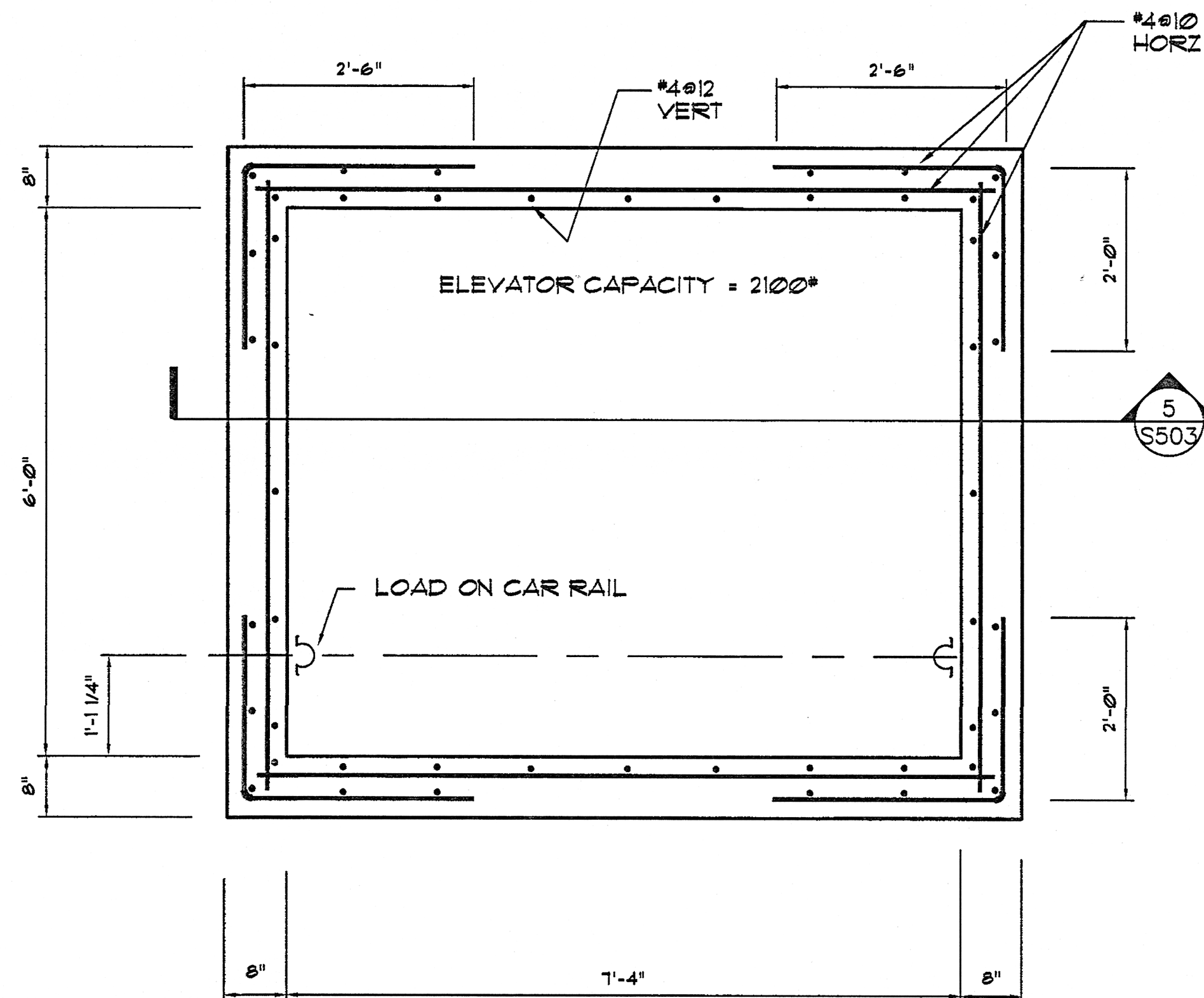
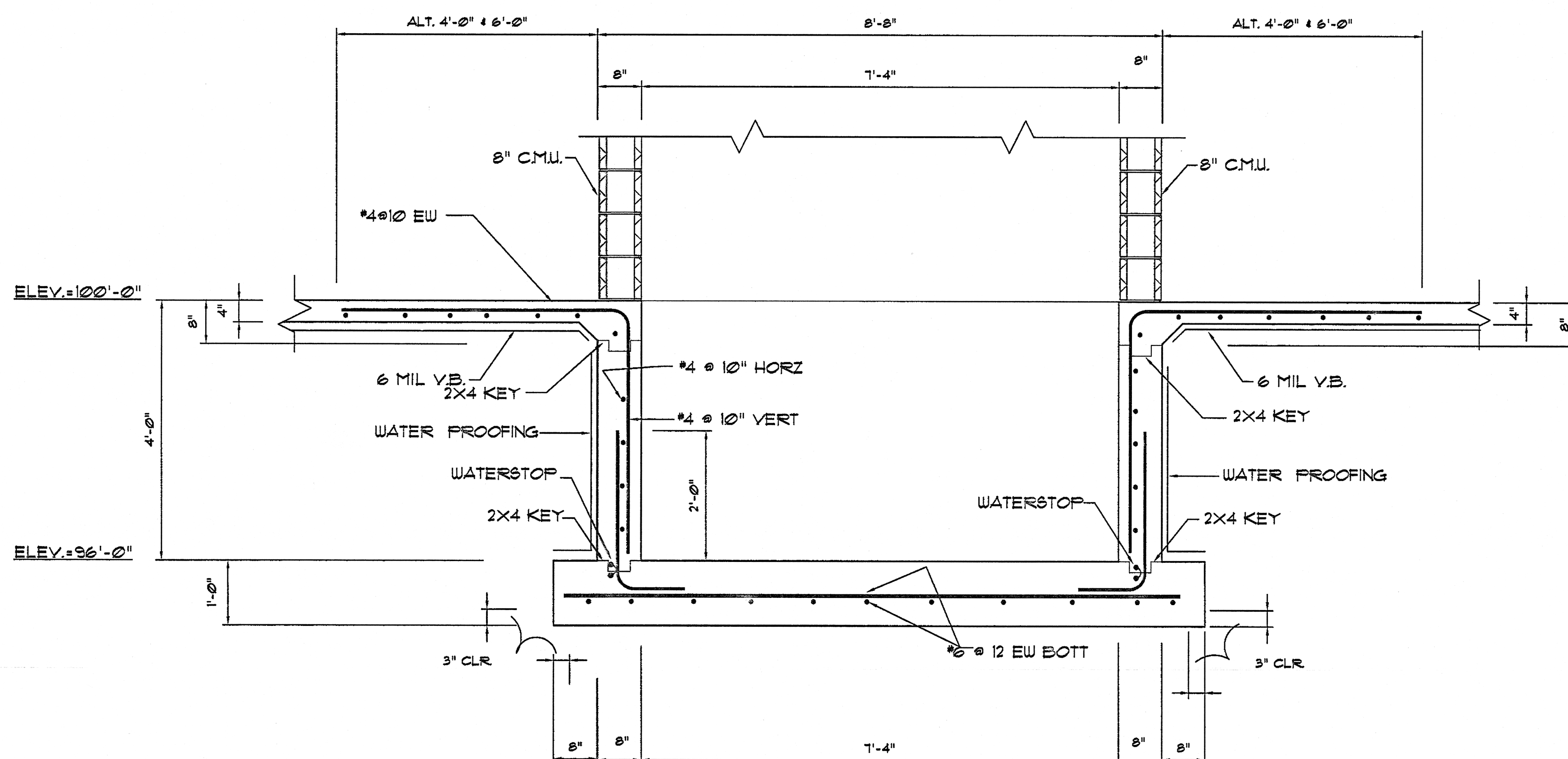


TYP. SLAB JOINT DETAILS
NOT TO SCALE



ELEVATOR PIT PLAN
SCALE: 3/4" = 1'-0"
PLAN NORTH



SECTION
SCALE: 3/4" = 1'-0"

NOTE: CONTRACTOR TO VERIFY DIMENSIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION

GENERAL NOTES:

1. BASED ON STANDARD BUILDING CODE 2000 EDITION
2. SOIL BEARING
 - A. DESIGN SOIL BEARING PRESSURE - 2,000 PSF TO BE VERIFIED BY GEOTECHNICAL ENGINEER
 - B. GEOTECHNICAL ENGINEERS FOR THE PROJECT ARE:
GEORGIA TESTING LABORATORIES, INC.
1555 WILLIAMS DRIVE NE - SUITE 103
MARIETTA, GEORGIA 30066
(770) 915-1332
CONTACT: LARRY JACOBS, P.E.
3. DESIGN LIVE LOADS:
 - A. ROOF - 20 PSF
 - B. COLLATERAL LOAD - 10 PSF
 - C. SECOND FLOOR AND ELEVATED SETTING - 100 PSF
 - D. WIND: 90 MPH EXPOSURE GROUP C

HEIGHT	WALL PRESSURE	FACTOR
15'	15.5 PSF	1.21
20'	16.5 PSF	1.23
25'	17.2 PSF	1.25
30'	17.9 PSF	1.27
35'	18.5 PSF	1.29
40'	19.0 PSF	1.31
45'	19.5 PSF	1.33
 - E. SNOW: 5 PSF
4. SEISMIC -
 - I. SITE CLASS = D
 - II. SEISMIC USE GROUP = II
 - III. $S_s = 0.20 g$, $S_1 = 0.09 g$
 - IV. $S_{ds} = 0.21 g$, $S_{d1} = 0.15 g$
 - V. SEISMIC DESIGN CATEGORY = C
 - VI. ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE
 - VII. STRUCTURAL SYSTEMS:
 - N-S ORDINARY STEEL MOMENT FRAME, $R=4$ $C_d=3/4$
 - E-W ORDINARY CONCENTRICALLY BRACED FRAME, $R=6$ $C_d=5$
5. FOOTINGS:
 - A. TOP OF SLAB AND FOOTINGS TO BE AS SHOWN ON THE DRAWINGS.
 - B. ALL FILL PLACED BELOW FOOTINGS SHALL BE COMPACTED TO NOT LESS THAN 95% OF MODIFIED PROCTOR (ASTM - D1557), AND BE TESTED BY THE GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL ALSO VERIFY THE SOIL BEARING CAPACITY.
 - C. FOOTINGS ARE NOT TO BE LOADED UNTIL THE CONCRETE HAS REACHED ITS DESIGN STRENGTH.
 - D. ALL EARTHWORK, FOOTING EXCAVATION, AND PREPARATION TO BE IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
 - E. BOTTOM OF FOOTING DEPTH TO BE A MIN. OF 18" BELOW FINISH GRADE.
 - F. PRE-ENG. BUILDING FOOTINGS SIZED FOR PRICING ONLY. CONTRACTOR TO SUBMIT REACTIONS PROVIDED BY METAL BUILDING MANUF. TO ENGINEER PRIOR TO CONSTRUCTION.
 - G. ALL GROUT TO BE NON-SHRINK GROUT.
6. CONCRETE:
 - A. CONCRETE COMPRESSIVE STRENGTH TO BE 3000 PSI, 28 DAYS, NORMAL WEIGHT (145 PCF.) EXCEPT SECOND FLOOR DECK CONCRETE TO BE LIGHT WEIGHT.
 - B. CONTRACTOR IS NOT TO ADD WATER TO READY-MIXED CONCRETE AT JOB SITE.
 - C. CONCRETE WILL NOT BE ALLOWED TO BE PLACED WHICH HAD WATER ADDED AT THE PLANT MORE THAN ONE HOUR BEFORE IT IS ABLE TO BE POURED.
 - D. APPLY COMBINED CONCRETE CURING COMPOUND AND SEALER UNO. CONTRACTOR NOT TO USE CURING COMPOUND ON ANY AREAS OWNER WISHES TO USE A SEALER OR TILE GROUT. NON-CURING COMPOUND AREAS TO BE CURED BY KEEPING WET. CONTRACTOR TO VERIFY LOCATION OF ANY AREAS TO BE SEALED WITH OWNER.
 - E. CONCRETE TEST REPORTS TO BE AVAILABLE AT THE JOB SITE.
 - F. CONCRETE MIX, TRANSPORTATION, AND PLACEMENT TO BE IN ACCORDANCE WITH THE LATEST A.C.I. STANDARDS.
 - G. THE BASE OF ALL FOUNDATION EXCAVATIONS SHOULD BE FREE OF WATER AND LOOSE SOIL PRIOR TO PLACING CONCRETE.
7. REINFORCING STEEL:
 - A. SHALL BE DETAILED, FABRICATED, AND PLACED ACCORDING TO THE LATEST STANDARDS OF THE AMERICAN CONCRETE INSTITUTE.
 - B. PROVIDE CORNER BARS AT ALL CORNERS OF GRADE BEAMS OR WALLS OF THE SAME SIZE AND NUMBER AS LARGER OF THE ADJACENT BEAM OR WALL BARS.
 - C. MESH SHALL BE PLACED IN THE CENTER OF ALL SLABS UNLESS NOTED OTHERWISE. JOINTS IN MESH ARE TO LAP A MINIMUM OF 6" UNLESS OTHERWISE NOTED.
 - D. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM-A615 GRADE 60.
 - E. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
 - F. CONTINUOUS REBAR IN REINFORCED CONCRETE TO LAP A MINIMUM OF 30 BAR DIAMETERS 180" (ACI 3.18).
8. CONSTRUCTION JOINTS:
 - A. CONSTRUCTION JOINTS TO BE LOCATED AS SHOWN ON DRAWINGS.
 - B. SLABS SHALL BE POURED SO THAT NO SECTION SHALL COVER AN AREA OF MORE THAN 1200 SQUARE FEET NOR SHALL THE LENGTH OF ANY SECTION EXCEED TWICE ITS WIDTH AND BE POURED IN A CHECKERBOARD PATTERN.
9. STRUCTURAL STEEL:
 - A. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM A-36. EXCEPT TUBE SECTIONS TO BE ASTM A500 GRADE B.
 - B. SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF THE A.I.S.C.
 - C. SHOP OR FIELD SPLICES BETWEEN THE SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED.
 - D. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION.

10. STEEL CONNECTIONS:
 - A. BOLTED:
 1. BOLTS SHALL BE HIGH STRENGTH A-325 BOLTS OF SIZE AND NUMBER AS SHOWN ON DRAWINGS.
 2. CONNECTIONS SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 BOLTS."
 3. WHERE NOT SHOWN OTHERWISE ON THE PLANS, BOLTED OR WELDED CONNECTIONS MAY BE USED PROVIDED THEY DEVELOP THE MAXIMUM REACTIONS OF BEAMS, AND THE ALLOWABLE TENSION IN ANGLES, RODS AND BARS.
 - B. WELDED:
 1. ELECTRODES FOR WELDED CONNECTIONS TO BE E-70.
 2. ALL WELDING SHALL CONFORM TO THE LATEST CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION OF THE AMERICAN WELDING SOCIETY AND ALL WORK SHALL BE PERFORMED BY FULLY QUALIFIED AND EXPERIENCED WELDERS IN THE TYPE OF CONSTRUCTION INVOLVED. WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.
11. STEEL DECK:
 - A. ROOF DECK
 - 15B, 22 GAGE, PAINTED
 - DEPTH = 1 1/2" $U_L = 160$ PSF
 - $S_p = 0.168$ IN²/FT
 - $S_n = 0.194$ IN²/FT
 - B. SECOND FLOOR DECK
 - 15VL1, 22 GAGE, GALV
 - DEPTH = 1 1/2" $U_L = 178$ PSF
 - $S_p = .181$ IN²/FT
 - $S_n = .189$ IN²/FT
 - C. DECK TO BE ATTACHED TO FRAMING BY WELDING PER THE MANUFACTURER'S WRITTEN RECOMMENDATIONS. SIDE LAP FASTENERS TO BE USED PER FACTORY MUTUAL REQUIREMENTS.
 - D. METAL DECK TO BE AS MANUFACTURED BY VULCRAFT OR APPROVED EQUIVALENT.
12. PAINT:
 - A. ALL INTERIOR STEEL TO HAVE A PRIME COAT OF PAINT.
 - B. ALL EXTERIOR STEEL TO HAVE 1 PRIME COAT AND 1 FINISH COAT OF PAINT.
13. PRE-ENGINEERED BUILDING:
 - A. BUILDING MANUFACTURER TO PROVIDE ENGINEER WITH BUILDING FRAME REACTIONS AND ANCHOR BOLT PLAN FOR REVIEW.
 - B. SHOP DRAWINGS FOR PRE-ENGINEERED BUILDING TO BE SUBMITTED TO THE ENGINEER AND BE AVAILABLE AT THE JOB SITE.
 - C. ANCHOR BOLTS TO BE PROVIDED AND PLACED BY GENERAL CONTRACTOR.
14. MASONRY:
 - A. MASONRY CONSTRUCTION SHALL CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND SPECIFICATIONS FOR MASONRY STRUCTURES.
 - B. WHERE STRUCTURE HAS BEEN DESIGNED AS A LOAD BEARING WALL STRUCTURE. ALL MASONRY UNITS SHALL BE LAID PRIOR TO CONC. PLACEMENT OF COLUMNS, BEAMS, AND SLABS OF SAME STORY.
 - C. USE TYPE "N" MORTAR
 - D. MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI ON NET SECTION, TO PROVIDE NET AREA COMPRESSIVE STRENGTH OF MASONRY (7"m) OF 1500 PSI.
 - E. PROVIDE FILLED CELLS AS SHOWN ON PLANS. IN ADDITION, PROVIDE FILLED CELLS ADJACENT TO ALL OPENINGS, AT ANCHORAGE OF CONNECTIONS.
 - F. PROVIDE FULL MORTAR BEDDING AROUND ALL FILLED CELLS W/VERTICAL REINFORCING.
 - G. REINFORCE WALL W/TRUSS TYPE REINFORCEMENT (DUR-O-WALL OR EQUAL) IN BED JOINTS AT 16" OC MEASURED VERTICALLY. LAP SPLICE ALL HORIZONTAL WALL REINFORCING 6". PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT ALL INTERSECTING WALLS.
 - H. REINFORCING FOR FILLED CELLS SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE THE FOLLOWING LAP SPLICES FOR REINF.:
 - #4 BARS = 24"
 - #5 BARS = 30"
 - #6 BARS = 36"
15. REFER TO TYPICAL WALL SECTIONS FOR MAX. CONSTRUCTION HEIGHT OF MASONRY WALLS. PROVIDE CLEAN-OUT HOLES AT BASE OF FILLED CELL WHEN THE CONC. FOUR EXCEEDS 5'-0" IN HEIGHT.

revisions

for construction
This drawing, as an instrument of service, is and shall remain the property of the Architect and Engineers and shall not be reproduced, published or used in any way without the written permission of the Architect or Engineer.
Copyright © 1998 By John Ware & Associates, Inc.

seal
Professional Engineer
No. 05508
John Ware & Associates, Inc.

John Ware & Associates, Inc.
Architects & Engineers
1954 Airport Road
Suite 100
Atlanta, GA 30341
(770) 451-1921
FAX (770) 457-1920

project
CHURCH IN THE NOW

address
1873 1/2 1933
IRIS DRIVE
CONYERS, GA
for
COVENANT SERVICES INC
1711 WESLEY WAY
CONYERS, GA

sheet description
FOUNDATION DETAILS

drawn MSM
checked JMW

job 01-145-B
date 08/15/02

S-503
of 16