

BROWN AND CALDWELL  
900 HAMMOND DRIVE, SUITE 500  
ATLANTA, GA 30328



THIS DRAWING IS NOT VALID FOR CONSTRUCTION PURPOSES UNLESS IT BEARS THE SEAL AND SIGNATURE OF A DULY REGISTERED PROFESSIONAL

Project Status



Project Name

REVISIONS

REV	DATE	DESCRIPTION

DESIGNED: Designer  
DRAWN: Author  
CHECKED: Checker  
CHECKED:  
APPROVED: Approver  
FILENAME  
BC PROJECT NUMBER  
Project Number  
CLIENT PROJECT NUMBER

LEVEL 2 FRAMING PLAN

DRAWING NUMBER  
**S102**

**LEVEL 2 FRAMING PLAN**

SCALE: 1/8" = 1'-0"

**STEEL FLOOR FRAMING PLAN NOTES:**

1. DENOTES 3 1/2" NORMAL WEIGHT CONCRETE ON 2"-20 GAUGE COMPOSITE METAL DECK (TOTAL THICKNESS = 5 1/2") REINFORCED W/ 4 PCY MACRO-SYNTHETIC FIBERS.  
MINIMUM DECK PROPERTIES:  
 $L_p = 0.417 \text{ IN}^2/\text{FT}$      $L_c = 0.412 \text{ IN}^2/\text{FT}$   
 $S_{ps} = 0.342 \text{ IN}^3/\text{FT}$      $S_{pc} = 0.347 \text{ IN}^3/\text{FT}$
2. T/SLAB = SEE LEVEL SCHEDULE THIS SHEET.
3. T/STEEL = SEE LEVEL SCHEDULE THIS SHEET.
4. DENOTES THE AMOUNT OF CAMBER ON THE BEAM OR GIRDER  
DENOTES QUANTITY OF 3/4" DIA x 4" LG HEADED STUD ANCHORS WELDED TO TOP FLANGE OF STEEL BEAM (NC): DENOTES NON-COMPOSITE BEAM  
DENOTES BEAM SIZE  
TOP OF STEEL ELEVATION (UNLESS NOTED OTHERWISE)  
W#x# (C)    #    C=#"
5. FOR TYPICAL COMPOSITE STEEL FRAMING SECTIONS AND DETAILS SEE S511 - S512
6. GENERAL CONTRACTOR SHALL VERIFY ALL OPENING DIMENSIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. V=#K DENOTES DESIGN SERVICE LEVEL (ALLOWABLE STRESS DESIGN) SHEAR REACTION. IF REACTION IS NOT SHOWN, DESIGN FOR 15 K.
8. DENOTES MOMENT CONNECTION. DESIGN CONNECTIONS FOR FORCES INDICATED ON PLAN AND IN ELEVATIONS. FOR CONNECTION DETAIL - SEE 4/S501.
9. DENOTES BEAM-THRU-BEAM MOMENT CONNECTION. DESIGN CONNECTIONS FOR FORCES INDICATED ON PLAN AND IN ELEVATIONS. FOR CONNECTION DETAIL - SEE 4/S501.
10. DENOTES DESIGN SERVICE LEVEL (ALLOWABLE STRESS DESIGN) MOMENT REACTION (+ OR -). IF FORCE IS NOT SHOWN, DESIGN FOR 25 K-FT.
11. DENOTES DRAG CONNECTION. DESIGN CONNECTIONS FOR AXIAL FORCES INDICATED ON PLAN AND IN ELEVATIONS. DRAG CONNECTIONS TO HSS COLUMNS SHALL BE THRU-PLATES. UNO - SEE 2/S501
12. DENOTES DESIGN SERVICE LEVEL (ALLOWABLE STRESS DESIGN) AXIAL DRAG FORCE. IF FORCE IS NOT SHOWN, DESIGN FOR 5 K.
13. DENOTES BEAM BOTTOM FLANGE BRACE PER DETAIL 3/S512.
14. POST-UP DENOTES COLUMN POST UP PER DETAIL 1/S501.

**WOOD ROOF FRAMING PLAN NOTES:**

1. DENOTES 5/8" WOOD ROOF SHEATHING ON GABLED PRE-ENGINEERED WOOD ROOF TRUSSES AT 24" OC, UNLESS NOTED OTHERWISE.
2. TRUSS BEARING ELEVATION = 12' - 0", UNLESS NOTED OTHERWISE.
3. TG DENOTES PRE-ENGINEERED WOOD TRUSS GIRDER. PROVIDE A STUD PACK WITH ONE MORE STUD THAN THE NUMBER OF PLYS IN TRUSS GIRDER DOWN TO FOUNDATION, UNLESS NOTED OTHERWISE ON SCHEDULE.
4. HG DENOTES PRE-ENGINEERED WOOD TRUSS HIP GIRDER. PROVIDE A STUD PACK WITH ONE MORE STUD THAN THE NUMBER OF PLYS IN TRUSS GIRDER DOWN TO FOUNDATION, UNLESS NOTED OTHERWISE ON SCHEDULE.
5. ST DENOTES PRE-ENGINEERED WOOD SHEAR TRUSS TO ALIGN WITH SHEAR WALL BELOW. DESIGN FOR IN-PLANE SHEAR LOAD PER SHEAR WALL SCHEDULE. FASTEN SHEAR TRUSS TO TOP OF SHEAR WALL BELOW PER 4/S611
6. ALL ROOF TRUSSES SHALL HAVE UPLIFT TIES AT ALL BEARING POINTS PER ROOF TRUSS TIE DOWN SCHEDULE ON THIS SHEET. REACTIONS SHALL BE PROVIDED BY THE TRUSS ENGINEER IN THE FINAL, FOR CONSTRUCTION TRUSS SHOP DRAWINGS.
7. X# - # DENOTES WOOD BEAM - SEE SCHEDULE ON THIS SHEET.  
NOMINAL BEAM DEPTH  
NUMBER OF PLYS  
D = DIMENSIONED LUMBER  
L = LVL
8. ALL WOOD ROOF BEAMS BEARING ON WALLS SHALL HAVE UPLIFT TIES AT BEARING POINTS - SEE WOOD ROOF BEAM SCHEDULE ON THIS SHEET.
9. FOR TYPICAL ROOF DIAPHRAGM FASTENING - SEE DETAIL 2/S611, UNLESS NOTED OTHERWISE ON PLAN
10. SEE FLOOR LEVEL BELOW FOR SHEAR WALL LOCATIONS. PROVIDE SHEAR TRUSS OVER THESE LOCATIONS CAPABLE OF TRANSFERRING LOAD TO WALL NOTED ON SHEAR WALL SCHEDULE.
11. DENOTES LOAD-BEARING WALLS BELOW.

**LEVEL SCHEDULE**

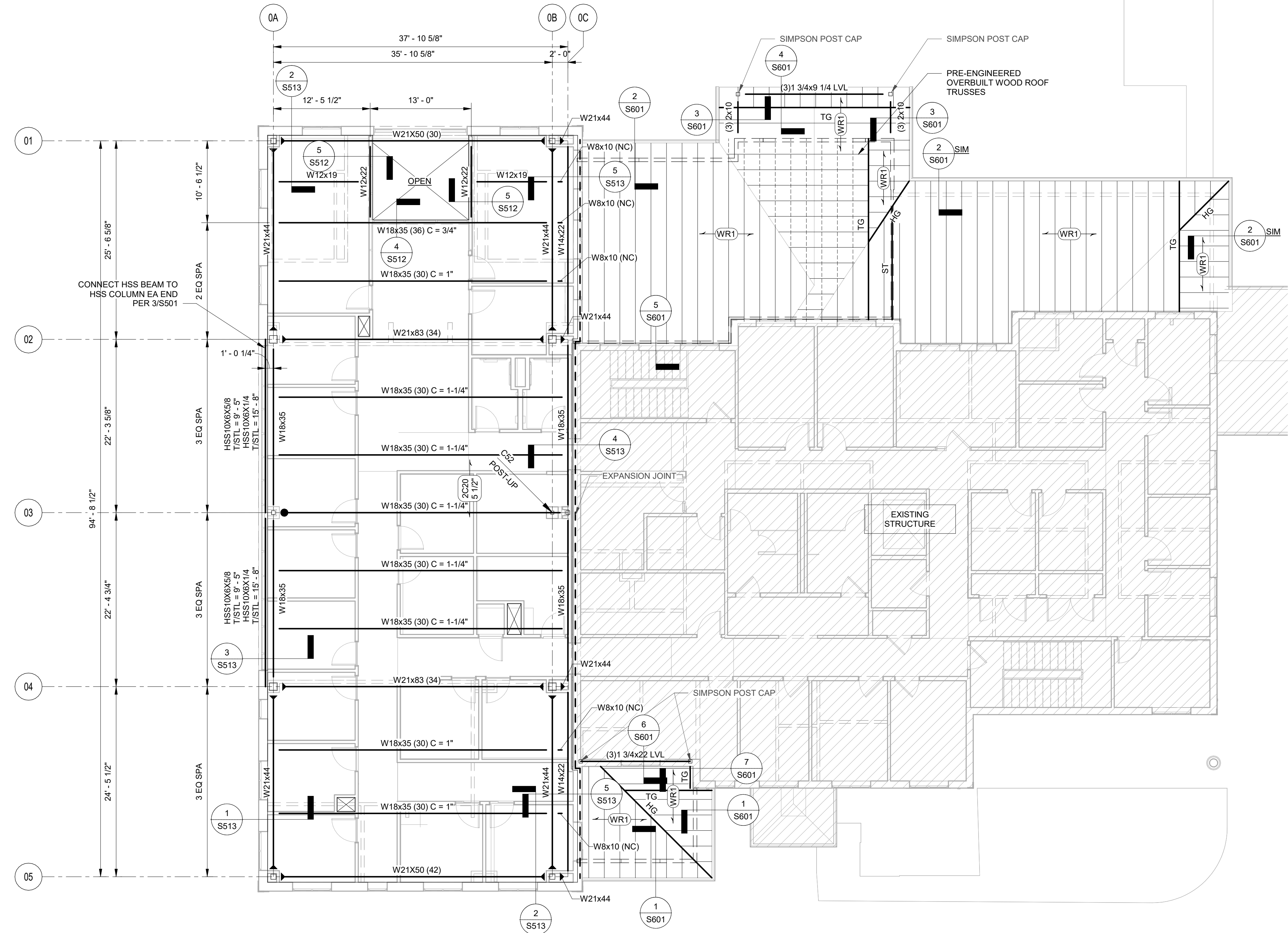
NAME	ELEVATION
FFE	0"
LEVEL 2 - T/STL	12' - 6 1/2"
LEVEL 2 - T/SLAB	13' - 0"

**WOOD BEAM SCHEDULE**

BEAM TYPE	STUD PACKS LEVEL 1	HANGER (WHERE REQUIRED)
(3) 1 3/4x9 1/4 LVL	SEE PLAN	
(3) 1 3/4x22 LVL	SEE PLAN	
(3) 2x10	(3) 2x6	

**ROOF TRUSS TIE DOWN SCHEDULE**

UPLIFT REACTION AT BEARING POINTS OF ROOF TRUSS (ASD)	SIMPSON TIE-ROOF @ EACH END OF BEAM	NAILS TO TRUSS	NAILS TO PL	SDS SCREWS TO TRUSS	THRU-BOLTS TO PL	REMARKS
< 540 LB	H2.5A SIMPSON TIE	0.131" x 1 1/2"	0.131" x 1 1/2"	-	-	-
540 LB ≤ REACTION < 1080 LB	(2) H2.5A SIMPSON TIES	0.131" x 1 1/2"	0.131" x 1 1/2"	-	-	-
1080 LB ≤ REACTION < 1420 LB	(2) H8 SIMPSON TIES	0.148" x 1 1/2"	0.148" x 1 1/2"	-	-	-
1420 LB ≤ REACTION < 3990 LB	(2) VGT/LR SIMPSON TIEDOWNS	-	-	1/4" x 3"	(2) 5/8" DIA	2 PLY MIN W/ HDU4 EA FACE BELOW TOP PL; 1/4" x 3" SQ WASHER PL
3990 LB ≤ REACTION < 6485 LB	HGT-2 SIMPSON TIEDOWN	0.148" x 3"	-	-	(2) 5/8" DIA	2 PLY W/ FULL-HEIGHT THREADED ROD; 1/4" x 3" SQ WASHER PL
6485 LB ≤ REACTION < 9035 LB	HGT-3 SIMPSON TIEDOWN	0.148" x 3"	-	-	(2) 5/8" DIA	3 PLY W/ FULL-HEIGHT THREADED ROD; 1/4" x 4" SQ WASHER PL



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