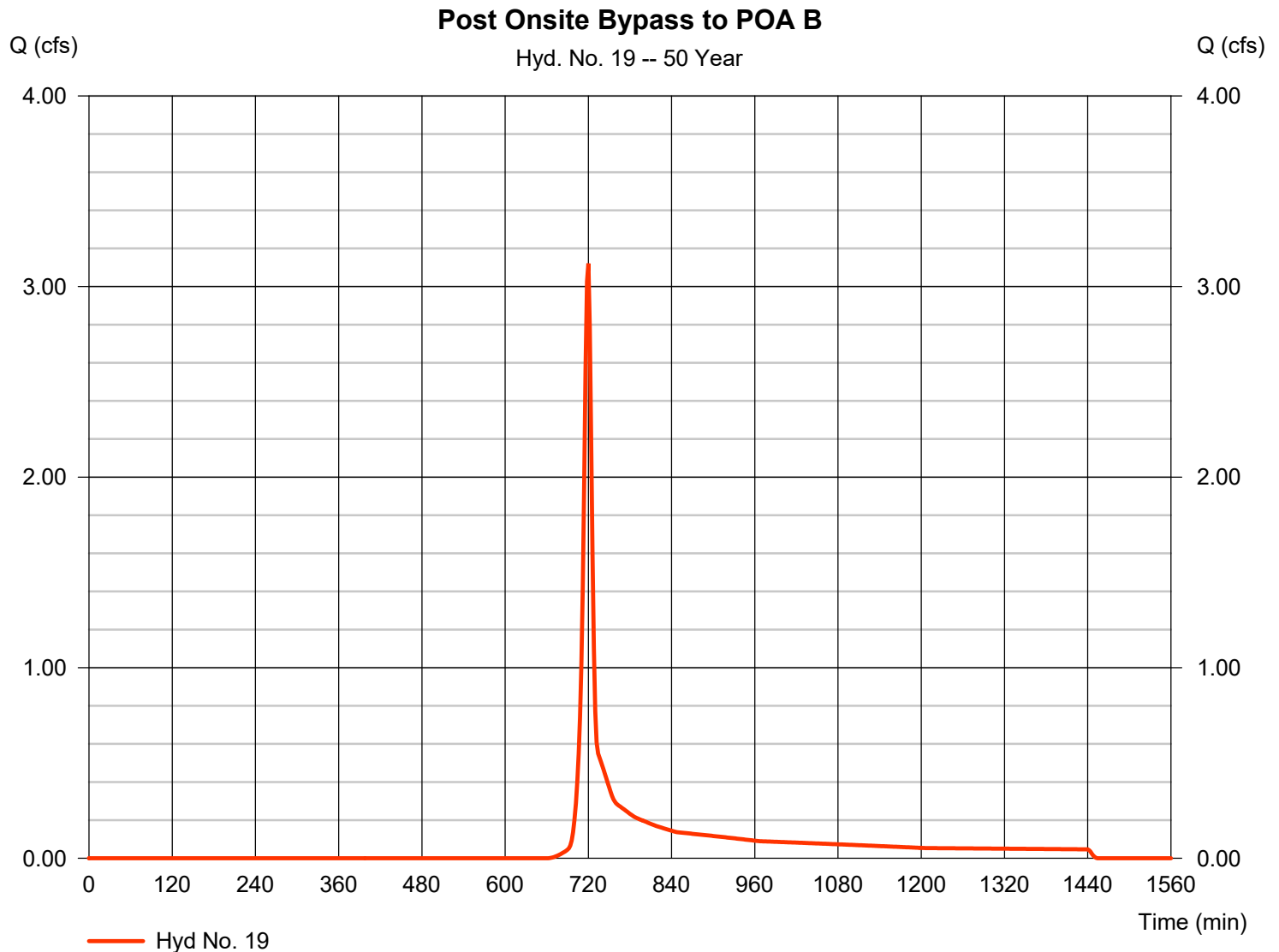
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 3.121 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 7,269 cuft
Drainage area	= 0.940 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

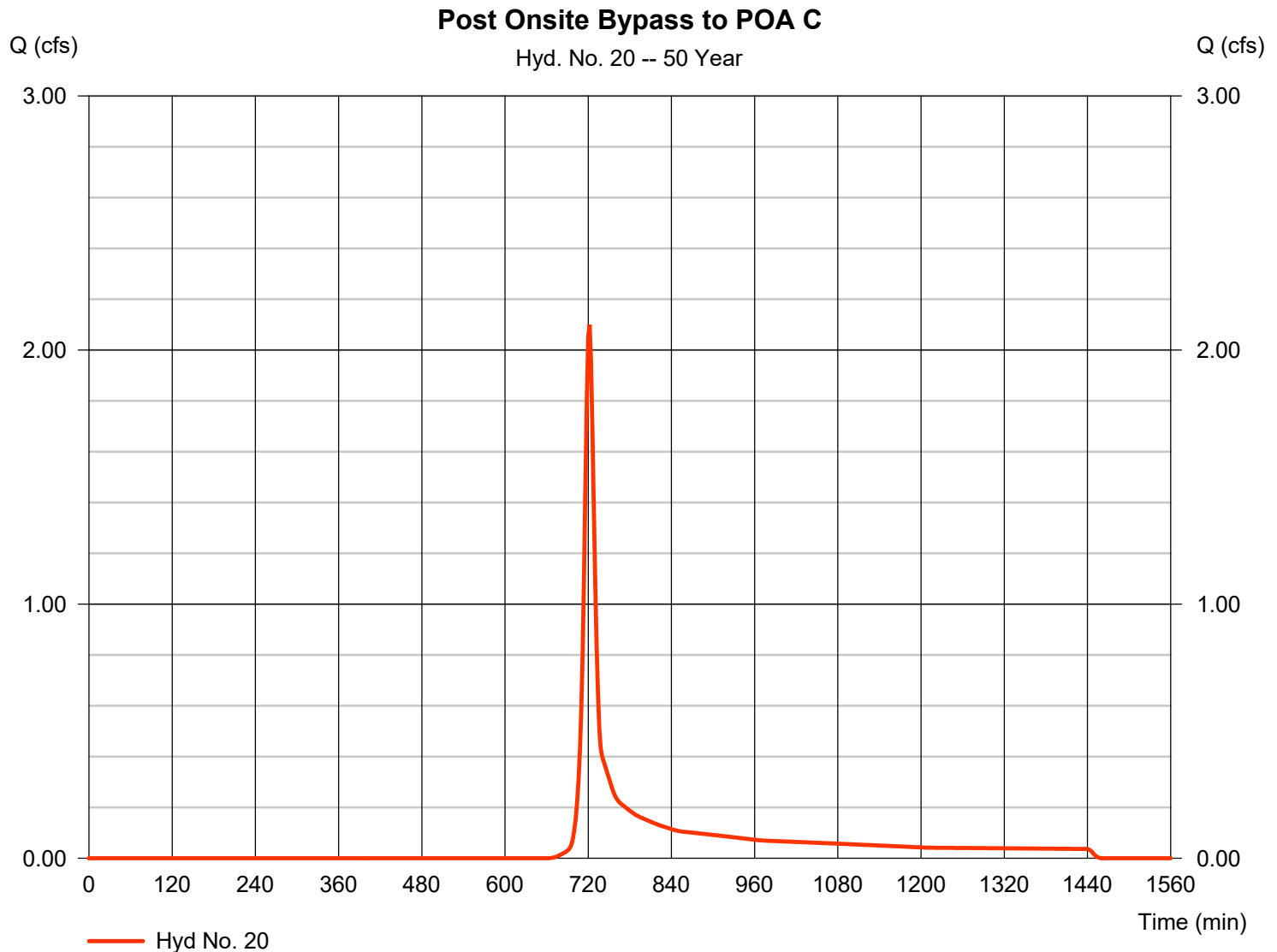
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 2.099 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 5,662 cuft
Drainage area	= 0.710 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 7.01 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

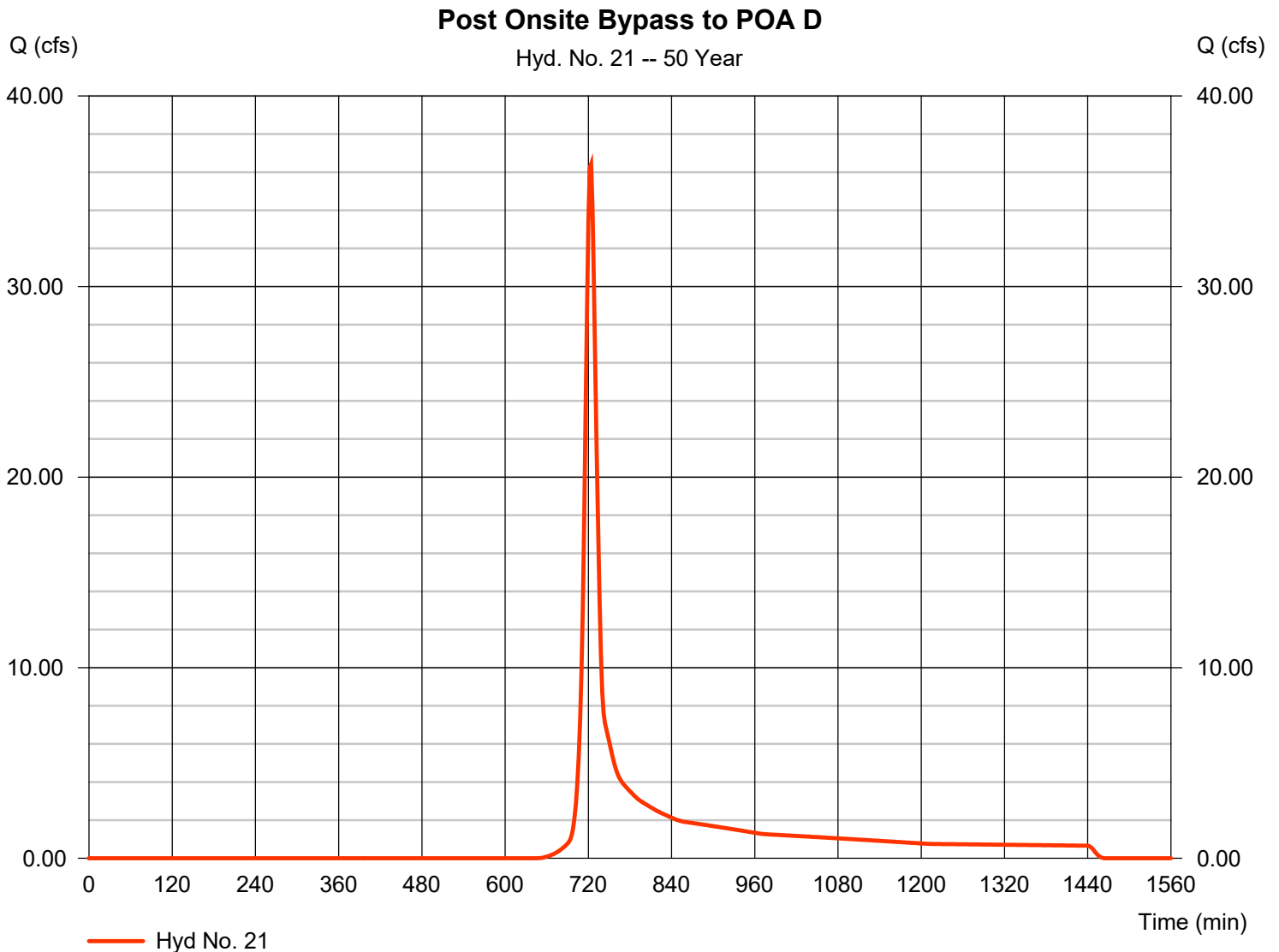
Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type = SCS Runoff
 Storm frequency = 50 yrs
 Time interval = 2 min
 Drainage area = 12.950 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 7.01 in
 Storm duration = 24 hrs

Peak discharge = 36.42 cfs
 Time to peak = 724 min
 Hyd. volume = 106,330 cuft
 Curve number = 57
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 15.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

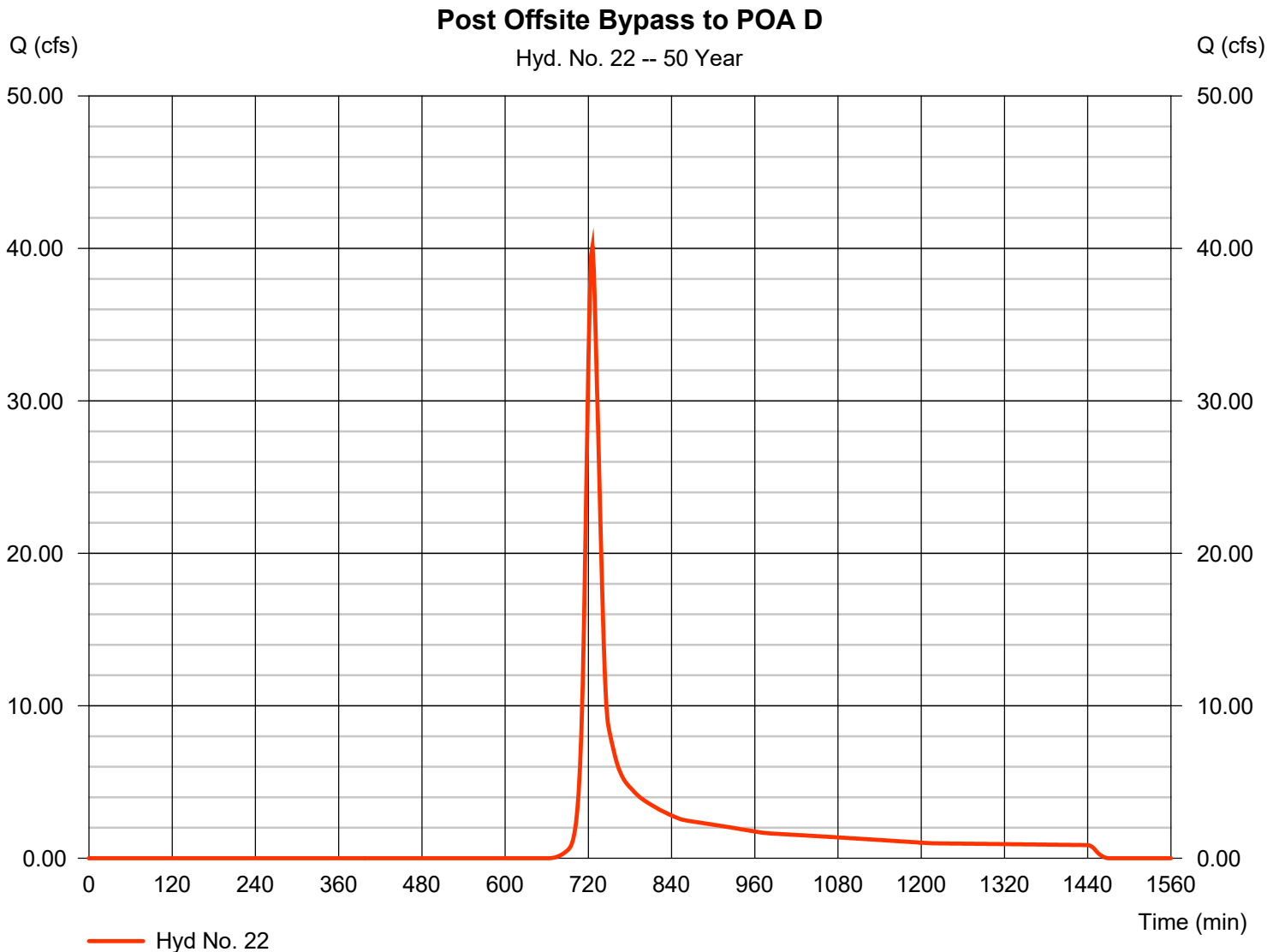
Wednesday, 06 / 3 / 2026

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type = SCS Runoff
Storm frequency = 50 yrs
Time interval = 2 min
Drainage area = 17.290 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.01 in
Storm duration = 24 hrs

Peak discharge = 40.14 cfs
Time to peak = 726 min
Hyd. volume = 133,698 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 17.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

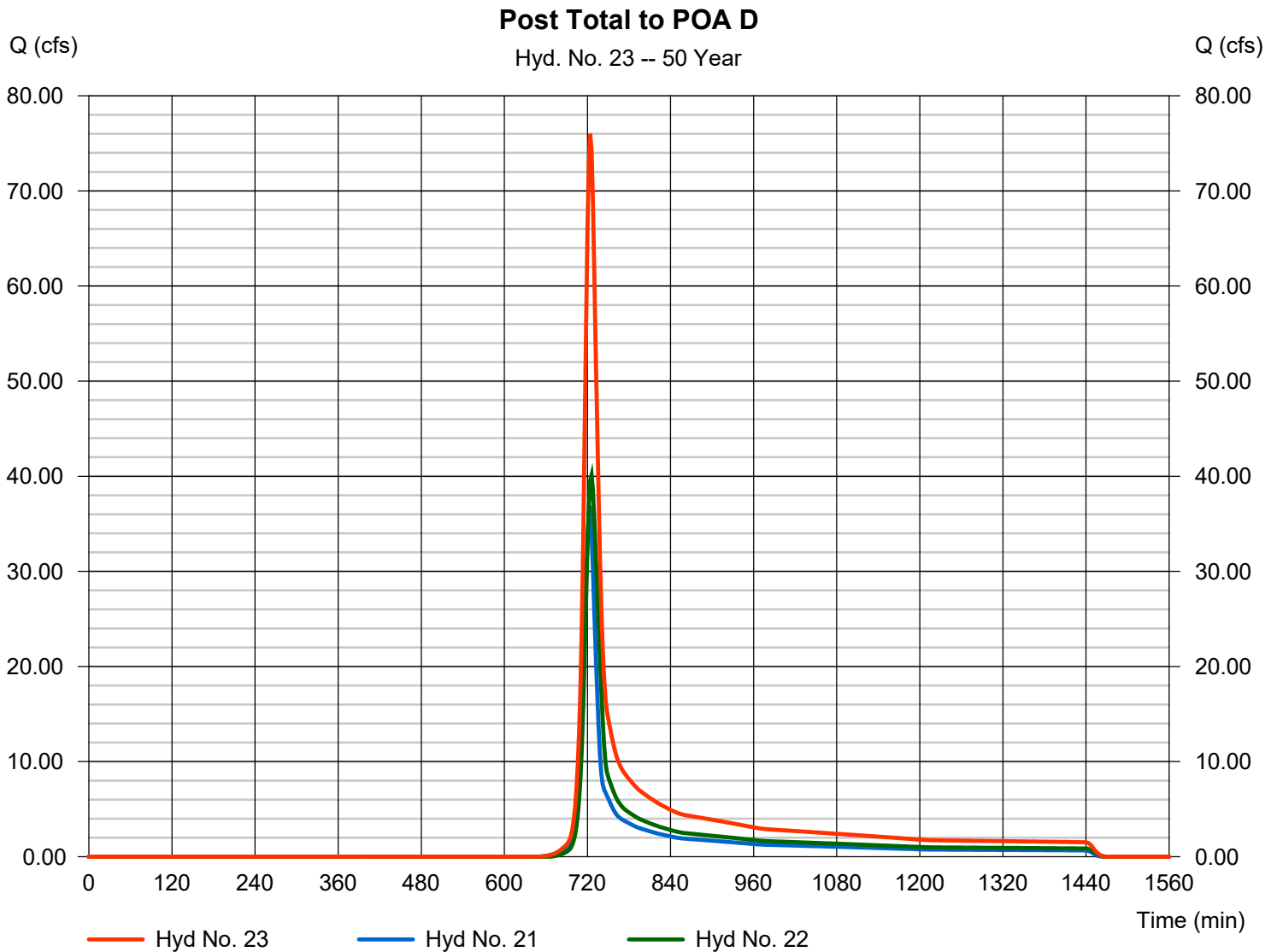
Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type = Combine
Storm frequency = 50 yrs
Time interval = 2 min
Inflow hyds. = 21, 22

Peak discharge = 76.00 cfs
Time to peak = 724 min
Hyd. volume = 240,027 cuft
Contrib. drain. area = 30.240 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	8.720	2	720	20,102	-----	-----	-----	Pre Onsite to POA A
2	SCS Runoff	9.913	2	722	26,339	-----	-----	-----	Pre Offsite to POA A
3	Combine	18.50	2	720	46,441	1, 2	-----	-----	Pre Total to POA A
4	SCS Runoff	24.88	2	722	66,097	-----	-----	-----	Pre Onsite to POA B
5	SCS Runoff	5.126	2	722	13,620	-----	-----	-----	Pre Onsite to POA C
6	SCS Runoff	43.04	2	724	125,173	-----	-----	-----	Pre Onsite to POA D
7	SCS Runoff	51.44	2	726	167,908	-----	-----	-----	Pre Offsite to POA D
8	Combine	94.09	2	724	293,081	6, 7	-----	-----	Pre Total to POA D
10	SCS Runoff	46.26	2	716	93,645	-----	-----	-----	Post Onsite to Pond 2
11	Reservoir	13.80	2	724	93,619	10	698.52	40,067	Dry ED Pond 2
12	SCS Runoff	8.062	2	718	18,452	-----	-----	-----	Post Onsite to Pond 1
13	SCS Runoff	6.709	2	722	17,826	-----	-----	-----	Post Offsite to Pond 1
14	Combine	26.82	2	722	129,897	11, 12, 13	-----	-----	Post Total to Pond 1
15	Reservoir	14.77	2	732	129,881	14	686.67	21,013	Dry ED Pond 1
16	SCS Runoff	4.324	2	720	9,904	-----	-----	-----	Post Onsite Bypass to POA A
17	SCS Runoff	3.166	2	722	8,412	-----	-----	-----	Post Offsite Bypass to POA A
18	Combine	17.04	2	730	148,197	15, 16, 17	-----	-----	Post Total to POA A
19	SCS Runoff	3.960	2	720	9,129	-----	-----	-----	Post Onsite Bypass to POA B
20	SCS Runoff	2.676	2	722	7,110	-----	-----	-----	Post Onsite Bypass to POA C
21	SCS Runoff	45.92	2	722	132,410	-----	-----	-----	Post Onsite Bypass to POA D
22	SCS Runoff	51.44	2	726	167,908	-----	-----	-----	Post Offsite Bypass to POA D
23	Combine	96.95	2	724	300,318	21, 22	-----	-----	Post Total to POA D
JKS002 5-21-26.gpw					Return Period: 100 Year			Wednesday, 06 / 3 / 2026	

Hydrograph Report

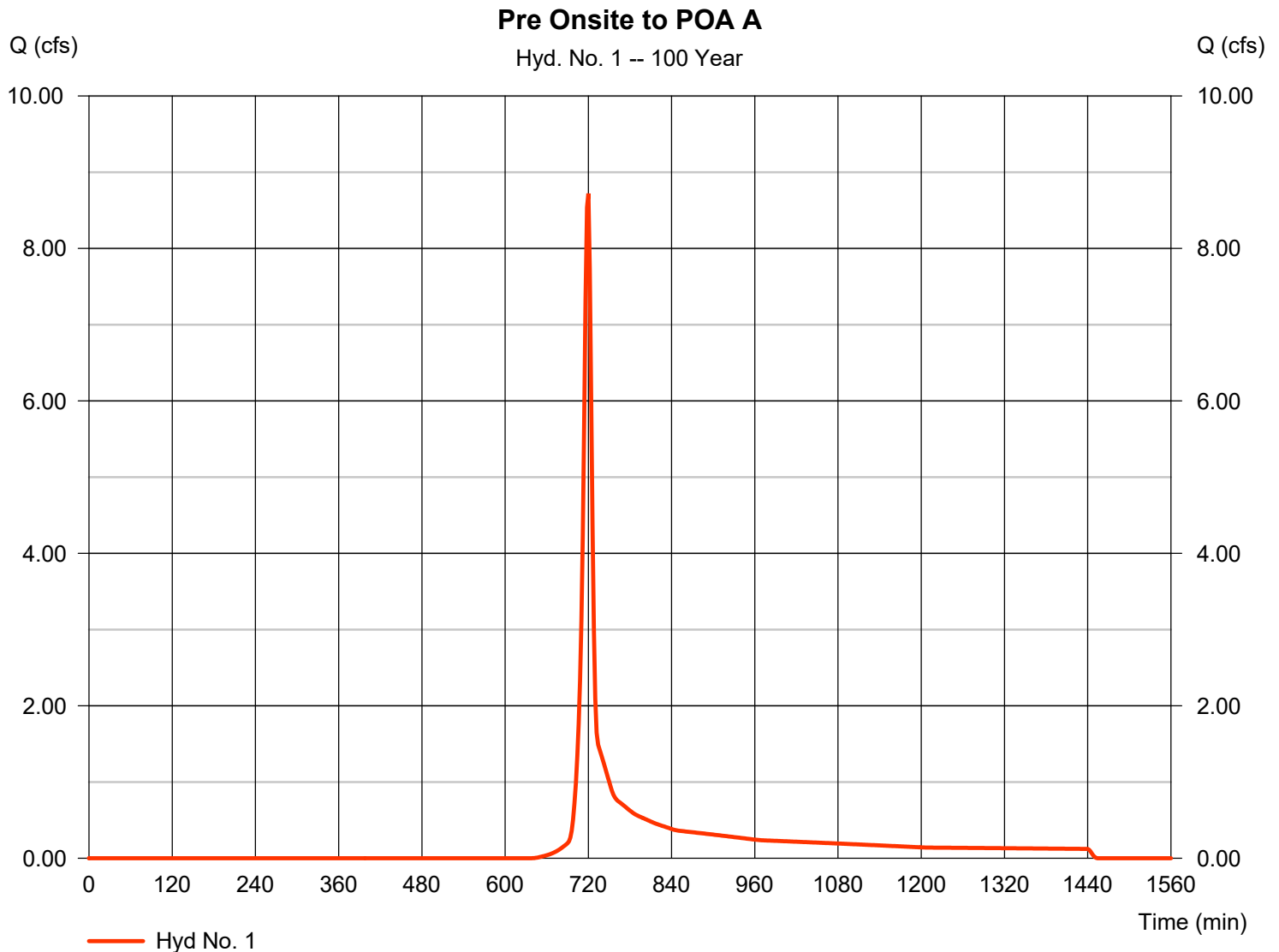
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 1

Pre Onsite to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 8.720 cfs
Storm frequency	= 100 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 20,102 cuft
Drainage area	= 2.070 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 9.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

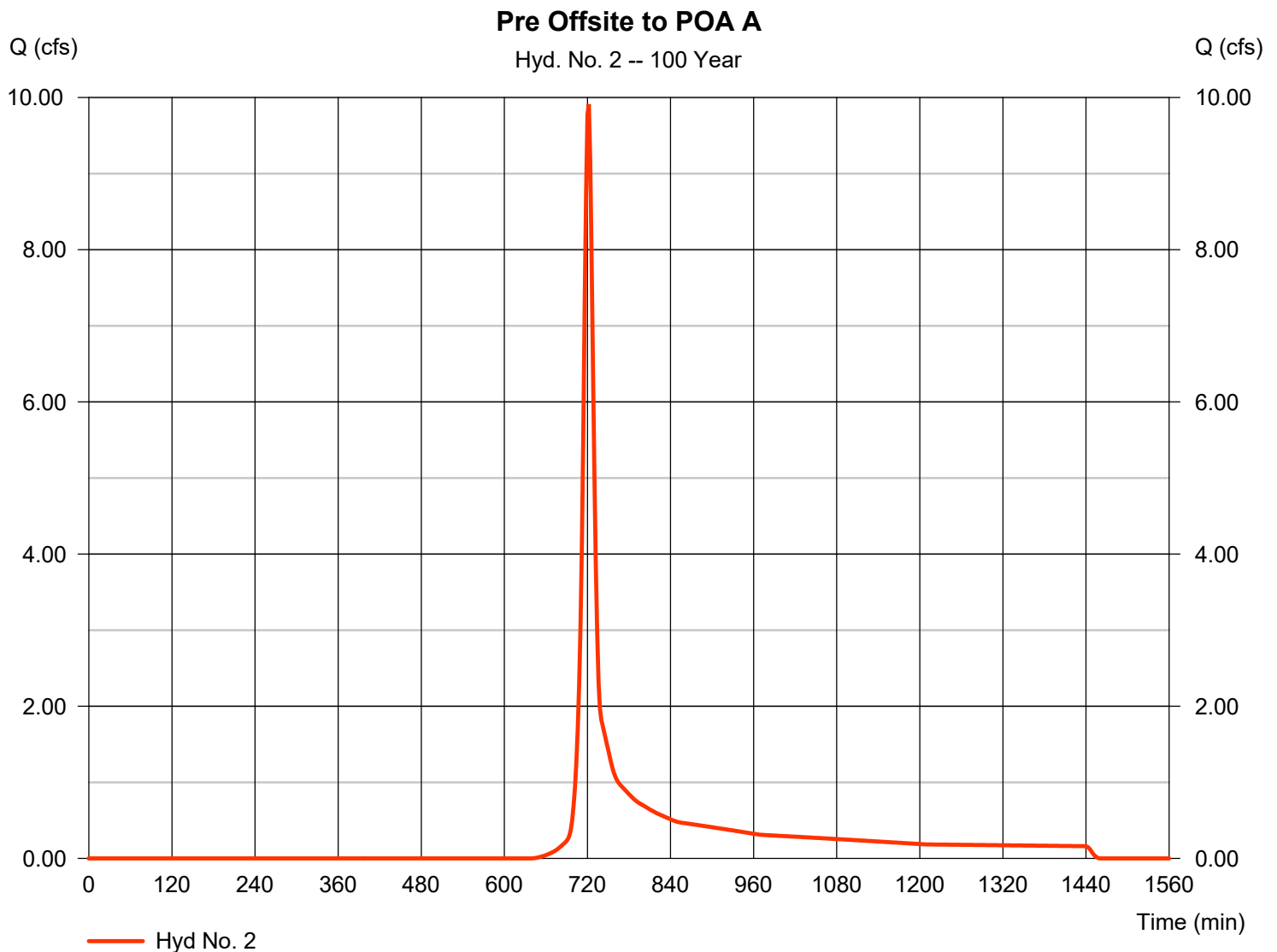
Wednesday, 06 / 3 / 2026

Hyd. No. 2

Pre Offsite to POA A

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 2 min
Drainage area = 2.630 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.84 in
Storm duration = 24 hrs

Peak discharge = 9.913 cfs
Time to peak = 722 min
Hyd. volume = 26,339 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 12.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

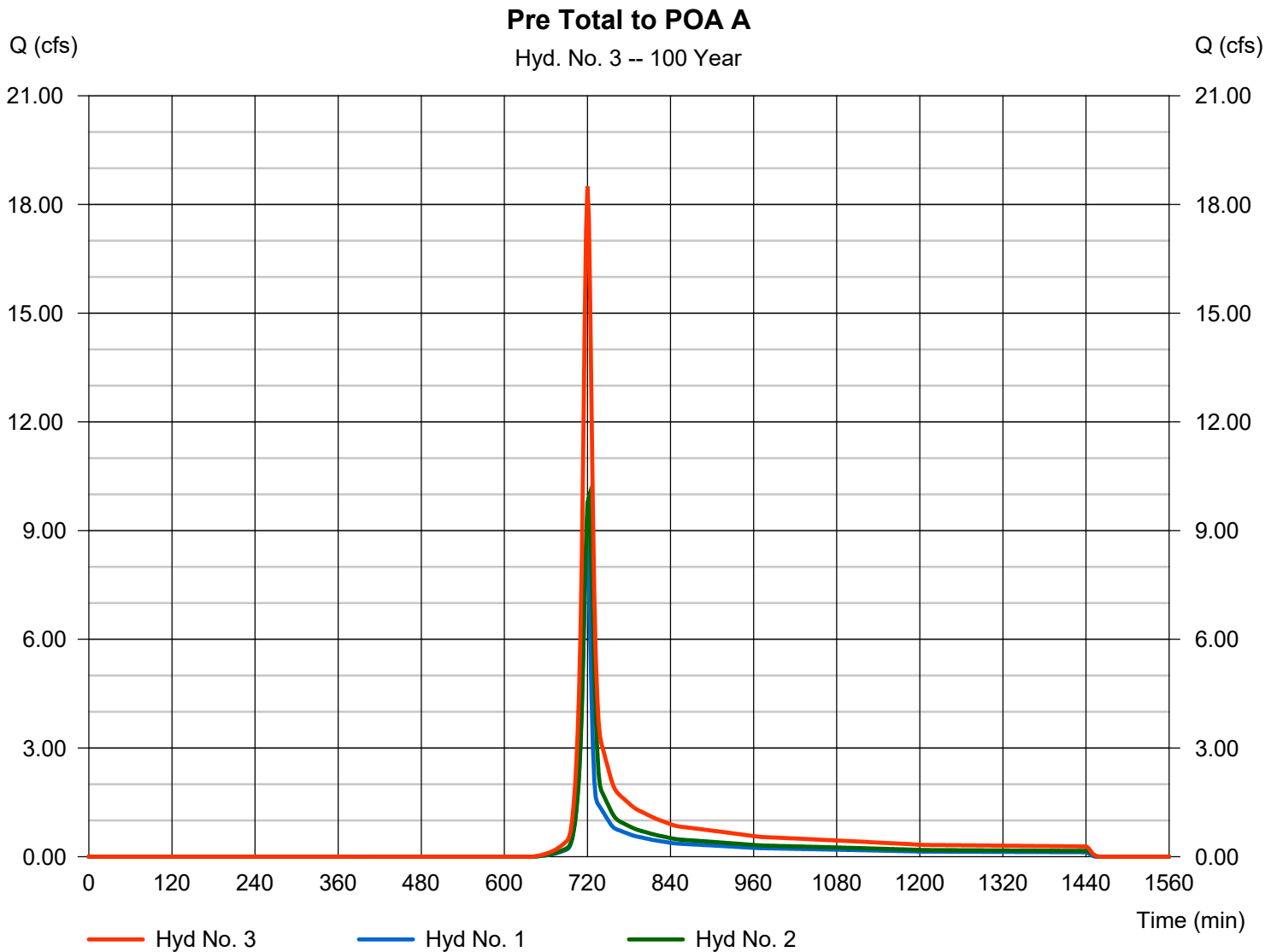
Wednesday, 06 / 3 / 2026

Hyd. No. 3

Pre Total to POA A

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 1, 2

Peak discharge = 18.50 cfs
Time to peak = 720 min
Hyd. volume = 46,441 cuft
Contrib. drain. area = 4.700 ac



Hydrograph Report

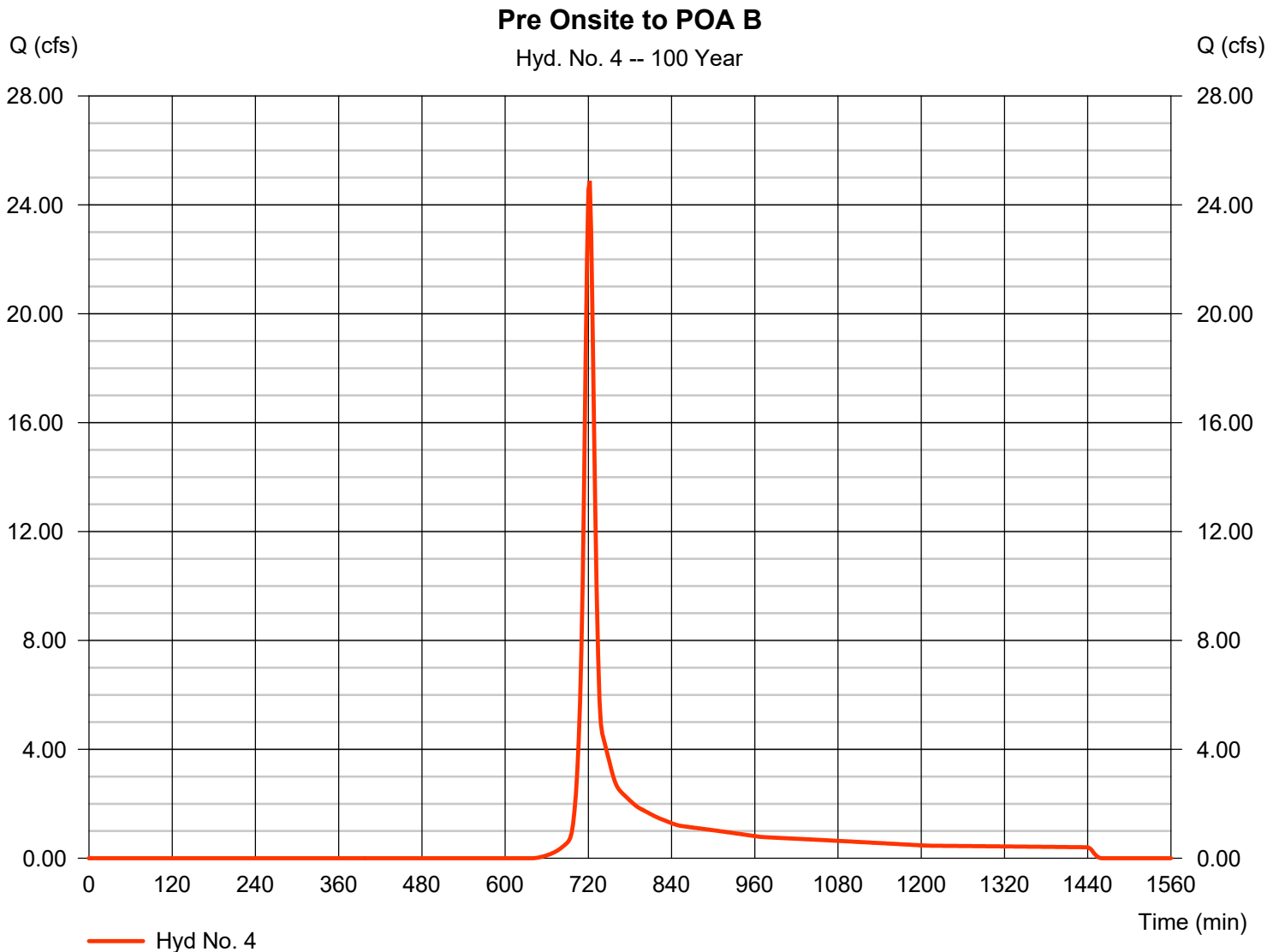
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 4

Pre Onsite to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 24.88 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 66,097 cuft
Drainage area	= 6.600 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

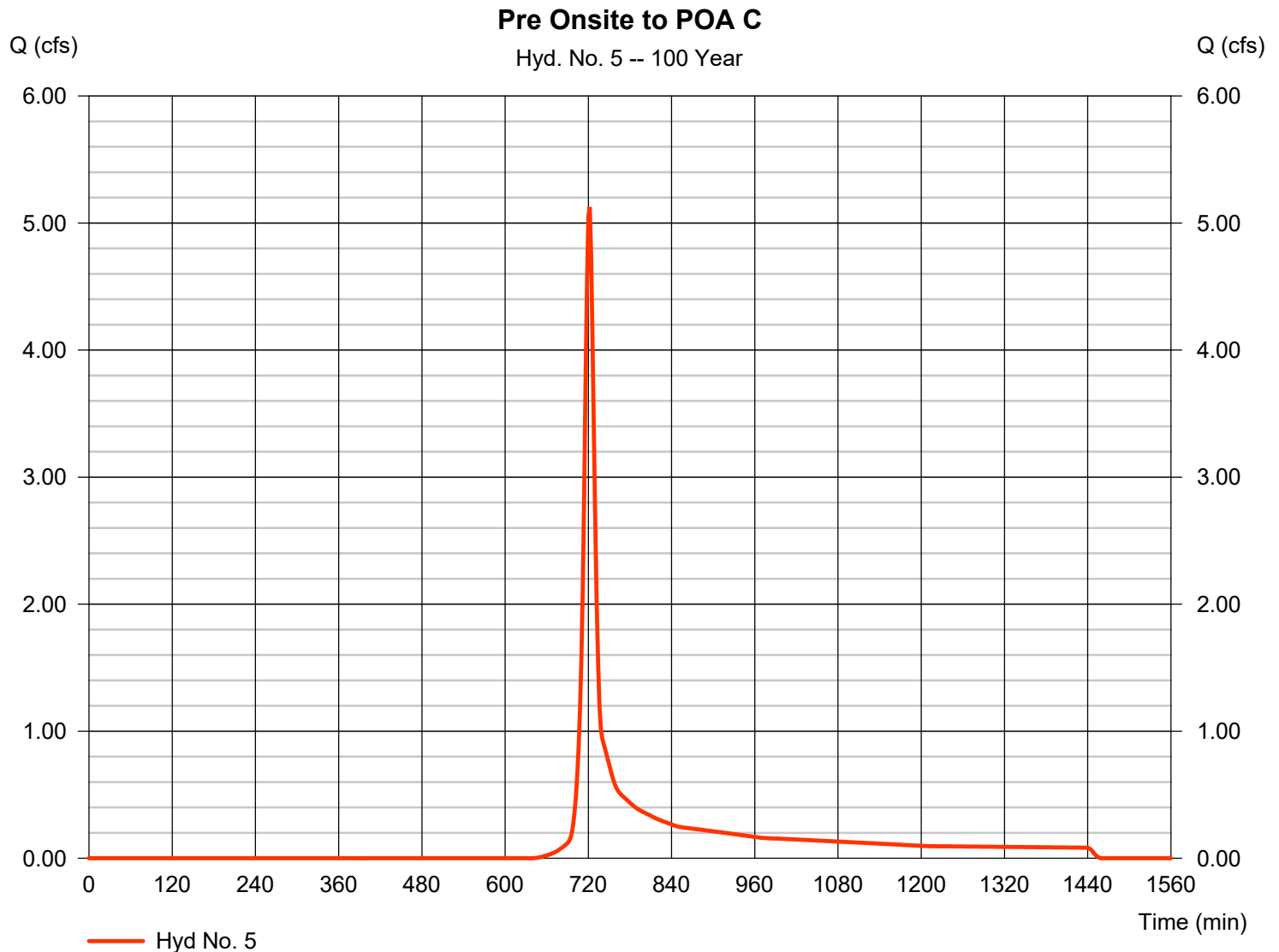
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 5

Pre Onsite to POA C

Hydrograph type	= SCS Runoff	Peak discharge	= 5.126 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 13,620 cuft
Drainage area	= 1.360 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

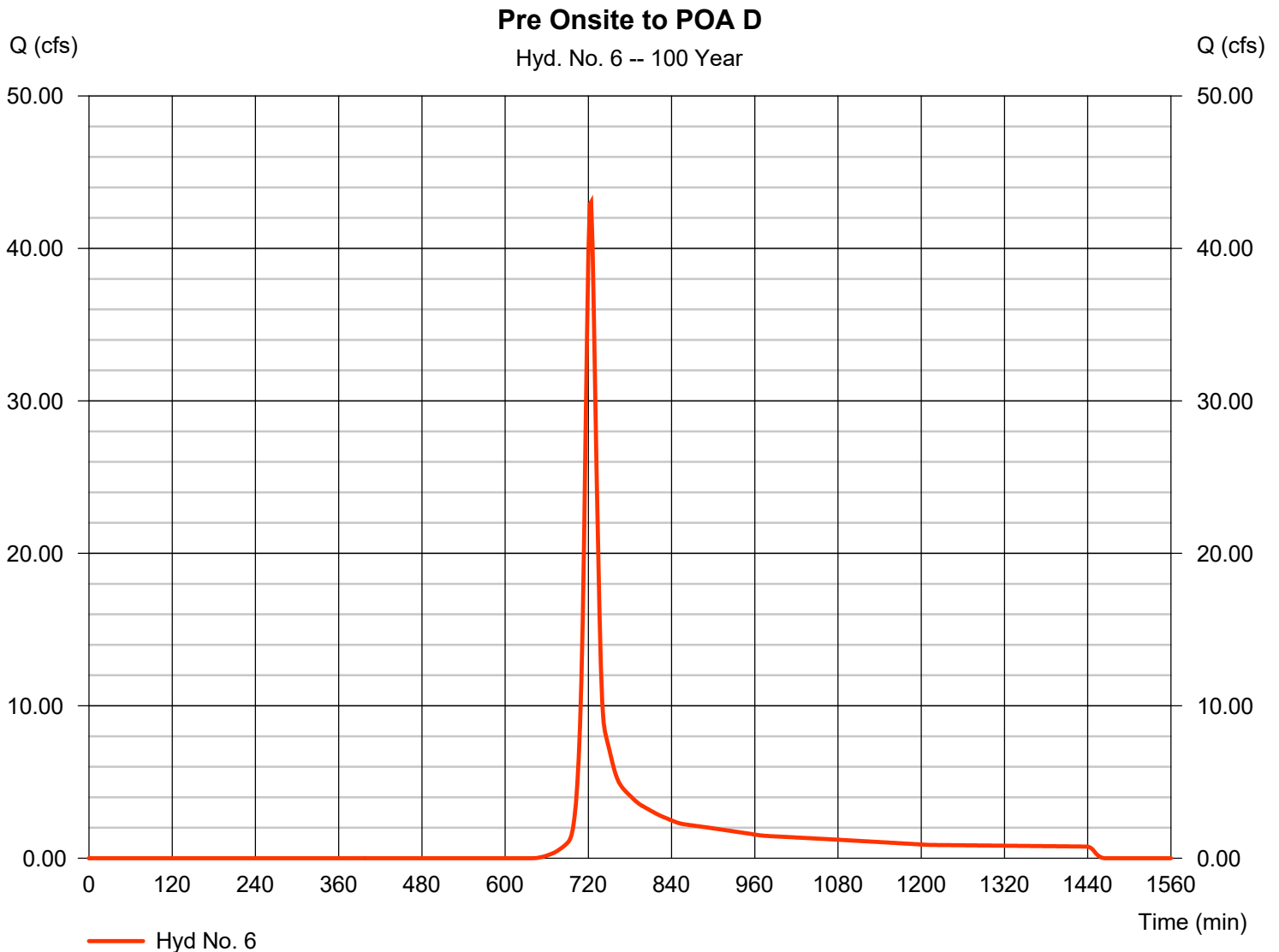
Wednesday, 06 / 3 / 2026

Hyd. No. 6

Pre Onsite to POA D

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 2 min
Drainage area = 13.220 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.84 in
Storm duration = 24 hrs

Peak discharge = 43.04 cfs
Time to peak = 724 min
Hyd. volume = 125,173 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 15.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

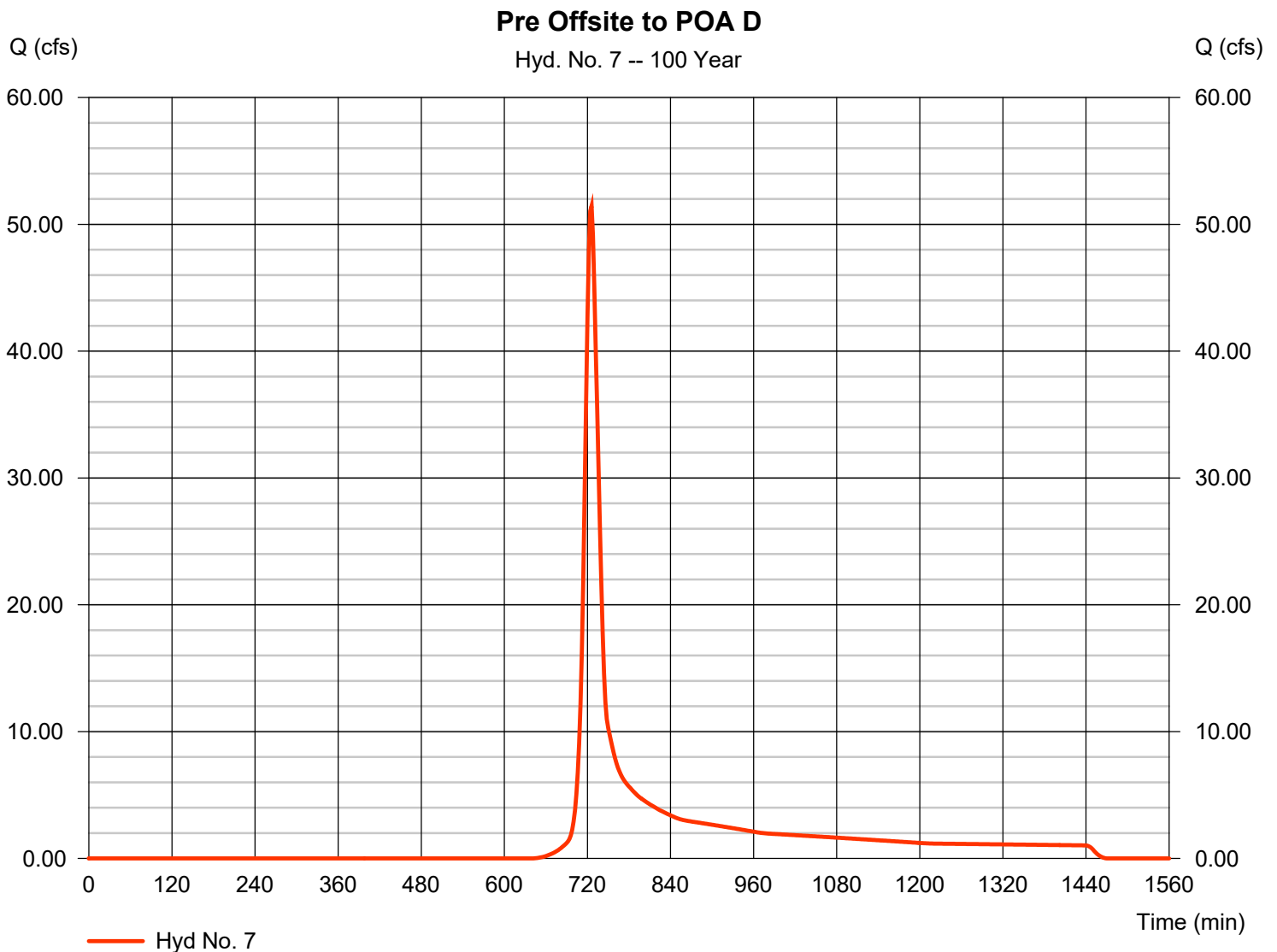
Wednesday, 06 / 3 / 2026

Hyd. No. 7

Pre Offsite to POA D

Hydrograph type = SCS Runoff
 Storm frequency = 100 yrs
 Time interval = 2 min
 Drainage area = 17.290 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 7.84 in
 Storm duration = 24 hrs

Peak discharge = 51.44 cfs
 Time to peak = 726 min
 Hyd. volume = 167,908 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 17.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

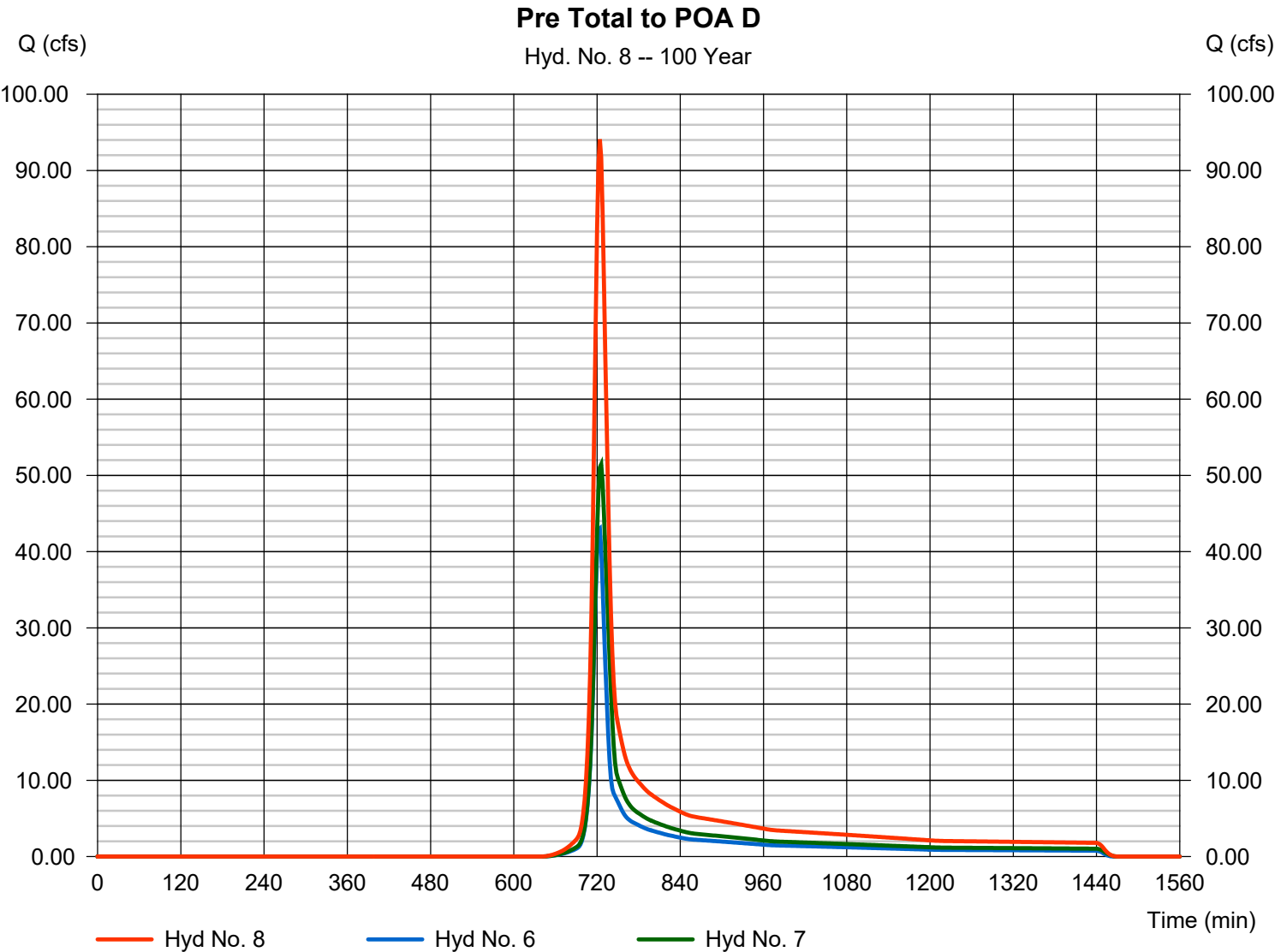
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 8

Pre Total to POA D

Hydrograph type	= Combine	Peak discharge	= 94.09 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 293,081 cuft
Inflow hyds.	= 6, 7	Contrib. drain. area	= 30.510 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

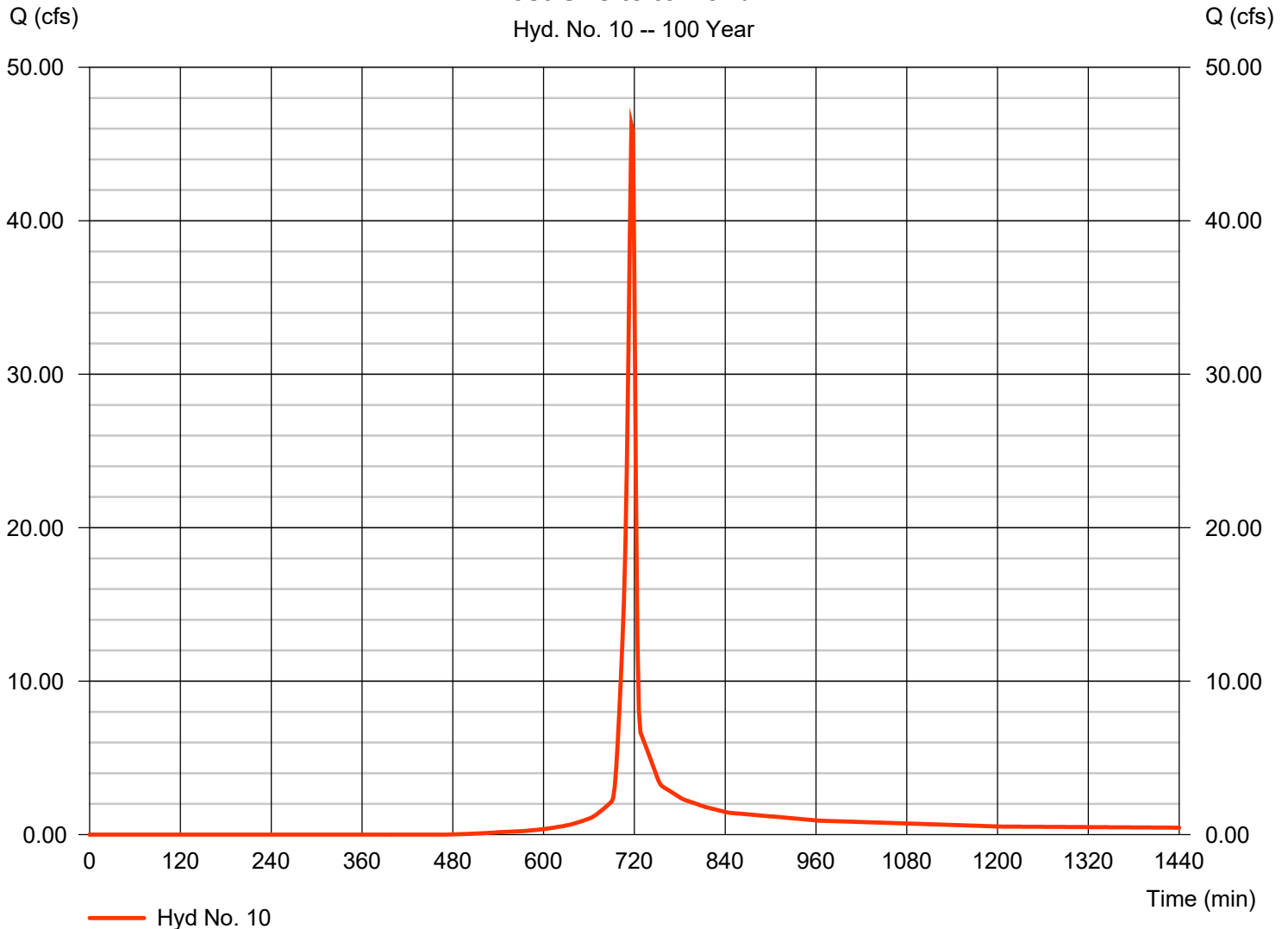
Wednesday, 06 / 3 / 2026

Hyd. No. 10

Post Onsite to Pond 2

Hydrograph type	= SCS Runoff	Peak discharge	= 46.26 cfs
Storm frequency	= 100 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 93,645 cuft
Drainage area	= 6.530 ac	Curve number	= 69
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

Post Onsite to Pond 2



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

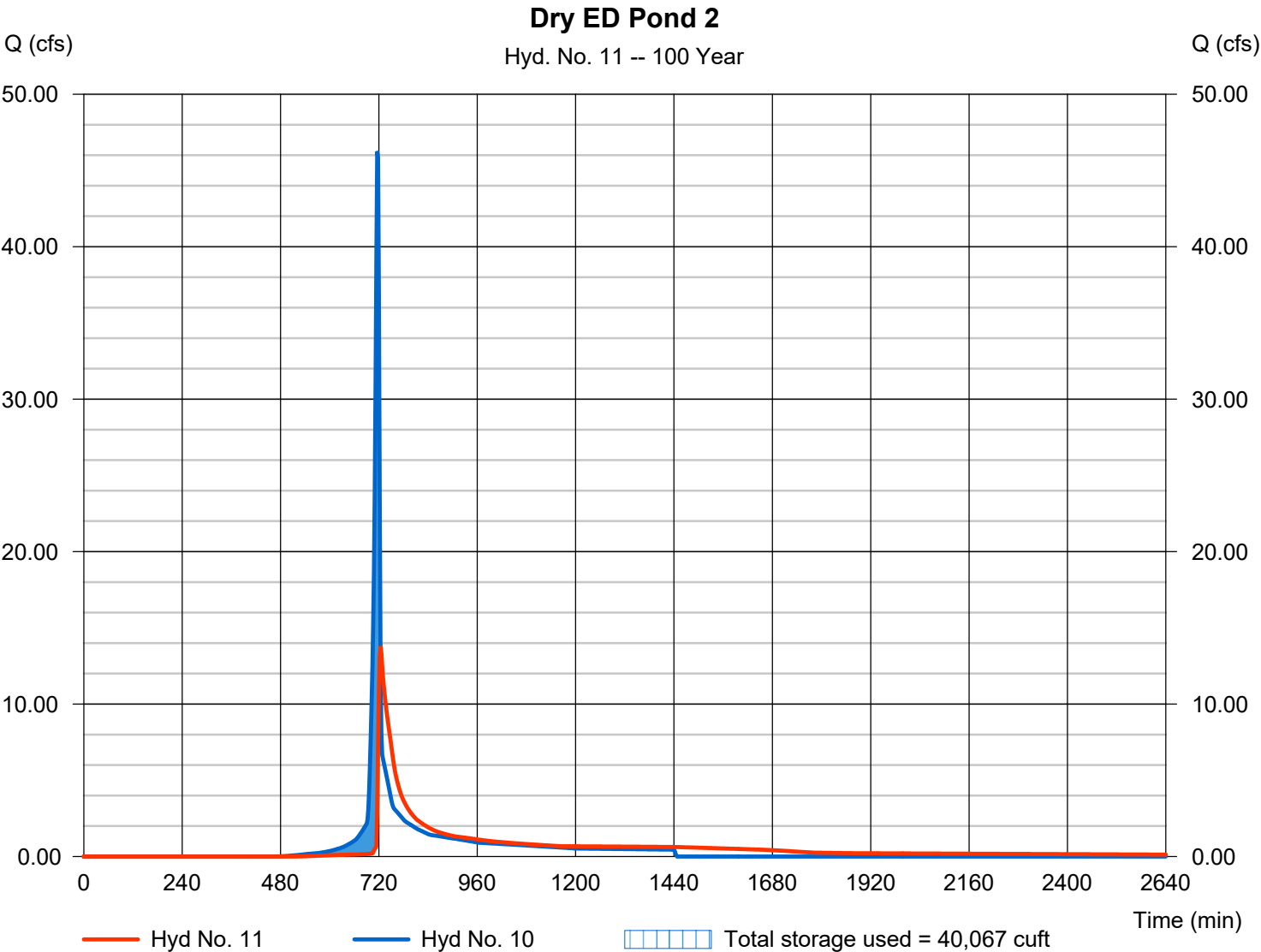
Wednesday, 06 / 3 / 2026

Hyd. No. 11

Dry ED Pond 2

Hydrograph type	= Reservoir	Peak discharge	= 13.80 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 93,619 cuft
Inflow hyd. No.	= 10 - Post Onsite to Pond 2	Max. Elevation	= 698.52 ft
Reservoir name	= Dry ED Pond 2	Max. Storage	= 40,067 cuft

Storage Indication method used.



Hydrograph Report

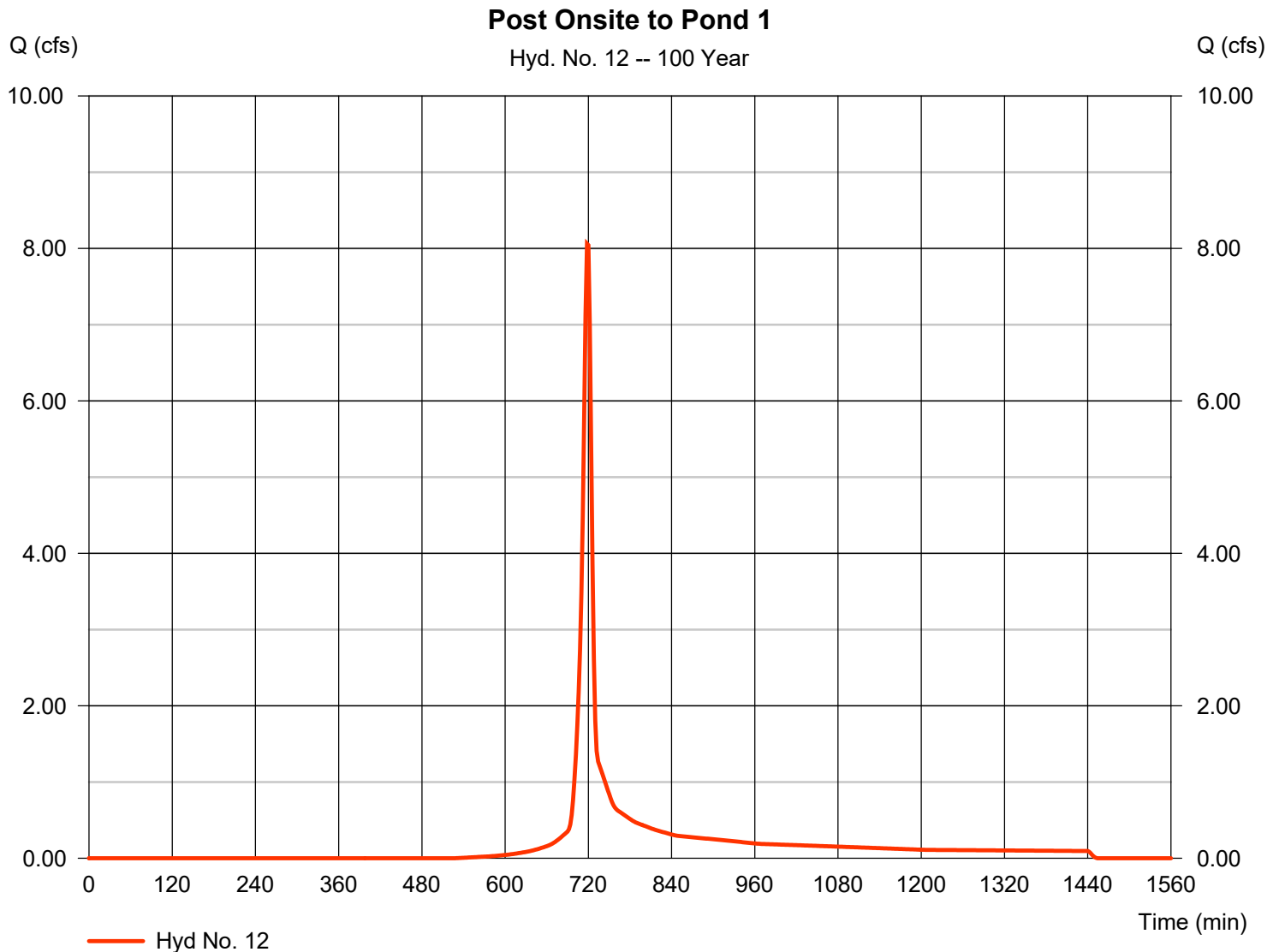
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 12

Post Onsite to Pond 1

Hydrograph type	= SCS Runoff	Peak discharge	= 8.062 cfs
Storm frequency	= 100 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 18,452 cuft
Drainage area	= 1.350 ac	Curve number	= 65
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

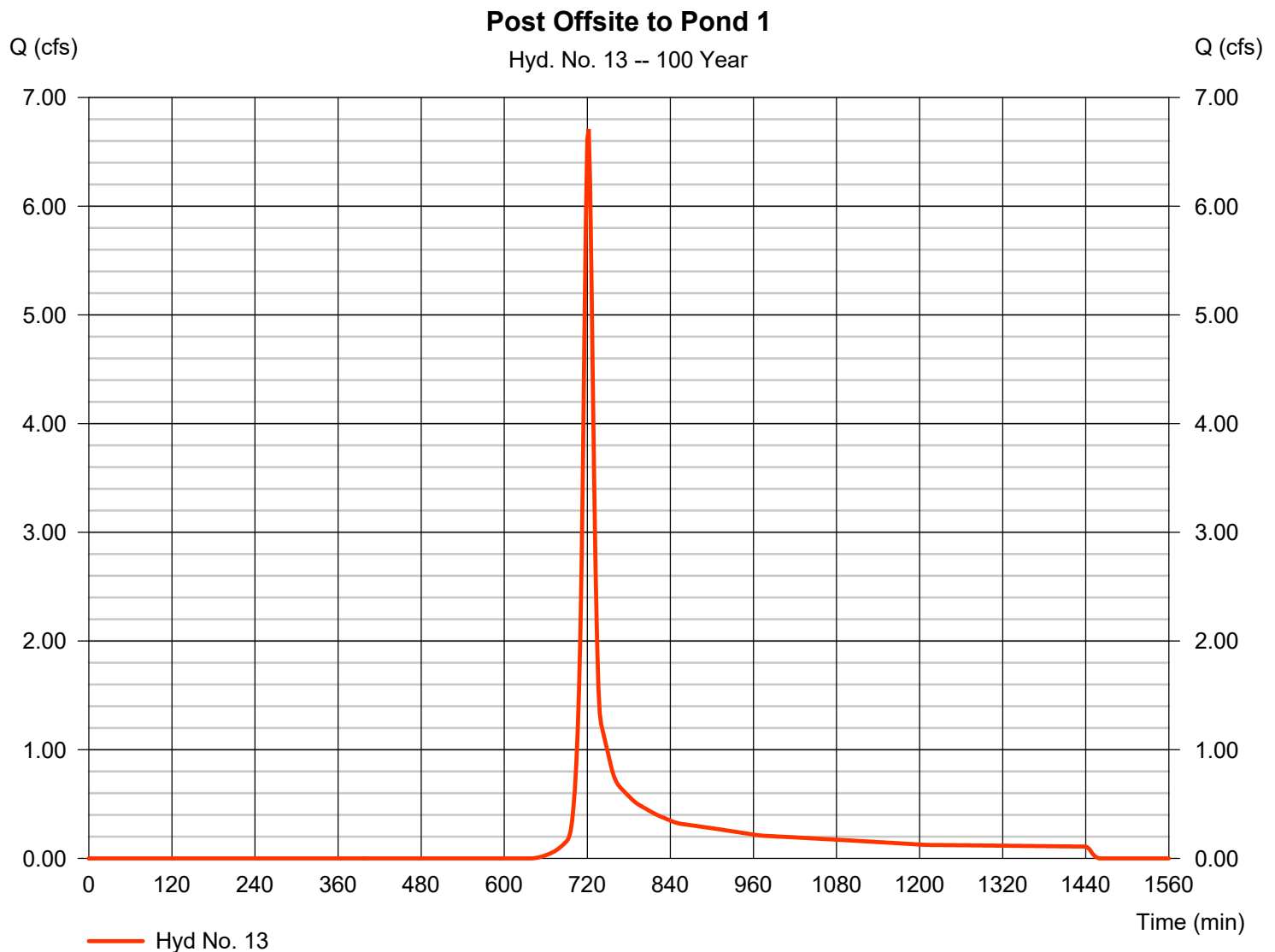
Wednesday, 06 / 3 / 2026

Hyd. No. 13

Post Offsite to Pond 1

Hydrograph type = SCS Runoff
 Storm frequency = 100 yrs
 Time interval = 2 min
 Drainage area = 1.780 ac
 Basin Slope = 0.0 %
 Tc method = User
 Total precip. = 7.84 in
 Storm duration = 24 hrs

Peak discharge = 6.709 cfs
 Time to peak = 722 min
 Hyd. volume = 17,826 cuft
 Curve number = 55
 Hydraulic length = 0 ft
 Time of conc. (Tc) = 12.00 min
 Distribution = Type II
 Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

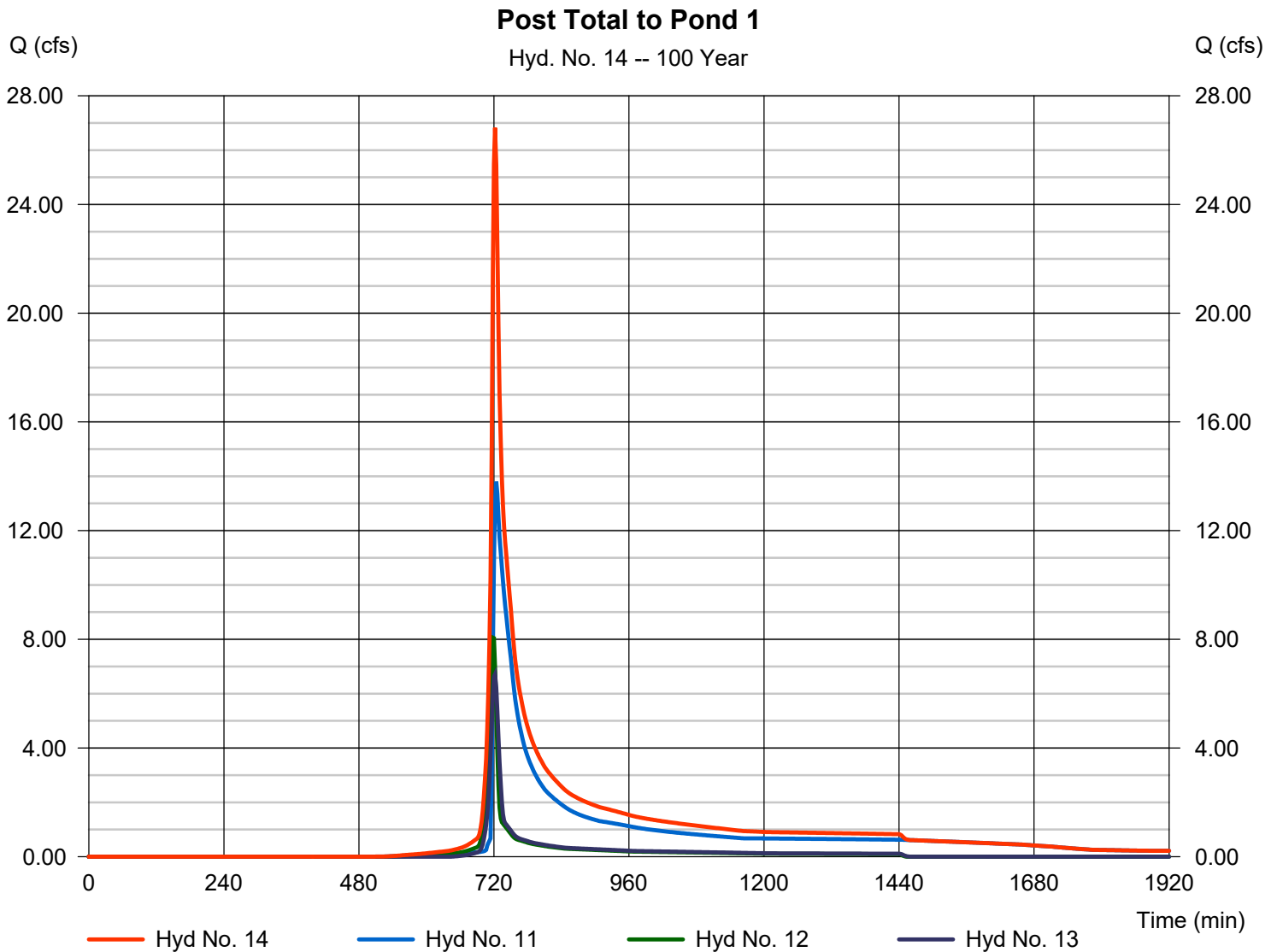
Wednesday, 06 / 3 / 2026

Hyd. No. 14

Post Total to Pond 1

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 11, 12, 13

Peak discharge = 26.82 cfs
Time to peak = 722 min
Hyd. volume = 129,897 cuft
Contrib. drain. area = 3.130 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

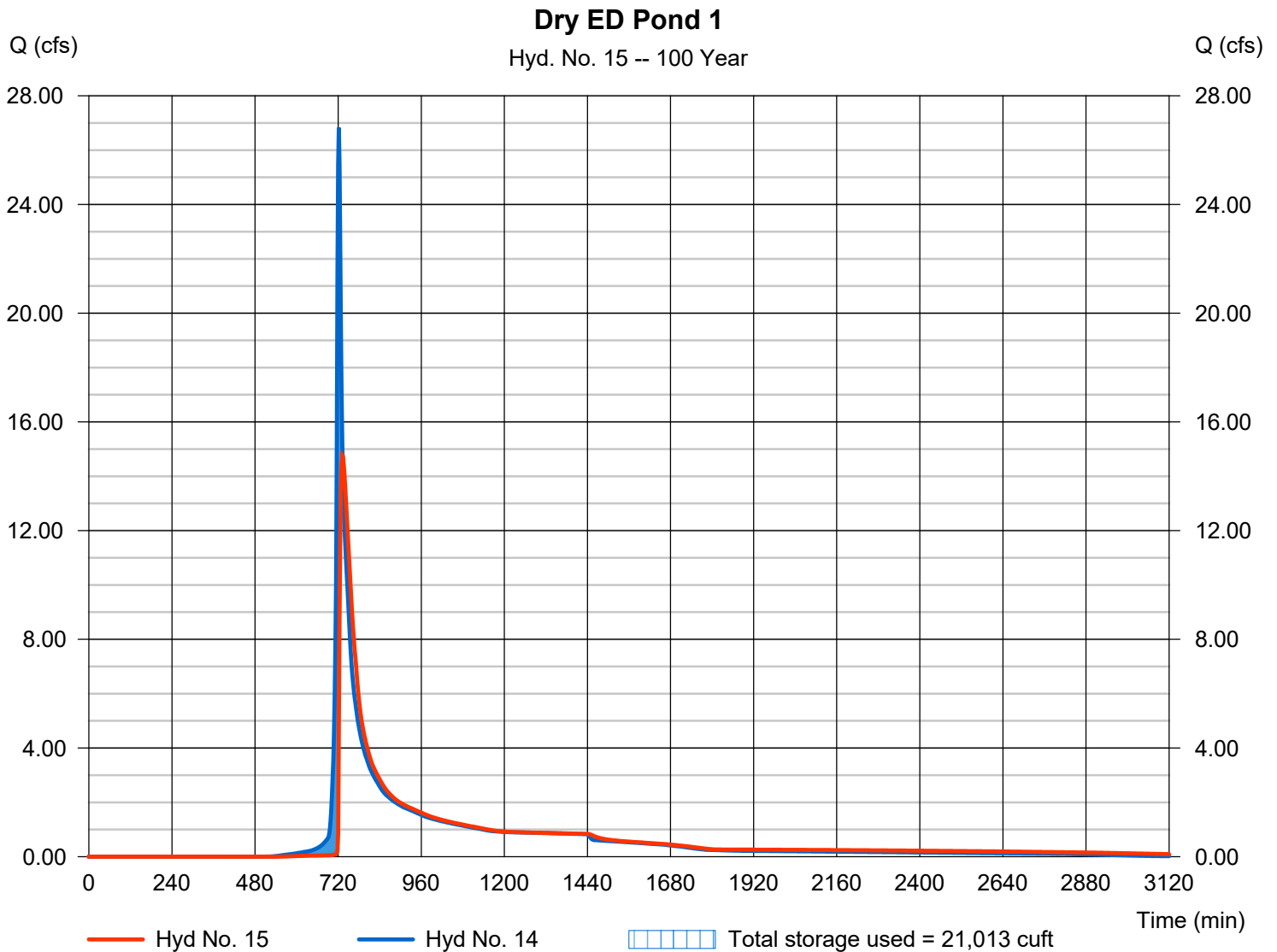
Wednesday, 06 / 3 / 2026

Hyd. No. 15

Dry ED Pond 1

Hydrograph type	= Reservoir	Peak discharge	= 14.77 cfs
Storm frequency	= 100 yrs	Time to peak	= 732 min
Time interval	= 2 min	Hyd. volume	= 129,881 cuft
Inflow hyd. No.	= 14 - Post Total to Pond 1	Max. Elevation	= 686.67 ft
Reservoir name	= Dry ED Pond 1	Max. Storage	= 21,013 cuft

Storage Indication method used.



Hydrograph Report

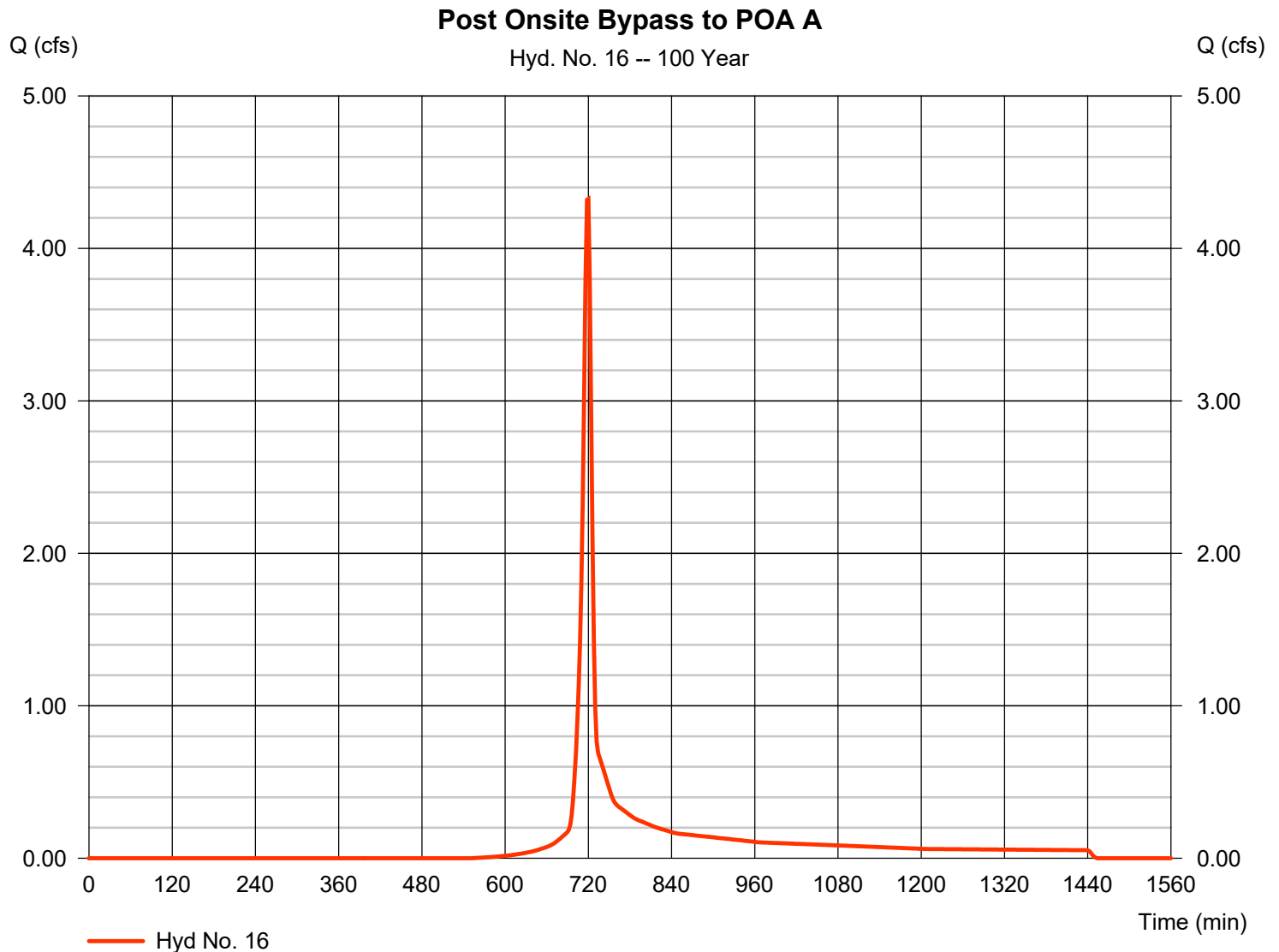
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 16

Post Onsite Bypass to POA A

Hydrograph type	= SCS Runoff	Peak discharge	= 4.324 cfs
Storm frequency	= 100 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 9,904 cuft
Drainage area	= 0.770 ac	Curve number	= 63
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

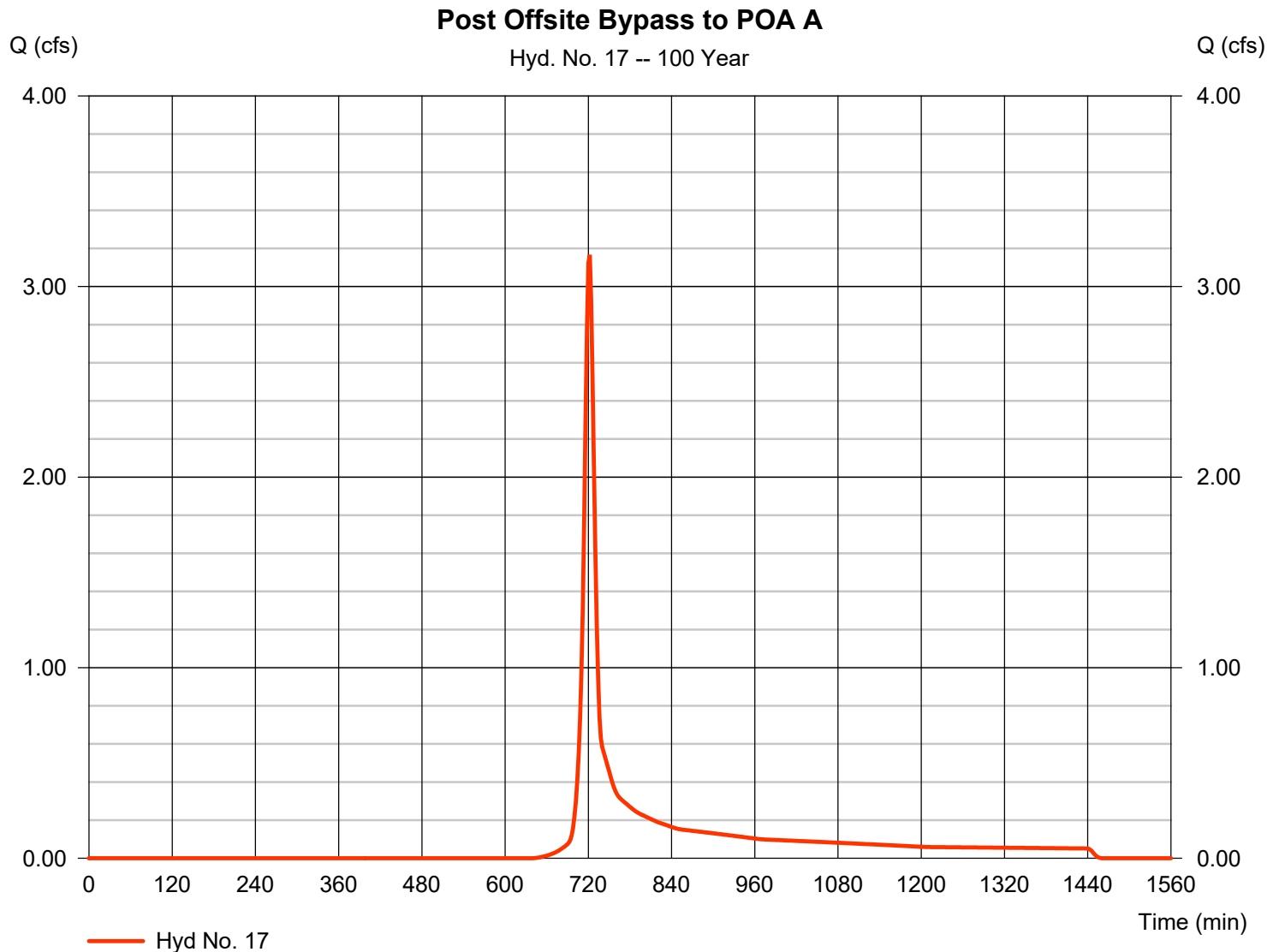
Wednesday, 06 / 3 / 2026

Hyd. No. 17

Post Offsite Bypass to POA A

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 2 min
Drainage area = 0.840 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.84 in
Storm duration = 24 hrs

Peak discharge = 3.166 cfs
Time to peak = 722 min
Hyd. volume = 8,412 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 11.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

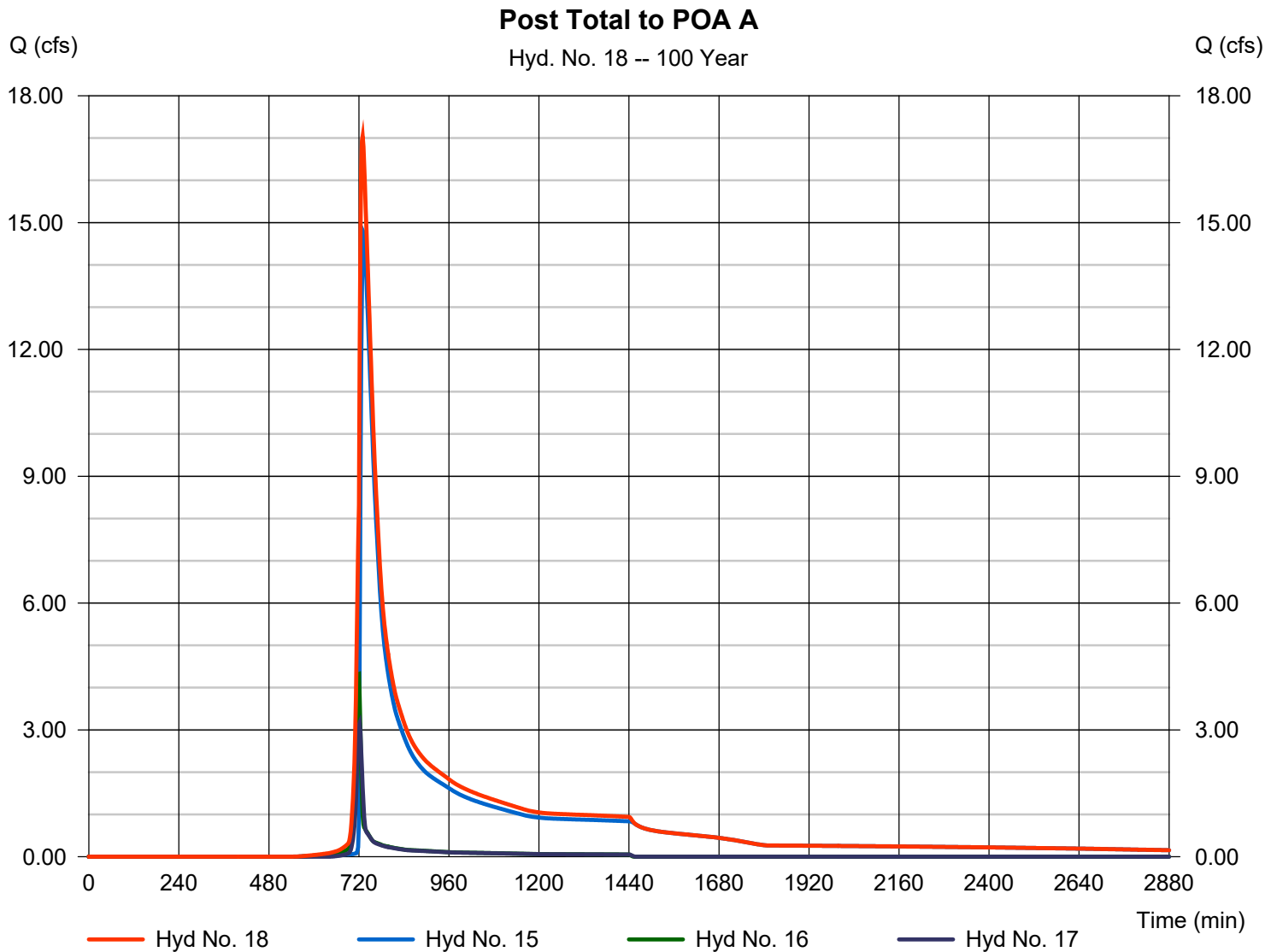
Wednesday, 06 / 3 / 2026

Hyd. No. 18

Post Total to POA A

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 15, 16, 17

Peak discharge = 17.04 cfs
Time to peak = 730 min
Hyd. volume = 148,197 cuft
Contrib. drain. area = 1.610 ac



Hydrograph Report

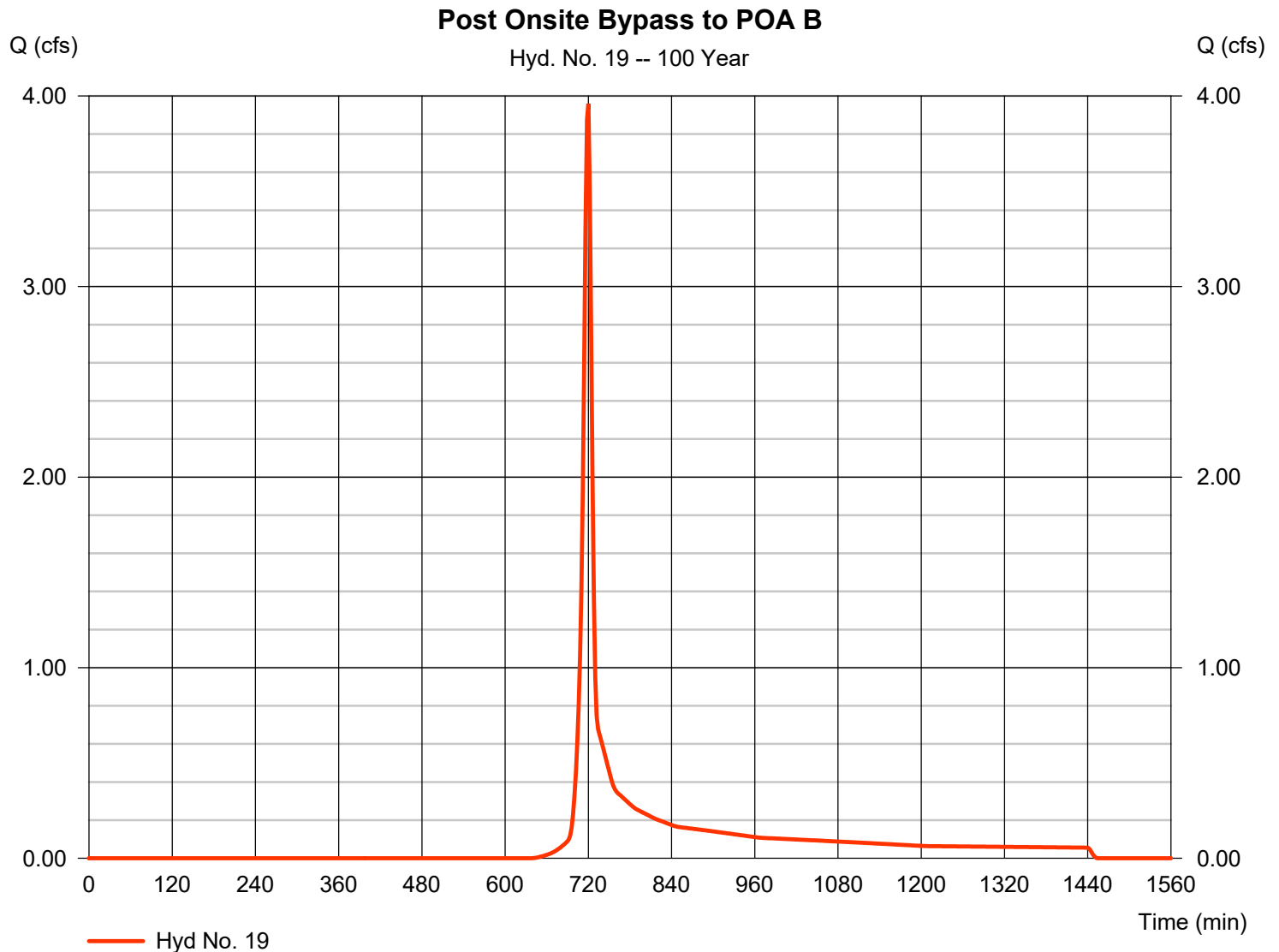
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Wednesday, 06 / 3 / 2026

Hyd. No. 19

Post Onsite Bypass to POA B

Hydrograph type	= SCS Runoff	Peak discharge	= 3.960 cfs
Storm frequency	= 100 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 9,129 cuft
Drainage area	= 0.940 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 7.84 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

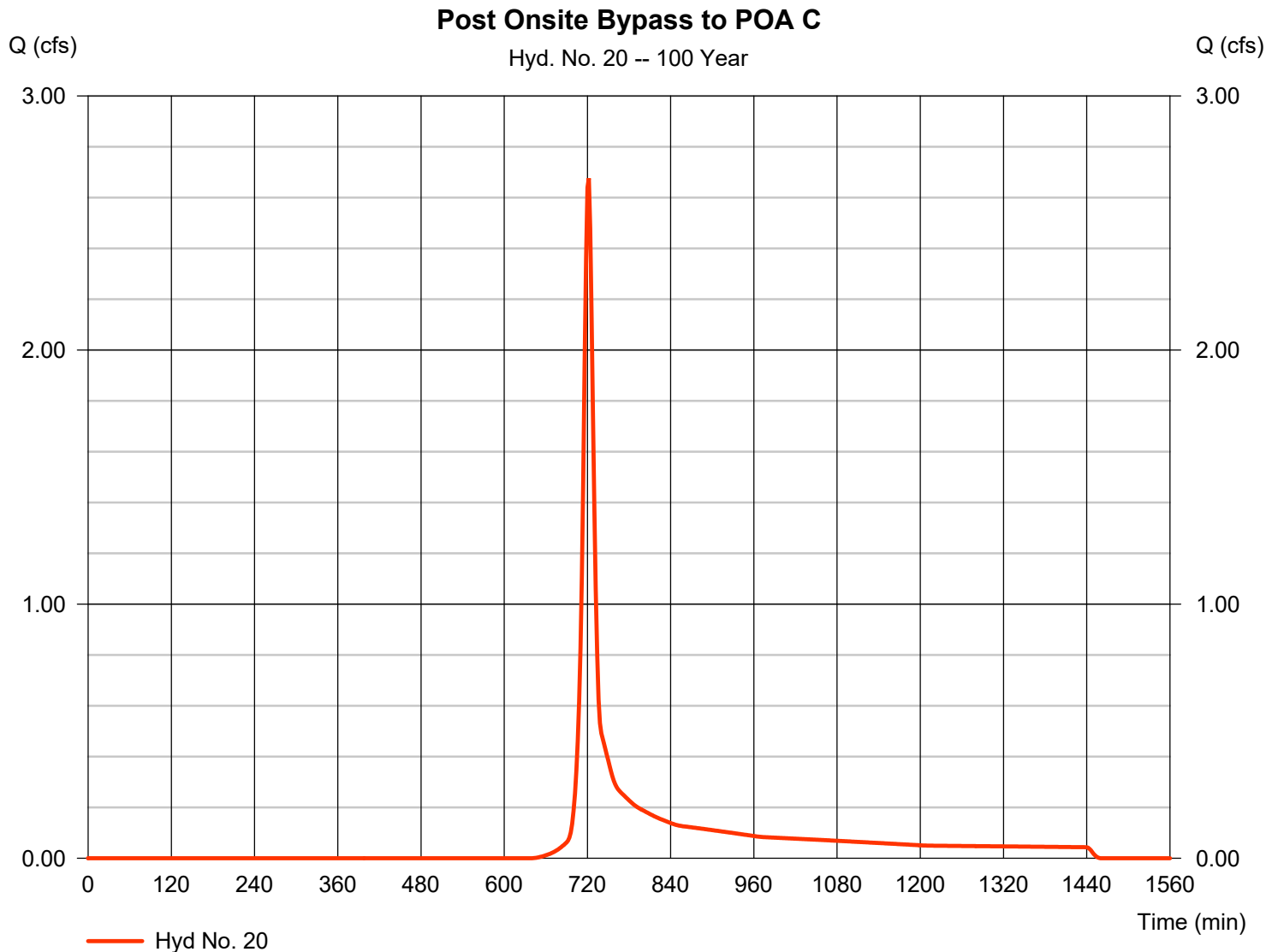
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 06 / 3 / 2026

Hyd. No. 20

Post Onsite Bypass to POA C

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.676 cfs
Storm frequency	=	100 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	7,110 cuft
Drainage area	=	0.710 ac	Curve number	=	55
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.00 min
Total precip.	=	7.84 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

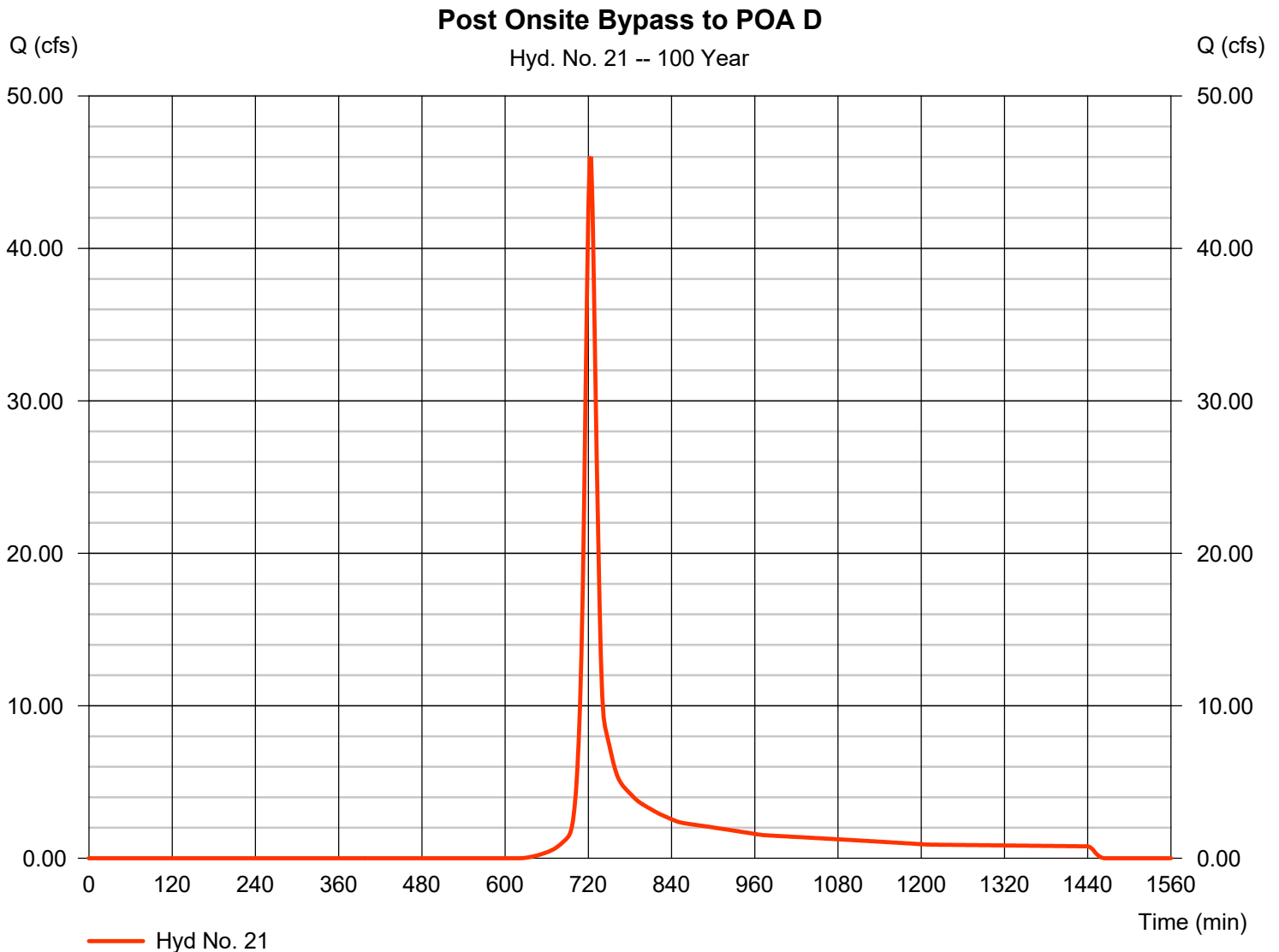
Wednesday, 06 / 3 / 2026

Hyd. No. 21

Post Onsite Bypass to POA D

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 2 min
Drainage area = 12.950 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.84 in
Storm duration = 24 hrs

Peak discharge = 45.92 cfs
Time to peak = 722 min
Hyd. volume = 132,410 cuft
Curve number = 57
Hydraulic length = 0 ft
Time of conc. (Tc) = 15.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

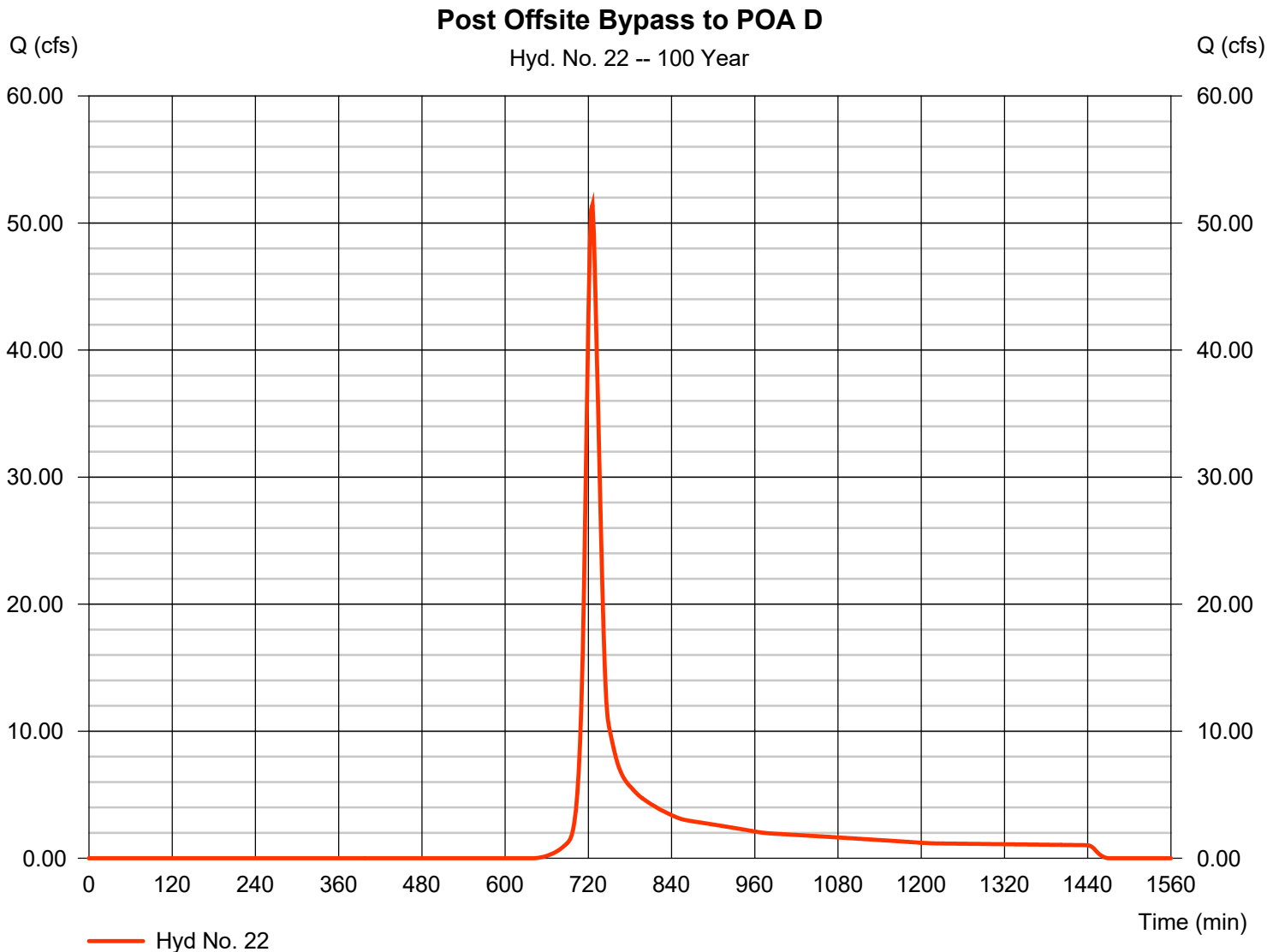
Wednesday, 06 / 3 / 2026

Hyd. No. 22

Post Offsite Bypass to POA D

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 2 min
Drainage area = 17.290 ac
Basin Slope = 0.0 %
Tc method = User
Total precip. = 7.84 in
Storm duration = 24 hrs

Peak discharge = 51.44 cfs
Time to peak = 726 min
Hyd. volume = 167,908 cuft
Curve number = 55
Hydraulic length = 0 ft
Time of conc. (Tc) = 17.00 min
Distribution = Type II
Shape factor = 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

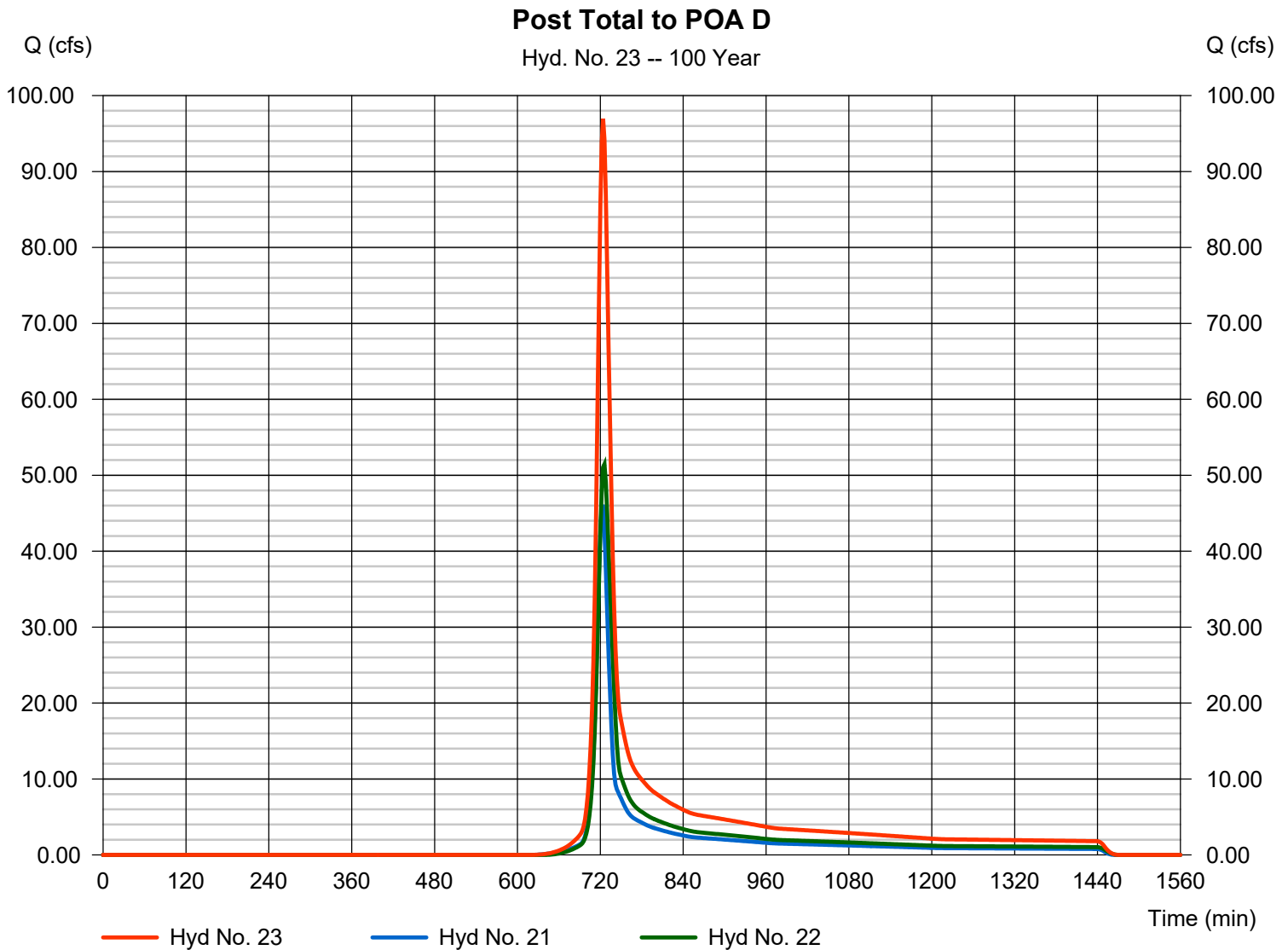
Wednesday, 06 / 3 / 2026

Hyd. No. 23

Post Total to POA D

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 21, 22

Peak discharge = 96.95 cfs
Time to peak = 724 min
Hyd. volume = 300,318 cuft
Contrib. drain. area = 30.240 ac



Hydrograph Return Period Recap..... 1

1 - Year

Summary Report.....	2
Hydrograph Reports.....	3
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	3
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	4
Hydrograph No. 3, Combine, Pre Total to POA A.....	5
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	6
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	7
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	8
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	9
Hydrograph No. 8, Combine, Pre Total to POA D.....	10
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	11
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	12
Pond Report - Dry ED Pond 2.....	13
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	14
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	15
Hydrograph No. 14, Combine, Post Total to Pond 1.....	16
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	17
Pond Report - Dry ED Pond 1.....	18
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	19
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	20
Hydrograph No. 18, Combine, Post Total to POA A.....	21
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	22
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	23
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	24
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	25
Hydrograph No. 23, Combine, Post Total to POA D.....	26

2 - Year

Summary Report.....	27
Hydrograph Reports.....	28
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	28
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	29
Hydrograph No. 3, Combine, Pre Total to POA A.....	30
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	31
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	32
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	33
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	34
Hydrograph No. 8, Combine, Pre Total to POA D.....	35
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	36
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	37
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	38
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	39
Hydrograph No. 14, Combine, Post Total to Pond 1.....	40
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	41
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	42

Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	43
Hydrograph No. 18, Combine, Post Total to POA A.....	44
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	45
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	46
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	47
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	48
Hydrograph No. 23, Combine, Post Total to POA D.....	49

5 - Year

Summary Report.....	50
Hydrograph Reports.....	51
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	51
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	52
Hydrograph No. 3, Combine, Pre Total to POA A.....	53
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	54
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	55
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	56
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	57
Hydrograph No. 8, Combine, Pre Total to POA D.....	58
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	59
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	60
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	61
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	62
Hydrograph No. 14, Combine, Post Total to Pond 1.....	63
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	64
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	65
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	66
Hydrograph No. 18, Combine, Post Total to POA A.....	67
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	68
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	69
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	70
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	71
Hydrograph No. 23, Combine, Post Total to POA D.....	72

10 - Year

Summary Report.....	73
Hydrograph Reports.....	74
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	74
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	75
Hydrograph No. 3, Combine, Pre Total to POA A.....	76
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	77
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	78
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	79
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	80
Hydrograph No. 8, Combine, Pre Total to POA D.....	81
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	82
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	83
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	84
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	85
Hydrograph No. 14, Combine, Post Total to Pond 1.....	86

Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	87
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	88
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	89
Hydrograph No. 18, Combine, Post Total to POA A.....	90
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	91
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	92
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	93
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	94
Hydrograph No. 23, Combine, Post Total to POA D.....	95

25 - Year

Summary Report.....	96
Hydrograph Reports.....	97
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	97
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	98
Hydrograph No. 3, Combine, Pre Total to POA A.....	99
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	100
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	101
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	102
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	103
Hydrograph No. 8, Combine, Pre Total to POA D.....	104
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	105
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	106
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	107
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	108
Hydrograph No. 14, Combine, Post Total to Pond 1.....	109
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	110
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	111
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	112
Hydrograph No. 18, Combine, Post Total to POA A.....	113
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	114
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	115
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	116
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	117
Hydrograph No. 23, Combine, Post Total to POA D.....	118

50 - Year

Summary Report.....	119
Hydrograph Reports.....	120
Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	120
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	121
Hydrograph No. 3, Combine, Pre Total to POA A.....	122
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	123
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	124
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	125
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	126
Hydrograph No. 8, Combine, Pre Total to POA D.....	127
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	128
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	129
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	130

Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	131
Hydrograph No. 14, Combine, Post Total to Pond 1.....	132
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	133
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	134
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	135
Hydrograph No. 18, Combine, Post Total to POA A.....	136
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	137
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	138
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	139
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	140
Hydrograph No. 23, Combine, Post Total to POA D.....	141

100 - Year

Summary Report.....	142
----------------------------	------------

Hydrograph Reports.....	143
--------------------------------	------------

Hydrograph No. 1, SCS Runoff, Pre Onsite to POA A.....	143
Hydrograph No. 2, SCS Runoff, Pre Offsite to POA A.....	144
Hydrograph No. 3, Combine, Pre Total to POA A.....	145
Hydrograph No. 4, SCS Runoff, Pre Onsite to POA B.....	146
Hydrograph No. 5, SCS Runoff, Pre Onsite to POA C.....	147
Hydrograph No. 6, SCS Runoff, Pre Onsite to POA D.....	148
Hydrograph No. 7, SCS Runoff, Pre Offsite to POA D.....	149
Hydrograph No. 8, Combine, Pre Total to POA D.....	150
Hydrograph No. 10, SCS Runoff, Post Onsite to Pond 2.....	151
Hydrograph No. 11, Reservoir, Dry ED Pond 2.....	152
Hydrograph No. 12, SCS Runoff, Post Onsite to Pond 1.....	153
Hydrograph No. 13, SCS Runoff, Post Offsite to Pond 1.....	154
Hydrograph No. 14, Combine, Post Total to Pond 1.....	155
Hydrograph No. 15, Reservoir, Dry ED Pond 1.....	156
Hydrograph No. 16, SCS Runoff, Post Onsite Bypass to POA A.....	157
Hydrograph No. 17, SCS Runoff, Post Offsite Bypass to POA A.....	158
Hydrograph No. 18, Combine, Post Total to POA A.....	159
Hydrograph No. 19, SCS Runoff, Post Onsite Bypass to POA B.....	160
Hydrograph No. 20, SCS Runoff, Post Onsite Bypass to POA C.....	161
Hydrograph No. 21, SCS Runoff, Post Onsite Bypass to POA D.....	162
Hydrograph No. 22, SCS Runoff, Post Offsite Bypass to POA D.....	163
Hydrograph No. 23, Combine, Post Total to POA D.....	164

IDF Report.....	165
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